

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

ROY COOPER
GOVERNOR

J.R. "JOEY" HOPKINS
SECRETARY

May 22, 2024

County: Wake

Subject: Encroachment Contract

SR 2053

E051-092-23-01269

Rivercrest Realty Associates, LLC 8816 Six Forks Rd, #201 Raleigh, NC 27615

Dear Sir or Madam,

Attached for your files is a copy of Right of Way Encroachment Agreement, which has been properly executed. This contract covers the following:

Approximately 700 LF of road widening for the construction of a left and right turn lane.

A PERFORMANCE AND INDEMNITY BOND IN THE VALUE OF \$70,000.00 IS REQUIRED AND SHALL BE POSTED WITH THE DISTRICT OFFICE PRIOR TO THE START OF WORK. THE BOND SHALL DISPLAY THE ENCROACHMENT AGREEMENT NUMBER.

This encroachment is approved subject to the Standard and Special Provisions which are attached to and made a part of the Encroachment Contract.

Sincerely,

Daniel T. Boulware, PE, District Engineer for B. H. Jones, PE, Division Engineer BHJ/dtb/mvs

cc: Mr. Daniel Boulware (w/ orig)
Town of Rolesville

Attachment

Mailing Address: NC DEPARTMENT OF TRANSPORTATION DIVISION 5 – DISTRICT 1 1575 MAIL SERVICE CENTER RALEIGH, NC 27699-1575 Telephone: (919) 814-6115 Fax: (919) 715-5778 Customer Service: 1-877-368-4968

Website: www.ncdot.gov

Location: 4009 DISTRICT DRIVE RALEIGH, NC 27607

Encroachment Special Provisions

- NCDOT WORK ZONE TRAFFIC CONTROL QUALIFICATIONS AND TRAINING PROGRAM:
 - A. Effective July 1, 2010, all flagging operations within NCDOT Right of Way require qualified and trained Work Zone Flaggers.
 - B. Effective July 1, 2011, qualified and trained Work Zone Traffic Control Supervisors will be required on Significant Projects.
 - C. Training for this certification is provided by NCDOT approved training sources and by private entities that have been pre-approved to train themselves. If you have questions, contact our web site at http://www.ncdot.org/doh/preconstruct/wztc/WZTCTrainingProgram/default.html, or contact Kenneth Thornewell, PE. with NCDOT Work Zone Traffic Control Unit at (919) 662-4338.
- 2. Before work begins, please forward the contact information of the general contractor to the Senior Assistant District Engineer, Montel Sparrow at mvsparrow@ncdot.gov. Include contact name, emergency phone number and email.
- 3. Current and future state projects take precedence over this encroachment.
- 4. The construction of the driveway(s) and associated roadway improvements shall be in accordance with Driveway Permit #D051-092-23-00187.
- 5. Roadway improvements shall be required as shown on the attached plans.
- 6. This encroachment agreement only covers work within NCDOT Right-of-Way as shown on the attached plans.
- 7. Before work begins, a preconstruction meeting with NCDOT shall be required. Please contact NCDOT 2 weeks prior to construction activities to set up this meeting by emailing Montel Sparrow, Senior Assistant District Engineer at mvsparrow@ncdot.gov.
- 8. Any personnel or equipment working within five feet of a travel lane shall require a full lane closure. No lane of traffic shall be closed or restricted between the hours of 6:00 AM to 9:00 AM and 4:00 PM to 7:00 PM Monday thru Friday, during any time of inclement weather, or upon District Engineers' directive. Traffic shall be maintained at all times. Any violation of these hours will result in termination of the Driveway Permit and liquidated damages in the amount of \$2,000.00 per hour or any portion thereof will be assessed by the District Engineers Office.
- 9. Nighttime and weekend operations will NOT be allowed unless written approval is received from the District Engineer. If nighttime or weekend work is allowed or required, all signs must be retro-reflective, and a work zone lighting plan must be submitted for approval prior to construction. If written approval is needed, please contact the District Office at 919-814-6115.
- 10. A \$70,000.00 Performance and Indemnity Bond shall be executed and posted with the District Office at 4009 District Drive, Raleigh, North Carolina 27607, prior to beginning any work on the Right of Way. When the project has been completed, and upon written request from the Permittee, the project will be inspected and reviewed by NCDOT. If all work is found to be satisfactory, the bond can be reduced and project will be placed under a one year warranty period. When the warranty period is completed, and upon written request by the Permittee to the District Office, final inspection and review will be performed by NCDOT. If all work is found to be satisfactory, the bond will be released.

E051-092-23-01269 Page 2 of 9

- 11. The encroacher shall submit a comprehensive traffic control plan to the District Engineer's office for approval prior to beginning work. Contact the District Office at (919) 814-6115.
- 12. Notify the District Office at 919-814-6115 a minimum of 48 hours prior to beginning work. Please provide name and contact number for the contractor in case of emergency.
- 13. Notify the Town of Rolesville before starting work.
- 14. Minimum pavement design on SR 2053 (Jones Dairy Road) shall be as follows:

For areas 6 feet wide or greater:

- 3 inches Bituminous Concrete Surface Course Type S9.5C (includes overlay)
- 4 inches Bituminous Concrete Binder Course Type I19.0C
- 10 inches Aggregate Base Course
- or for areas 6 feet wide or less:
- 3 inches Bituminous Concrete Surface Course Type S9.5C (includes overlay)
- 4 inches Bituminous Concrete Binder Course Type I19.0C
- 5 inches Bituminous Concrete Base Course Type B25.0C
- 15. A minimum 1.5 inch overlay of Type S9.5C asphalt shall be required as shown on the attached plans and in any areas of conflicting pavement markings. Mill a minimum of 1.5 inches to match existing pavement.
- 16. The edges of the existing asphalt shall be saw cut to provide a straight and uniform edge before paving to it. Diagonal joints will not be permitted.

Any existing attachments or paved shoulder that do not meet the required pavement design shall be removed before any widening begins. All edges shall be saw cut to provide a good longitudinal joint.

The width of the existing paved shoulder shall be included in addition to the required widening.

- 17. Pavement pre-markings shall be field reviewed and approved by the District Office before thermoplastic pavement markings are placed.
- 18. It shall be the responsibility of the Encroacher to address existing pavement failures prior to asphalt overlay. Please contact the District Office at (919) 814-6115 to mark areas to be addressed.
- 19. Any abandoned storm drainage pipe and associated structures shall be removed and properly backfilled.
- 20. Crossline installation shall have one passing subgrade density test performed prior to the replacement of asphalt for each half of the roadway crossing. Only one half of the roadway shall be open cut at one time in order to maintain traffic movements.
- 21. Pipes shall be installed as necessary to maintain existing drainage patterns. Pipes shall be sized properly to accommodate the drainage area at its point of discharge. Pipes shall be reinforced concrete pipe with a minimum inside diameter of 15 inches. Storm drain crosslines traversing the roadway shall be reinforced concrete pipe with a minimum inside diameter of 18 inches.
- 22. Existing driveway pipes affected by the proposed widening may need to be moved back from the edge of pavement to allow for a proper ditch and shoulder section, as per NCDOT Standards and Specifications.
- 23. Any catch basins and/or drop inlets that are to be converted to traffic bearing junction boxes shall include a manhole frame and cover. Blind boxes are not permitted within NCDOT Right-of-Way.

E051-092-23-01269 Page 3 of 9

- 24. All (cast-in-place and/or pre-cast) splice boxes, handholes, manholes, drainage structures and other appurtenances within NCDOT Right of Way shall be of a NCDOT approved design for traffic bearing, HS-20 loading, and shall be flush mounted. Manholes, handholes and vaults shall not be placed in the ditch-line, side slopes of the ditch or in the pavement.
 - All frames, grates, rings, covers, etc. are to be manufactured in accordance with the requirements of Section 106-1B "Domestic Steel". Foreign castings are not approved for use within NCDOT Right of Way.
- 25. All curb and gutter constructed in the right of way shall be 30 inch standard. Any curb and gutter removed shall be removed in full sections at existing joints. A compacted base shall be prepared prior to placement of curb and gutter; consisting of a minimum 10 inches of Aggregate Base Course or a minimum 5 inches of Asphalt Concrete Base Course, type B25.0C. Placement and compaction shall adhere to NCDOT Standards and Specifications.
- 26. The addition/replacement of curb and gutter next to an existing asphalt roadway shall be constructed as follows:
 - 1.) The edges of the existing asphalt shall be saw-cut to provide a straight and uniform edge for concrete to be placed along.
 - 2.) Mill minimum 1.5-in by 1.5-ft section at the edge of pavement along the proposed curb & gutter.
 - 3.) The contractor shall use an appropriate method to provide a straight, uniform front edge of concrete where tying to the ultimate top of pavement. This office recommends using a 2x4 laid flat and pinned along the milled surface at the edge of pavement. Concrete curb and gutter shall be constructed per NCDOT Standards and Specifications.
 - 4.) Once the concrete is poured and set, a minimum 1.5-inch lift of surface asphalt, type S9.5C, shall be placed in the milled section. It is the sole responsibility of the contractor to ensure that the proposed curb and gutter is placed at the correct height to provide proper drainage in addition to a smooth tie-in once asphalt is placed. Additional milling may be required to repair any damages done to the existing asphalt during construction.
 - For questions feel free to contact the District Engineer's Office at (919) 814-6115.
- 27. The Encroacher shall contact the District Engineer's office at (919) 814-6115 for inspection of forms or grade line prior to placing concrete for curb and gutter. The Encroacher shall provide 24 hours notification for inspections. All hoods, frames and grates intended for use in NCDOT Right of Way shall be onsite for inspection at this time or the curb and gutter inspection will not be conducted.
- 28. Sidewalks and curb cuts and ramps for disabled persons shall be constructed in accordance with the current NCDOT "Standard for Wheelchair Ramp Curb Cuts" and the Americans With Disabilities Act (ADA) Accessibility Guidelines for Buildings and Facilities. NCDOT shall not maintain the proposed sidewalks.
- 29. All Traffic signs moved shall be reinstalled as soon as possible to meet NCDOT specifications.
- 30. All pavement cuts, including those for the tapping for water services, sanitary sewer and/or gas line attachments, shall be made prior to paving the area to be widened/overlayed.
- 31. All bare and disturbed areas must have a sufficient stand of vegetation. Address any erosion issues that may arise during time of construction. Monitor these areas as needed to assure this requirement is satisfied.
- 32. The roadway shall be kept free of silt, mud and debris at all times.

E051-092-23-01269 Page 4 of 9

- 33. Pavement cores made to expose existing utilities shall be made with a maximum 18" pavement core. Pavement core locations shall not be placed in the wheel path whenever possible. Vacuum excavation shall be utilized to expose underground utilities. Pavement cores shall be repaired within the same working day. The pavement core shall be retained and reused to fill the core hole. The excavation shall be backfilled and compacted with select material to the bottom of the existing pavement structure or as indicated on the manufacturer's specifications. The retained core shall be placed in the hole and secured with a waterproof, mechanical joint. All materials must be listed on the NCDOT Approved.
 - Products List (APL) found here: https://apps.ncdot.gov/vendor/approvedproducts/ Prior to completion, ensure road surface is clean. If the pavement core is damaged and cannot be re-used, the core may be replaced with the surface mix, S9.5B. The asphalt patch shall match the thickness of the existing asphalt or four inches, whichever is greater. Prior to completion, ensure road surface is clean. If it becomes necessary for NCDOT to make any repairs associated with this encroachment, then the encroaching party will be charged for all costs associated with the repair.

E051-092-23-01269 Page 5 of 9

Encroachment Standard Provisions

- 1. Current and future state projects take precedence over this encroachment.
- 2. An executed copy of this encroachment agreement will be present at the construction site at all times during construction. NCDOT reserves the right to stop all work unless evidence of approval can be shown.
- 3. NCDOT reserves the right to revise, restrict, suspend and/or void this encroachment agreement if the execution and/or operation of said permit is found to be a hazard to the traveling public.
- 4. This encroachment agreement only covers work within NCDOT Right-of-Way. The encroacher is responsible for verifying all right of way. NCDOT does not guarantee the right of way on this road. If the right of way was not obtained by the fee simple method, it is the responsibility of the encroacher to obtain permission from the underlying property owner/owners.

Encroacher shall be responsible for obtaining all necessary permanent and/or temporary construction, drainage, utility and/or sight distance easements. All Right of Way and easements necessary for construction and maintenance shall be dedicated to NCDOT with proof of dedication furnished to the District Engineer prior to beginning work.

- 5. Notify the District Engineer's Office at (919) 814-6115 or at 4009 District Drive, Raleigh, NC 27607, prior to beginning and after completion of work.
- 6. The encroacher is responsible for any claim for damages brought by any property owner by reason of the installation.
- 7. The Encroacher shall notify the public, including all adjacent property owners and businesses, a minimum of 2 weeks prior to beginning work.
- 8. Any and all changes noted in red on the plans shall be incorporated into and made part of the approved permit.
- 9. The encroaching party shall comply with all applicable local, state and federal environmental regulations, and shall obtain all necessary state and federal environmental permits, including but not limited to, those related to sediment control, storm water, wetland, streams, endangered species, and historical sites.
- 10. All materials and construction shall be in accordance with NCDOT standards and specifications, including but not limited to, the NCDOT Standard Specifications for Roads and Structures 2024, the NCDOT Roadway Standards Drawings, and NCDOT Policies and Procedures for Accommodating Utilities on Highway Rights of Way.
- 11. The encroacher shall provide traffic control devices, lane closures, road closures, positive protection and/or any other warning or positive protection devices necessary for the safety of road users during construction and any subsequent maintenance. This shall be performed in conformance with the latest NCDOT Roadway Standard Drawings and Standard Specifications for Roads and Structures and Amendments or Supplements thereto. When there is no guidance provided in the Roadway Standard Drawings or Specifications, comply with the Manual on Uniform Traffic Control Devices for Streets and Highways and Amendments or Supplements thereto. No work shall be performed in the Right of Way unless this requirement is satisfied. NCDOT reserves the right to require a written traffic control plan for encroachment operations.

Sidewalk closures shall be installed as necessary. Pedestrian traffic shall be detoured around these closures and shall be signed appropriately and in accordance with The American with Disabilities Act Accessibility Guidelines.

12. No parking or material storage shall be allowed along the shoulders of any NCDOT roadways.

E051-092-23-01269 Page 6 of 9

- 13. Two-way traffic shall be maintained at all times.
- 14. No lane closures shall be permitted between the hours of 6:00 AM to 9:00 AM and 4:00 PM to 7:00 PM, Monday through Friday unless otherwise specified in the Special Provisions of this encroachment agreement.
- 15. At the end of each working day, equipment shall be parked outside of the clear recovery zone in order to avoid any obstruction to the travelling public. This clear recovery zone is measure from the edge of the nearest travel lane.
- 16. Work shall not be performed on both sides of the road simultaneously within the same area.
- 17. Ingress and egress shall be maintained to all businesses and dwellings at all times.
- 18. The paving of this roadway shall be in accordance with the revised NCDOT 2024 Standard Specifications, Sections 610, 1012 and 1020. The Contractor shall follow all procedures of the attached Quality Management System (QMS) for asphalt pavement Maintenance Version. The Contractor must adhere to all testing requirements and quality control requirements specified. The Contractor shall contact the NCDOT Division 5 QA Supervisor at (919) 562-0018 prior to producing plant mix and make the Supervisor aware that the mix is being produced for a future NCDOT road. Only NCDOT approved mix designs will be acceptable. A quality control plan shall be submitted to the District Engineer's Office prior to asphalt production. Use form QMS-MV1 for the Quality Control Plan submittal. Failing mixes and/or densities are subject to penalties including monetary payments or removal and replacement.
- 19. Roadway certification reports sealed by a Professional Engineer shall be submitted to the North Carolina Department of Transportation at 4009 District Drive, Raleigh, North Carolina, indicating the following:
 - * Pavement thickness by type
 - * Pavement density, core and/or test locations
 - * Base thickness
 - * Base density
 - * Subgrade density

Test frequency and method shall be in conformance with the NCDOT "Materials and Tests" Manual. Test must be performed by a Certified Technician including name and Certification number on report.

- 20. Any existing driveways, pavement, sidewalk, curb and gutter or drainage structures that are damaged during construction shall be repaired to their original condition.
- 21. When surface area in excess of one acre will be disturbed, the Encroacher shall submit a Sediment and Erosion Control Plan which has been approved by the appropriate regulatory agency or authority prior to beginning any work on the Right of Way. Failure to provide this information shall be grounds for suspension of operations.
- 22. All erosion control devices and measures shall be constructed, installed, maintained, and removed by the Encroacher in accordance with all applicable Federal, State, and Local laws, regulations, ordinances, and policies. All earth areas shall be regraded and seeded in accordance with NCDOT Standards Specifications for Roads and Structures 2012.
- 23. The applicant is responsible for identifying project impacts to waters of the United States (wetlands, intermittent streams, perennial streams and ponds) located within the NCDOT right-of-way. The discharge of dredged or fill material into waters of the United States requires authorization from the United States Army Corps of Engineers (USACE) and certification from the North Carolina Division of Water Quality (NCDWQ). The applicant is required to obtain pertinent permits or certification from these regulatory agencies if construction of the project impacts waters of the United States within the NCDOT right-of-way. Additional information can be obtained by contacting the USACE or NCDWQ.

E051-092-23-01269 Page 7 of 9

- 24. The applicant is responsible for avoiding impacts to federally protected species during project construction. Bald eagle, Michaux's sumac, smooth coneflower, dwarf wedgemussel, harperella, red-cockaded woodpecker and tar spinymussel are federally protected species that have been identified within NCDOT right-of-way in Durham, Person, Granville, Wake, Franklin, Vance, and Warren counties. Additional information can be obtained by contacting the North Carolina Natural Heritage Program or the United States Fish and Wildlife Services.
- 25. The applicant is responsible for complying with the Neuse and Tar-Pamlico Riparian Buffer Rule as regulated by the NCDWQ. The Rule regulates activity within a 50-foot buffer along perennial streams, intermittent streams and ponds. Additional information can be obtained by contacting the NCDWQ.
- 26. All proposed landscaping and plantings located within the NCDOT right of way shall be approved by the Division Roadside Environmental Unit. Contact Corey Sudderth at (919) 816-9290.
 - In the event these plants require relocation or removal for highway construction, reconstruction, or maintenance of safety, such removal or relocation will be done immediately by the permittee upon notification by the NCDOT entirely at the expense of the permittee.
- 27. Existing drainage patterns shall be maintained at all times throughout the proposed construction. The encroacher shall keep the roadway clean of dirt and debris at all times throughout the duration of the project.
- 28. Excavation within 1000 feet of a signalized intersection will require notification by the encroaching party to the Division Traffic Engineer at telephone number (919) 536-4000 no less than one week prior to beginning work. All traffic signal or detection cables must be located prior to excavation. Cost to replace or repair NCDOT signs, signals, pavement markings or associated equipment and facilities shall be the responsibility of the encroaching party. Any intersection modifications pertaining to signalization, lane additions/modifications, striping changes/additions, and or pedestrian accommodation installation/revision shall require a new signal plan of record be prepared, submitted and reviewed by the Division Traffic Engineer.
- 29. All temporary and final pavement markings, reflective pavement markings, raised pavement markers, non-cast iron snowplowable pavement markers and signage are the responsibility of the Encroacher. All final pavement markings shall be thermoplastic. Any pavement markings/markers that are damaged or obliterated shall be restored at no expense to NCDOT.
- 30. All Traffic signs moved shall be reinstalled as soon as possible to meet NCDOT specifications.
- 31. Strict compliance with the Policies and Procedures for Accommodating Utilities on Highway Right of Way manual shall be required.
- 32. It shall be the responsibility of the Encroacher to determine the location of other utilities within the encroachment area. The Encroacher shall be responsible for notifying other utility owners and providing protection and safeguards to prevent damage or interruption to existing facilities and to maintain accessibility to existing utilities.
- 33. All earth areas disturbed shall be regraded and reseeded in accordance with Division of Highways Standards and Specifications.
- 34. The Encroacher shall remove all trees, stumps and vegetative material from the right of way and dispose of in a licensed landfill or disposal site.
- 35. Excavated material shall not be placed on the roadway at any time.
- 36. Trenching, bore pits and/or other excavations shall not be left open or unsafe overnight. The Contractor shall comply with all OSHA requirements and provide a competent person on site to supervise excavation at all times.

E051-092-23-01269 Page 8 of 9

- 37. All excavations inside the theoretical 1:1 slope from the existing edge of pavement to the bottom of the nearest excavation wall should be made in accordance with the following conditions. Traffic should be moved to a travel lane outside the limits of a theoretical one-to-one slope from the bottom of the nearest trench wall to the pavement surface. Active excavation shoring, such as sheet piling, shall be installed. The design of the shoring shall include the effects of traffic loads. The shoring system shall be designed and sealed by an engineer registered in North Carolina. Trench boxes shall not be accepted as shoring. The trench backfill material should meet the Statewide Borrow Criteria.
- 38. Excavated areas adjacent to pavement having more than a 2 inch drop shall be made safe with a 6:1 or flatter slope and shall be designated by appropriate delineation during periods of construction inactivity, including, but not limited to, night and weekend hours.
- 39. Backfill material is to be placed at a maximum of 6 inch loose layers and each layer thoroughly compacted. All embankment backfill shall be compacted to 95% density and all subgrade to 100% density in accordance with AASHTO T-99 as modified by NCDOT. They shall be signed by a Professional Engineer and sent to the District Engineers Office at 4009 District Drive, Raleigh, NC 27607.
- 40. No commercial advertising shall be allowed within NCDOT Right of Way.
- 41. Guardrail shall be installed where warranted and in accordance with the guidelines shown in the 2024 Highway Design Branch Roadway Standard Drawings.
 - Guardrail removed or damaged during construction shall be replaced or repaired to their original condition.
- 42. Poles shall be located/relocated at or as near as possible to the right-of-way line, shall be set outside the Clear Recovery Area as outlined by AASHTO and outside sight distance triangles.
 - Poles located within guardrail sections shall be installed a minimum of 5 feet behind any guardrail. When applicable, poles shall be placed behind sidewalk.
 - Any associated guy wires to ground anchors and stub poles shall not be placed between a pole and the travel way and should be located outside the clear recovery area.
 - Minimum vertical clearance shall be 18' for aerial crossings over NCDOT roadways and 15'-6" for installations parallel to the roadway.
- 43. Fire Hydrants shall be of the break-away type. Hydrants shall be placed a maximum of one foot inside the right of way in ditch sections or a minimum of 6 feet behind the curb in curb and gutter sections.
- 44. Retaining walls or other vertical structures shall not be permitted inside NCDOT right of way.

E051-092-23-01269 Page 9 of 9

ROUTE	SR-2053	PROJECT	ONES DAIRY STORAGE FACILITY	COUNTY OF	WAKE OF NORTH CAROLINA
				: 	
DEP	ARTMENT OF TRANSPO	RTATION			ROACHMENT AGREEMENT FOR , PAVEMENT WIDENING AND
	-AND-			RM DRAINAGE	
RIVERCREST REA	ALTY ASSOCIATES, LLC		<u>_</u>		
8816 Six Forks Rd,	#201		-		
Raleigh, NC 27615					
	AGREEMENT, made and ortation, party of the first p			May , 202	4, by and between the Department
- · · · · · · · · · · · · · · · · · · ·		0			party of the second part,
-			WITNESSET		
Т	HAT WHEREAS, the party	of the second	part desires to encroa	ch on the right of	way of the public road designated as
Route(s)	SR-2053		, located	FROM THE INTERSECT	ION OF
SR-2053 (JONES	DAIRY ROAD) AND SR-4403 (WINTER SPR	ING DR) TO A POINT AF	PPROXIMATELY 650-LF EAST		
with the c	onstruction and/or erection	n of: APPROXIMATE	LY 700-LF OF ROAD WIDENING FOR TOW	N OF ROLESVILLE ULTIMATE F	RIGHT-OF-WAY CONSTRUCTION AND LEFT AND RIGHT TURN LAND
	OPMENTS SITE DRIVEWAY.				

WHEREAS, it is to the material advantage of the party of the second part to effect this encroachment, and the party of the first part in the exercise of authority conferred upon it by statute, is willing to permit the encroachment within the limits of the right of way as indicated, subject to the conditions of this agreement;

NOW, THEREFORE, IT IS AGREED that the party of the first part hereby grants to the party of the second part the right and privilege to make this encroachment as shown on attached plan sheet(s), specifications and special provisions which are made a part hereof upon the following conditions, to wit:

That the said party of the second part binds and obligates himself to install the encroaching facility in such safe and proper condition that it will not interfere with or endanger travel upon said highway.

That the party of the second part agrees to provide during construction proper signs, signal lights, flagmen and other warning devices for the protection of traffic in conformance with the latest <u>Manual on Uniform Traffic Control Devices for Streets and Highways</u> and Amendments or Supplements thereto. Information as to the above rules and regulations may be obtained from the Division Engineer of the party of the first part.

That the party of the second part hereby agrees to indemnify and save harmless the party of the first part from all damages and claims for damage that may arise by reason of the installation and maintenance of this encroachment.

It is clearly understood by the party of the second part that the party of the first part will assume no responsibility for any damage that may be caused to such facilities, within the highway rights of way limits, in carrying out its

That the party of the second part agrees to restore all areas disturbed during construction to the satisfaction of the Division Engineer of the party of the first part. The party of the second part agrees to exercise every reasonable precaution during construction and maintenance to prevent eroding of soil; silting or pollution of rivers, streams, lakes, reservoirs, other water impoundments, ground surfaces or other property; or pollution of the air. There shall be compliance with applicable rules and regulations of the North Carolina Division of Environmental Management, North Carolina Sedimentation Control Commission, and with ordinances and regulations of various counties, municipalities and other official agencies relating to pollution prevention and control. When any construction operation disturbs the ground surface and existing ground cover, the party of the second part agrees to remove and replace the sod or otherwise reestablish the grass cover to meet the satisfaction of the Division Engineer of the party of the first part.

That the party of the second part agrees to assume the actual cost of any inspection of the work considered to be necessary by the Division Engineer of the party of the first part.

That the party of the second part agrees to have available at the encroaching site, at all times during construction, a copy of this agreement showing evidence of approval by the party of the first part. The party of the first part reserves the right to stop all work unless evidence of approval can be shown.

Provided the work contained in this agreement is being performed on a completed highway open to traffic; the party of the second part agrees to give written notice to the Division Engineer of the party of the first part when all work contained herein has been completed. Unless specifically requested by the party of the first part, written notice of completion of work on highway projects under construction will not be required.

That in the case of noncompliance with the terms of this agreement by the party of the second part, the party of the first part reserves the right to stop all work until the facility has been brought into compliance or removed from the right of way at no cost to the party of the first part.

That it is agreed by both parties that this agreement shall become void if actual construction of the work contemplated herein is not begun within one (1) year from the date of authorization by the party of the first part unless written waiver is secured by the party of the second part from the party of the first part.

R/W (161B): Party of the Second Part certifies that this agreement is true and accurate copy of the form

FORM R/W 16.1B (Revised February 2021)

R/W (161B) incorporating all revisions to date.

IN WITNESS WHEREOF, each of the parties to this agreement has caused the same to be executed the day and year first above written.

DEPARTMENT OF TRANSPORTATION

DIVISION ENGINEER

INSTRUCTIONS

When the applicant is a corporation or a municipality, this agreement must have the corporate seal and be attested by the corporation secretary or by the empowered city official, unless a waiver of corporate seal and attestation by the secretary or by the empowered City official is on file in the Raleigh office of the State Utilities Manager. In the space provided in this agreement for execution, the name of the corporation or municipality shall be typed above the name, and title of all persons signing the agreement should be typed directly below their signature.

When the applicant is not a corporation, then his signature must be witnessed by one person. The address should be included in this agreement and the names of all persons signing the agreement should be typed directly below their signature.

This agreement must be accompanied, in the form of an attachment, by plans or drawings showing the following applicable information:

- All roadways and ramps.
- Right of way lines and where applicable, the control of access lines.
- 2. 3. 4. Location of the proposed encroachment.
- Length and type of encroachment.
- 5. Location by highway survey station number. If station number cannot be obtained, location should be shown by distance from some identifiable point, such as a bridge, road, intersection, etc. (To assist in preparation of the encroachment plan, the Department's roadway plans may be seen at the various Highway Division Offices, or at the Raleigh
- Drainage structures or bridges if affected by encroachment.
- 7. Typical section indicating the pavement design and width, and the slopes, widths and details for either a curb and gutter or a shoulder and ditch section, whichever is applicable.
- 8. Horizontal alignment indicating general curve data, where applicable.
- Vertical alignment indicated by percent grade, P.I. station and vertical curve length, where
- 10. Amount of material to be removed and/or placed on NCDOT right of way, if applicable.
- Cross-sections of all grading operations, indicating slope ratio and reference by station 11. where applicable.
- 12 All pertinent drainage structures proposed. Include all hydraulic data, pipe sizes, structure details and other related information.
- 13. Erosion and sediment control.
- Any special provisions or specifications as to the performance of the work or the method of construction that may be required by the Department must be shown on a separate sheet attached to encroachment agreement provided that such information cannot be shown on plans or drawings.
- The Department's Division Engineer should be given notice by the applicant prior to actual starting of installation included in this agreement.
- 16. Method of handling traffic during construction where applicable.
- Scale of plans, north arrow, etc.

SUMMARY OF PROBABLE CONSTRUCTION COSTS

Road Improvements (Non-Utility Related; 2-Party Road Widening)

Project Name: Jones Dairy Storage

Rolesville, NC

25-Jan-24

OFF-SITE IMPROVEMENTS	Quantity	Unit		Unit Cost	Total Cost
General Conditions					
General Conditions, Bonding & Layout	10	%	\$	2,641	\$ 26,408
Erosion Control					
Erosion Control	1	LS	\$	7,500	\$ 7,500
Traffic Control					
Flaggers and Barricades	15	Days	\$	1,500	\$ 22,500
Demolition					
Mill Tapers on Each End	2	EA	\$	1,500.00	\$ 3,000
Grading					
Earthwork (Cut & Fill)	500	CY	\$	10.00	\$ 5,000
Fine Grading	20000	SF	\$	1.00	\$ 20,000
Final Shoulder Backfill	1200	LF	\$	1.00	\$ 1,200
Site Improvements					
Asphalt Pvmt 1.5" Overlay	1760	SY	\$	25.00	\$ 44,000
Asphalt Pvmt NCDOT Full Depth Widening	870	SY	\$	128.00	\$ 111,360
Asphalt Pvmt Asphalt Side path	290	SY	\$	45.00	\$ 13,050
Curb & Gutter - 30"	280	LF	\$	24.00	\$ 6,720
Conc Driveway Aprons	100	SY	\$	85.00	\$ 8,500
Conc. ADA Ramps	2	EA	\$	2,500.00	\$ 5,000
15-in RCP Storm Sewer	50	LF	\$	75.00	\$ 3,750
15-in Conc. FES	4	EA	\$	500.00	\$ 2,000
Thermo Pavement Markings	1	EA	\$	2,500.00	\$ 2,500
Mailbox Relocations	1	LS	\$	1,000.00	\$ 1,000
Landscaping					
Seeding & Cleanup	20000	SF	\$	0.25	\$ 5,000
Allowances	2				
Unsuitable Soil Allowance SEAL	20	CY	\$	100	\$ 2,000
955780	K.				
Unsuitable Soil Allowance SEAL 055780		Subtotal	:		\$ 290,488
PRITE IN	Allinia 10%	Co	ntrac	tors OH&P	\$ 29,049
	5%			ontingency	14,524
F	ROAD IMPRO	VEMENT	COS	ST TOTAL:	\$ 334,061

Timmons Group's Opinion of Probable Construction Cost (OPCC) is based on best judgment, experience and being qualified professionals generally familiar with the construction industry. Because Timmons Group has no control over the cost of labor, materials, equipment, services furnished by others, or over competitive bidding or market conditions, Timmons Group cannot guarantee that actual construction costs will not vary from the OPCC presented.

VERIFICATION OF COMPLIANCE WITH **ENVIRONMENTAL REGULATIONS**

(Check Appropriate Box)

V	Permits from the N.C. Department of Environmental Quality and the U.S. Army Corp of Engineers are not required for this project. However, all applicable federal and state regulations have been followed.				
	The required permits from the N.C. Department of Environmental Quality and the U.S. Army Corp of Engineers have been obtained for this project. Copies of the permits are attached.				
V	All applicable NPI be met for this pr	DES Stormwater Permit r oject.	equirements have b	een or will	
V	₫ Ø	ompliance with all applic ws and regulations.	cable sedimentation	and	
Pro	ject Name:	Jones Dairy Storage Facility			
Tov	vnship:	Rolesville	County:	Wake	
Pro	ject Engineer:	Garrett Frank	Phone Number	919-866-4503	
Pro	ject Contact:	Garrett Frank			
App	olicant's Name:	Rivercrest Realty Associates,	LLC ANTERINGE	ECSERIO CANALLES SIGNAL	
Dat	e Submitted:	10/27/2023		-1-2342 SEAL / 165780	
For	m VCER-1			GINEE RATE	

Form VCER-1 June 2016

MA-22-09 CONDITIONS OF APPROVAL:

- DEVELOPMENT OF THE POPERTY SHALL BE IN SUBSTANTIAL CONFORMANCE WITH THE ACCOURANTNE ENHIGHT O CONCERT PLAN. I CONTROL SHOWN FOR COMMITTED LESIENTS INCLUDING, BUT ON THE POPERTY SHALL BE IN CONTROL SHOWN FOR CONTROL MASSURES SHOWN ON EXHIBIT OF LAKE CONCEPTUAL MAD PROVIDED FOR ILLUSTRATION AND CONTEXT ONLY. IN ALL CONCEPTUAL SHALL BE CHEENING AS SUBSECURED IS SHALES OF A POPERTY IN ALL CONTROL SHELL SHALL BE CHEENING AS SUBSECURED IS SHALES OF A POPERTY IN ALL CONTROL SHALL SHALL BE CHEENING AS SUBSECURED IN SHALES SHALL SHALL BE CHEENING AS SUBSECURED IN SHALL SHALL BE CHEENING AS THE SHALL SH
- EACLUSION STORM TO A SPECIAL OFFICE A SPECIAL PLANS IS USED AS DEVINED STORMAGE ASSESSION AS LOCATION OF A SPECIAL OFFICE ASSESSION AS DEVINED ASSESSION AS A SPECIAL OFFICE ASSESSION AS

- 10PALAND 6AM.
 ON EXTERIOR FAÇADES FACING A PUBLIC RIGHT-OF-JWAY OR EXTERIOR FAÇADES FACING PINS.
 1444706822. 1850604353, 1850606343, 1850606333, 1850607323, AND 1850608313. THE FOLLOWING MATERIALS ARE PROHIBITED: CINDERBLOCK, CONCRETE, METAL SIDING, AND
- VINYL SIDING, AND BRICK, EIFS BRICK, STONE, OR CONCRETE MASONRY COMPONENTS SHALL BE A MINIMUM OF

SITE PLAN REVIEW DRAWINGS: **FOR**

JONES DAIRY STORAGE FACILITY

1200 JONES DAIRY RD ROLESVILLE, NC 27587

SITE PLAN#: SDP-23-03 REZONING CASE #: MA-22-09 TEXT AMENDMENT CASE #: TA-22-02

APPROVED TA-22-02

Sheet List Table

COVER OVERALL SITE PLAN & KEY MA

EXISTING CONDITIONS & DEMOLITION PLAN

SITE PLAN

GRADING & DRAINAGE PLAN

SITE LITELITY PLAN

OFF-SITE UTILITY PLAN OFF-SITE UTILITY PLAN & PROFILE

FROSION & SEDIMENATION CONTROL PLAN- PH I

EROSION & SEDIMENATION CONTROL PLAN - PH II

SITE DETAILS

C0.0

C2.0

C3.0

C4 0

C4.1

C4.2

C5.0

C5.1

C6.0

C6.1

C6.2

C6.3 C6.4

C6.5

C6.6

C6.7

5.1.6.0.5.b. - 'INDUSTRIAL, LIGHT' PRINCÍPAL USE - SELF-SERVICE STORAGE SHALL BE CONTAÍNED WITHIN A FULLY ENCLOSED BUILDING AND CONTAÍNED IN A SINGLE BUILDING, ACCESS INTERNALL' EXCEPT IN THE GENERAL INDUSTRICT NOT LOCATED ON NAM IS 70 AS MAIN ST. 5.1.61.4.1. "VAREHOUSING" PRINCÍPAL USE - EXAMPLES. INCLUDES WAREHOUSES AND MINWARCHOUSES.

APPROVED ALTERNATE PARKING PLAN CONDITIONS

1 PARKING SPACE PER 100 SELF-STORAGE UNITS. PARKING PERMITTED BETWEEN A PROPOSED BUILDING AND THE STREET FRONTAGE.

ATTENTION CONTRACTORS

Failure to call for Impection, Install a Domistream Plug, has Permitted Plans on the Jobale, or any other Violation of City of Baleigh Standards will result to a Fine and Possible Evolution to lature work in the City of Raleigh.

PLAN - DEMO, SITE AND SIGNAGE & STRIPING PLANS

ROAD WIDENING PLAN - GRADING, DRAINAGE & FROSION CONTROL PLAN

JONES DAIRY ROAD - CROSS SECTIONS STA. 10+00-

JONES DAIRY ROAD - CROSS SECTIONS STA, 11+00

JONES DAIRY ROAD - CROSS SECTIONS STA. 12+60-

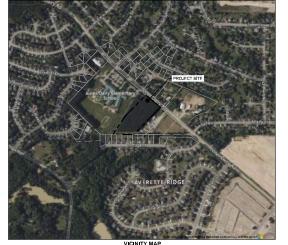
JONES DAIRY ROAD - CROSS SECTIONS STA, 13+00 -JONES DAIRY ROAD - CROSS SECTIONS STA. 14+00

IONES DAIRY ROAD - CROSS SECTIONS STA. 16+00 IONES DAIRY ROAD DRIVEWAY STEM PROFILE

Sheet List Table

SITE DATA TABLE

PROJECT NAME:	JONES DAIRY STORAGE FACILITY
SITE PLAN NUMBER	SDP-23-03
PROPERTY OWNER/ DEVELOPER:	ROLESVILLE (LOCKBOX) LLC 8816 SIX FORKS ROAD, SUITE 201 RALEIGH, NC 27615
REAL ESTATE ID:	0162697
PIN:	1850608722
DEED ACRES:	5.50 ACRES
PROPERTY ADDRESS:	1200 JONES DAIRY RD, ROLESVILLE NC 27587
PROPERTY ZONING:	GI-CZ
CURRENT USE:	VACANT
PROPOSED USE:	SELF STORAGE (COMMERCIAL)
DISTURBED AREA:	5.77 ACRES
CURRENT IMPERVIOUS:	0 ACRES
PROPOSED IMPERVIOUS:	2.92 ACRES
TREE SAVE SUMMARY:	and the state of t
REQUIRED TREE PRESERVATION:	10% SAVE = 0.10'4.76 (TREE AREA ON-SITE) = 0.476 ACRES
PROVIDED TREE PRESERVATION:	0.607 ACRES = 13%
PARKING SUMMARY:	
REQUIRED VEHICULAR SPACES:	1 SPACE PER 100 STORAGE UNITS = 540/100 = 6 SPACES
PROVIDED VEHICULAR SPACES:	7 SPACES
ADA PARKING SUMMARY:	
REQUIRED VEHICULAR SPACES:	1 SPACE
PROVIDED VEHICULAR SPACES:	1 SPACE
BUILDING HEIGHT:	MAXIMUM BUILDING HEIGHT: WITHOUT SPRINKLERS: 35-FT WITH SPRINKLERS: 60-FT PROVIDED BUILDING HEIGHTS: 16 FT
BUILDING SQUARE FOOTAGE(S):	CLIMATE CONTROLLED BUILDING: 62,610 SQFT STORAGE BUILDING(S): 1800 SQFT (2), 3800 SQFT (2), 8600 SQFT
NUMBER OF STORAGE UNITS:	TOTAL NUMBER OF UNITS 5-00 BALLONG A SAS & UNITS
BUILDING/STRUCTURE SETBACKS:	
FRONT:	30-FT
SIDE:	15-FT
REAR:	36-FT
LANDSCAPE BUFFERS:	
ADJACENT PROPERTY:	50' TYPE "4" BUFFER (SOUTH & WEST OF PARCEL)
ADJACENT PROPERTY:	SEE CONDITION 4 (EAST OF PARCEL)
WATERSHED:	NEUSE RIVER
RIVER BASIN:	SANFORD CREEK (NEUSE)
SURFACE WATER CLASSIFICATION:	NONE



PROJECT TEAM

LAND OWNER

CONTACT: ROLESVILLE (LOCKBOX) LLC 8816 SIX FORKS ROAD, SUITE 201 RALEIGH NC 27615

DEVELOPER

RIVERCREST REALTY INVESTORS CONTACT: BRIAN HOLDER PHONE: (919) 846-4046 8816 SIX FORKS ROAD, SUITE 201 RALEIGH, NC 27615

CIVIL ENGINEER

TIMMONS GROUP CONTACT: GARRETT FRANK, PE, PLA PHONE: (919) 866-4503 5410 TRINITY ROAD, SUITE 102 RALEIGH, NC 27607

Public Sewer Collection / Extension System

CITY OF RALEIGH - PLANS AUTHORIZED FOR CONSTRUCTION

ELECTRONIC APPROVAL THIS APPROVAL DE SIENDISSUED ELECTRONIC CAPPROVAL THIS APPROVAL DE SIENDISSUED ELECTRONICALLY THA APPROVAL IS VALID ONLY UPON THE SIGNATURE OF A CITY OF MELGIER REVIEW OF THE SIENDISSUE OF THIS APPROVAL VISIT APPROVAD DAMA, ANY VORK AUTHORIZED BY THIS APPROVAL VISIT ELECTRONIC APPROVAL ANY MOY BE CITY ON CESSION. ANY MODIFICATION TO THIS APPROVAL ONCE SISUED WILL INVALIDATE THIS APPROVAL.

CITY OF RALEIGH DEVELOPMENT APPROVAL RALEIGH WATER REVIEW OFFICER

EROSION CONTROL, STORMWATE AND FLOODPLAIN MANAGEMENT APPROVED
EROSION CONTROL
5-

STORMWATER MGMT. S-

FLOOD STUDY S-



08/01/ 04/05/2023

L. BARNES DESIGNED O G. FRANK CHECKED B G. FRANK

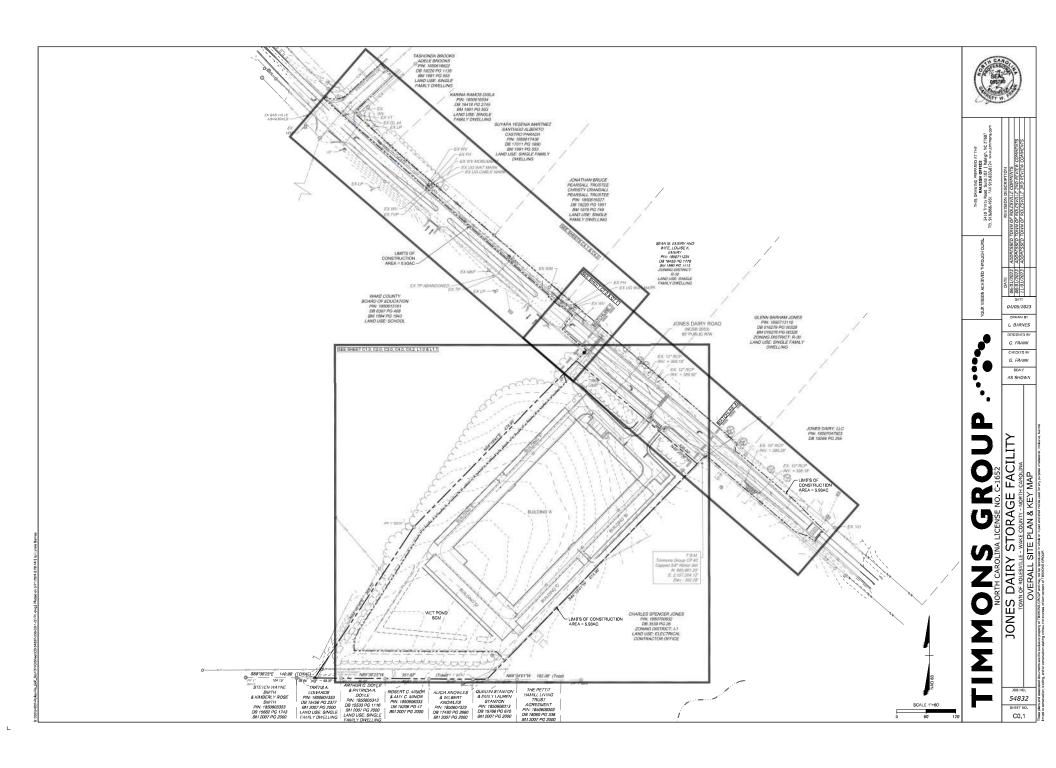
AS SHOWN

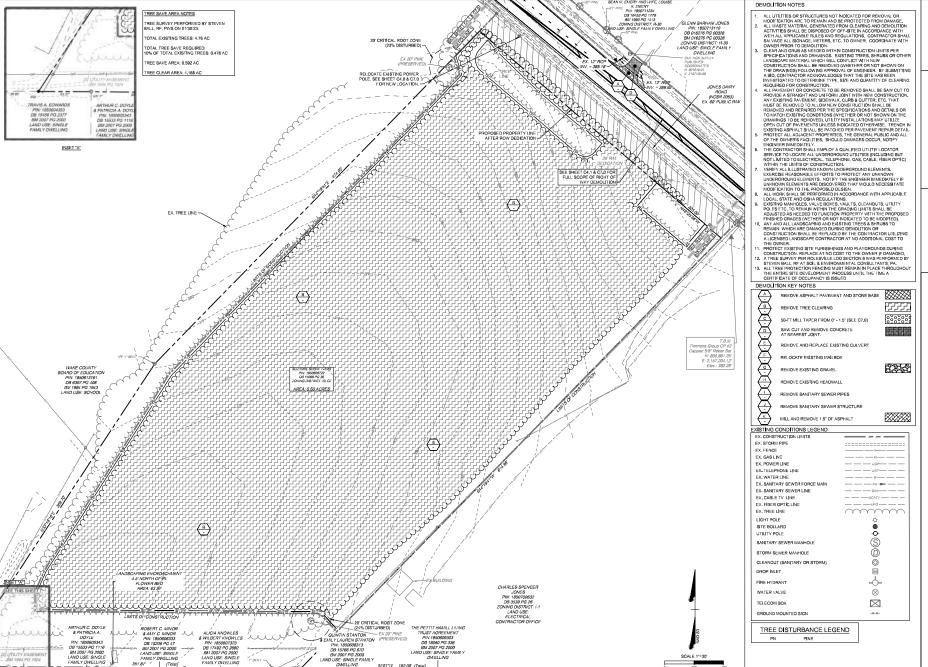
FACILITY

STORAGE I

DAIRY JONES L

54832 C0.0





34'01"W 192.08' (Total)



ADDRESSED TOWN OF ROLESVILLE COMMENTS
ADDRESSED TOWN OF ROLESVILLE ZNO REVIEW COMMENTS
ADDRESSED TOWN OF ROLESVILLE ZNO REVIEW COMMENTS DATE 06/01/2023 08/01/2023 11/01/2023

> 04/05/2023 L. BARNES DESIGNED OF

G. FRANK CHECKED BY G. FRANK SCALE

AS SHOWN

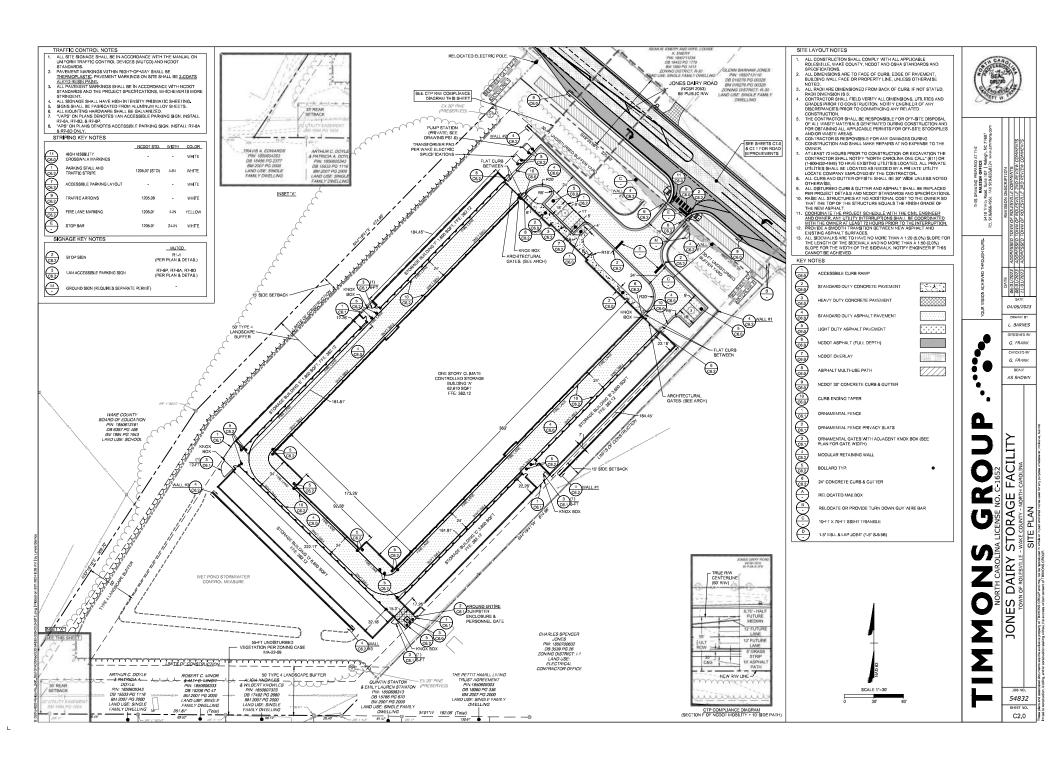
Ω

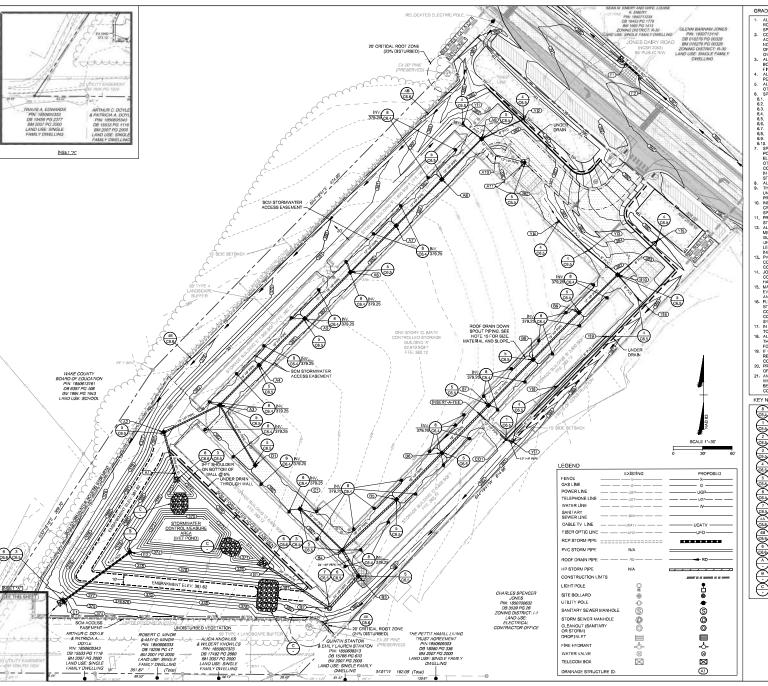
E FACILITY N PLAN

AJRY STORAGE FACT ROLESTILE - WANTE COUNTY - MOETH CAROLINA 3 CONDITIONS & DEMOLITION PI DAIRY EXISTING (

JONES [

54832 SHEET NO. C1.0





GRADING & DRAINAGE NOTES

- ALL CONSTRUCTION SHALL COMPLY WITH ALL APPLICABLE ROLESVILLVE, WAKE COUNTY, NCDOT AND OSHA STANDARDS AND SPECIFICATIONS.
- COORDINATE THE PROJECT SCHEDULE WITH THE OWNER, AND COORDINATE THE PROJECT SCHEDULE WITH THE OWNER, AND ADJACEN USERS OF THE PROPERTY. MAINTAIN TRAFFIC FLOW AND DO NOT INTERRUPT UTILITIES AROUND THE SITE. DO NO DISTURB OPERATIONS OF ADJACENT SITES AND FACILITIES ANDIOT THEIR OWNERS THE ADJACHULES STORD FACILITIES AND THEIR OWNERS THE ADJACHULES STORD THE STRUCTURES, VALVE
- ALLESIA NA VALLA S ANDROCA DA SANDROCA SA NEEDED TO MATCH
 HAISHED GRANG, S, CTL. SANL BE ADJUSTED AS NEEDED TO MATCH
 HAISHED GRANG, S, CTL. SANL BE ADJUSTED AS NEEDED TO MATCH
 HAISHED GRANG, S, CTL. SANL BE ADJUSTED AS NEEDED TO MARCH
 ALL BACCHILL, COMPACTION, SOILS TESTING, ETC. SHALL BE
 PERCENCE OF THE OWNERS NORPHONEMENT TO THE OWNERS NOTED
 SPOT GRADE ASSENSATION SOILS

 1. TO TOP OF CURB
 1. THE COMPACT OF THE MATCH TOP OF CURB UNLESS NOTED
 SOILS THE COMPACT OF THE MATCH TOP OWNERS
 1. THE COMPACT OF THE MATCH TOP OWNERS
 1. THE ES, CLEANOUTS, ETC. SHALL BE ADJUSTED AS NEEDED TO MATCH

- 3.8. T.L. FLOW LINE
 3. GEO SCHOOL
 3.0. GEO
 3. GEO
 3.0. G

- ALL RUDE UNLESS INDICATED OTHERWISE. ALL ROOF DRAINS FROM BUILDING TS. (C. AND TO, SHALL BE 8" PVC (SCH 40) BT. NEWS MIN. SLOPE UNLESS INDICATED OTHERWISE. USE DOUTLE IRON WHEN SLOPE UNLESS INDICATED OTHERWISE. USE DUTLE IRON WHEN COVER IS LESS THAN 24-IN. MATCH PIPE CROWNS WITH CONNECTION TO DROP
- INLET.

 13. PVC ROOF DRAIN PIPING UNDER PAVEMENT SHALL HAVE 24-IN MINIMUM
- COVER, IF ROOF DRAIN PIPING UNDER PAVEMENT HAS LESS THAN 24-IN COVER, ROOF DRAIN PIPING SHALL BE 8" OIP (IN LIEU OF PVC). JOINT FILL AND CAULK EACH CONCRETE EXPANSION JOINT AND WHERE CONCRETE PAVEMENT ABUTS OTHER PAVEMENTS, SIDEWALKS, OR HARD SUBFACEST.
- HARD SUIFACES.

 I. MAINTAIN ALL PROSION CONTROL DEVICES AFTER EACH RAINFALL EVENT IN ACCORDANCE WITH NODEO LAND QUALITY REQUIREMENTS AND AS BIRECTED BY THE NODEO, AND CHILD WITH REQUIREMENTS AND AS BIRECTED BY THE NODEO, AND CHILD WITH ALL SEQUENCY COUNTY OF STRUCTURES OF SUIL CHILD WITH SIZE ARBILLATION AND IT HE END OF STRUCTURES OF SUIL CHILD WITH SIZE ARBILLATION AND IT HE END OF CONSTRUCTION TO JOINT ABOUT AN PROPER FUNCTIONING OF THE DRAINAGE SYSTEM.

- CONSTRUCTION TO MAINTAIN PROPER FUNCTIONING OF THE DRAINAGE
 SYSTEM.

 17. IN DISTURBED PARS, AMEND THE TOP 8 INCHES OF LAWN AREAS WITH
 18. IN STRUMBED PARS, AREND THE TOP 8 INCHES OF LAWN AREAS WITH
 18. ALL SIDEWANS ARE TO HAVE NO MORE THAN A 120 GMB, SLOPE FOR
 THE MEDITATION OF THE SIDEWANS AND NO MORE THAN A 120 GMB, SLOPE
 FOR THE MIDIT OF THE SIDEWANG
 19. IF CONTRACTOR MOTIONES AND DISCRETE IN ANY OF THESE SLOPE
 REQUIREMENT, IT IS THE CONTRACTOR'S RESEPONSIBILITY TO
 19. IN CONTRACTOR MOTIONS AND DISCRETANCE IN ANY OF THESE SLOPE
 REQUIREMENT IN IS THE CONTRACTOR'S RESEPONSIBILITY TO
 19. IN CONTRACTOR MOTION OF THE CONTRACTOR AND THE CONTRACTOR THE CONTRACTO

KEY NOTES

DOWNSPOUT BOOT CONNECTOR STORM SEWER CLEANOUT C8.5 STORM DRAINAGE TRENCHES

FLARED END SECTION

YARD INLET

CONCRETE DROP INLET

STORM SEWER MANHOLE

STORM SEWER MANHOLE COVER SILT FENCE (COMBINATION FENCE; SEE TREE FENCE DETAIL; SEE EROSION CONTROL PLAN)

TREE PROTECTION FENCE (SEE EROSION CONTROL PLAN) RIP RAP OUTLET PROTECTION (SEE EROSION CONTROL PLAN)

PROVIDE SMOOTH PAVING TRANSITION

CONCRETE RISER STRUCTURE (SEE SHEET C3.3)

CLASS B RIP RAP, SEE SCM DETAIL SHEETS.

DATE 06/01/2023 08/01/2023 11/01/2023

04/05/2023 L. BARNES

DESIGNED OF G. FRANK CHECKED B

G. FRANK SCALE

AS SHOWN

FACILITY PLAN

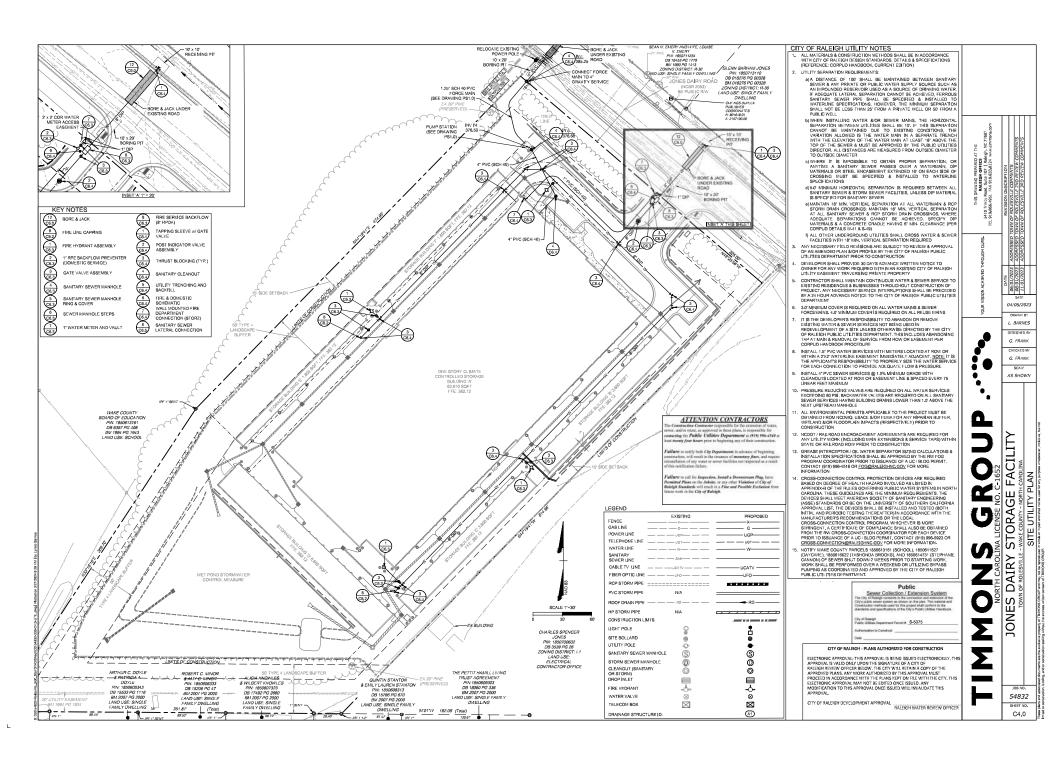
STORAGE F

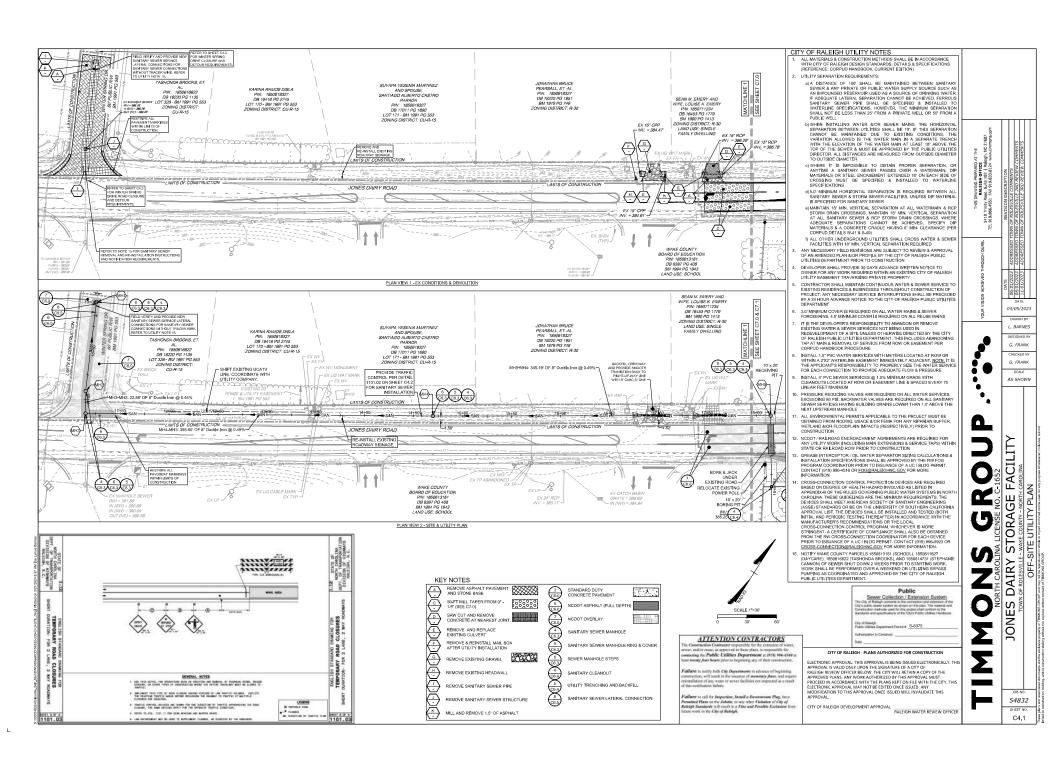
- WAKE COUNTY - NORTH C

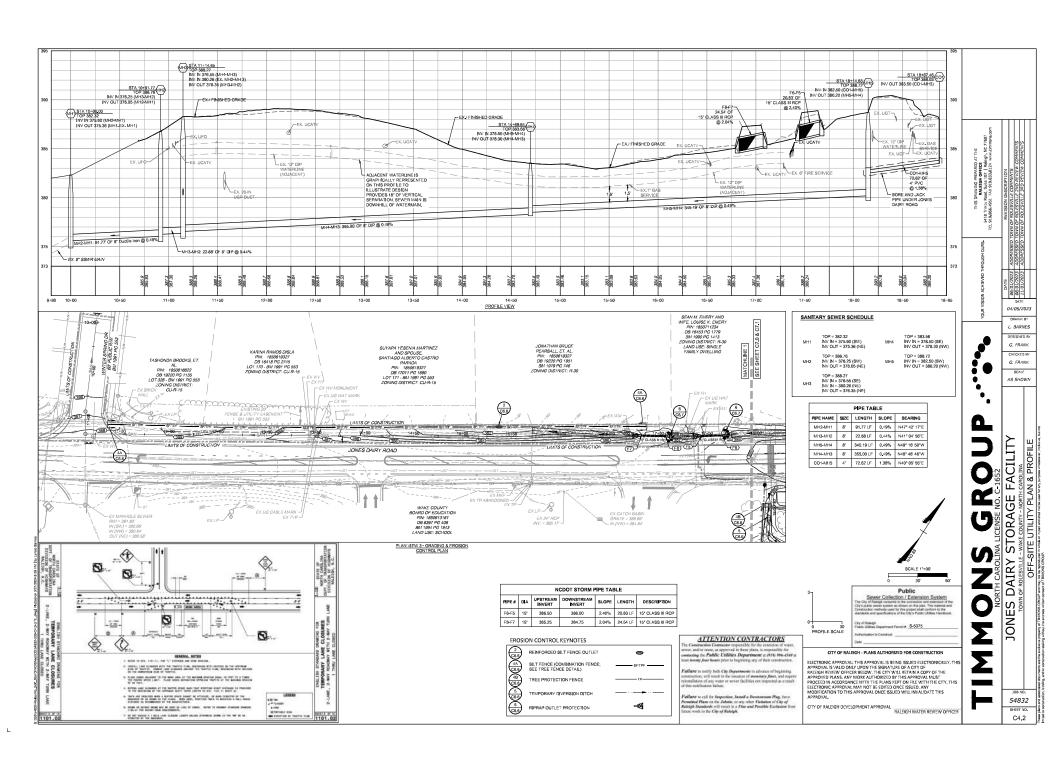
& DRAINAGE PLA DAIRY

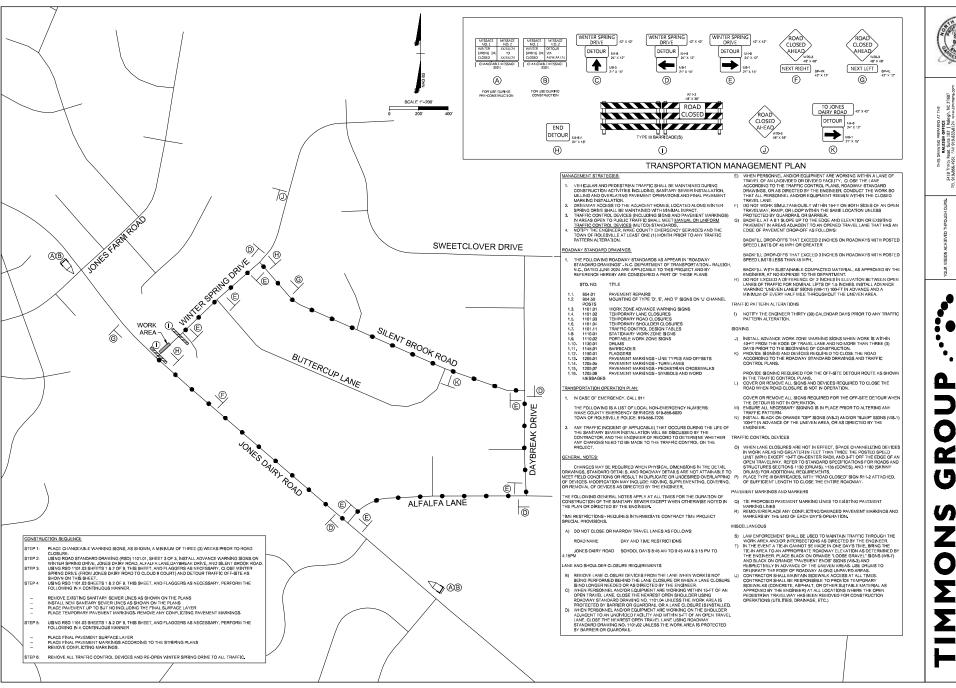
JONES L

54832 C3.0











5410 Trinity Ros 919-866-4951

06/01/2023 08/01/2023 11/01/2023

04/05/2023 L. BARNES

DESIGNED B G. FRANK CHECKED BY G. FRANK SCALE

AS SHOWN

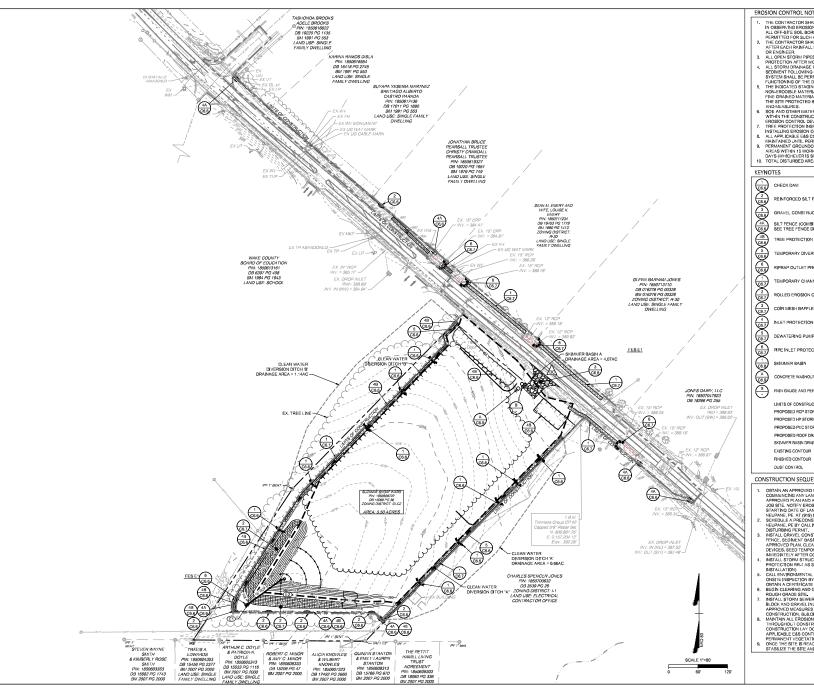
FACILITY

STORAGE F.

- WAKE COUNTY - NORTH CA
TE DETOUR PLAN OFF-SITE I DAIRY 1 OF ROLESVILLE

JONES I

54832 C4.3



FROSION CONTROL NOTES

- WITHIN THE CONSTRUCTION LIMITS PROTECTED BY SEQUENT AND RESPONDED CONTINUE DEVICES AND MEASURES.

 NETALLING ERROSEN CONTROL DEVICES.

 ALL APPLICABLE SES CONTROL DEVICES.

 APPLICABLE SES CONTROL DEVICES.

CHECK DAM _ REINFORCED SILT FENCE OUTLET GRAVEL CONSTRUCTION ENTRANCE CAO SILT FENCE (COMBINATION FENCE; SEE TREE FENCE DETAIL) TREE PROTECTION FENCE TEMPORARY DIVERSION DITCH -≪ RIPRAP OUTLET PROTECTION TEMPORARY CHANNEL LINING ROLLED EROSION CONTROL PRODUCT COIR MESH BAFFLES

DEWATERING PUMP WITH SILT BAG

盐

짱

,,,,,,

- an-

<u>68</u>

PIPE INLET PROTECTION SKIMMER BASIN

CONCRETE WASHOUT AREA

RAIN GAUGE AND PERMIT RECORDS BOX

LIMITS OF CONSTRUCTION PROPOSED RCP STORM SEWER PIPES PROPOSED HP STORM SEWER PIPES PROPOSED PVC STORM SEWER PIPES

PROPOSED ROOF DRAINS SKIMMER BASIN DRAINAGE AREA EXISTING CONTOUR

FINISHED CONTOUR DUST CONTROL

CONSTRUCTION SEQUENCE - PH I

- OBTAIN AN APPROVED PLAN AND APPLICABLE PERMIT PRIOR TO COMMISCIONE AND APPLICABLE PERMIT PRIOR TO COMMISCIONE AND APPLICABLE PERMIT PRIOR TO COMMISCIONE AND APPLICABLE AND
- DISTURBING FERVIT.

 IN STALL GRANE CONSTRUCTION PAD, TEMPORARY DISTRIBUTION, SILT FENCE, EDIDMENT MASHIS OR OF HER MEASURES AS SHOWN ON THE FENCE, EDIDMENT MASHIS OR OF HER MEASURES AS SHOWN ON THE DEVICES, SEED THAT WASHINGTON, THE DEVICES, SEED THAT WASHINGTON, THE MEASURES AND BASINS IMMEDIATELY AFTER CONSTRUCTION.

 INSTALL STORM STRUCTURES SEED WITH HIS PARP QUITE OF SHAMED AS SHOWN ON PLANS, PRIOR TO SKIMMED AS SHOWN ON THE STALL AND PRIOR THE CONSTRUCTION BY THE ENVIRONMENTAL CONSULTANT TO SHOW THE SHAMED AS SHOWN ON THE SHAMED AS S

- BEGIN CLEARING AND GRUBBING, MAINTAIN DEVICES AS NEEDED.
- BEGIN CLEARING AND GRUSSING, MINITAIN DEVICES AS NEEDED, NOUGH GRAZE SAMER, IES HOWN, AND PROTECT INLETS WITH BLOCK AND GRAVEL NIET CONTROLS, SEDIMENT TRAPS OR OTHER APPROVED MEASURES AS SHOWN ON THE PLAN. BEGIN CONSTRUCTION, BULLDING, ETC.
 MANTAIN ALL EROSION & SEDIMENT CONTROL MEASURES
- MAN IAN ALL EROSION & SEMMENT CONTROL INC. IN ASSURES TO THE ASSURES AND AN ARTHUR AND AN ARTHUR AND AN ARTHUR AND ARTHUR



ADDRESSED TOWN OF ROLESVILLE COMMENTS
ADDRESSED TOWN OF ROLESVILLE 2ND REVIEW COMMENTS
ADDRESSED TOWN OF ROLESVILLE 3RD REVIEW COMMENTS DATE 06/01/2023 08/01/2023 11/01/2023

04/05/2023 L. BARNES

DESIGNED OF G. FRANK CHECKED B G. FRANK

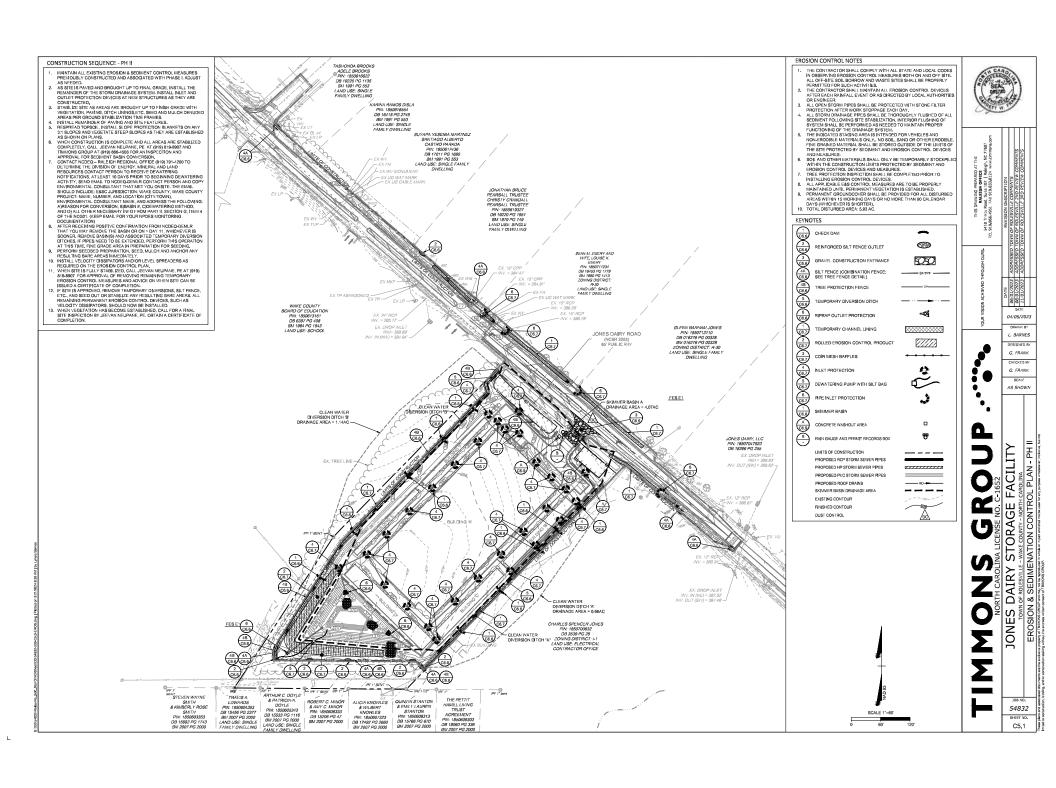
SCALE

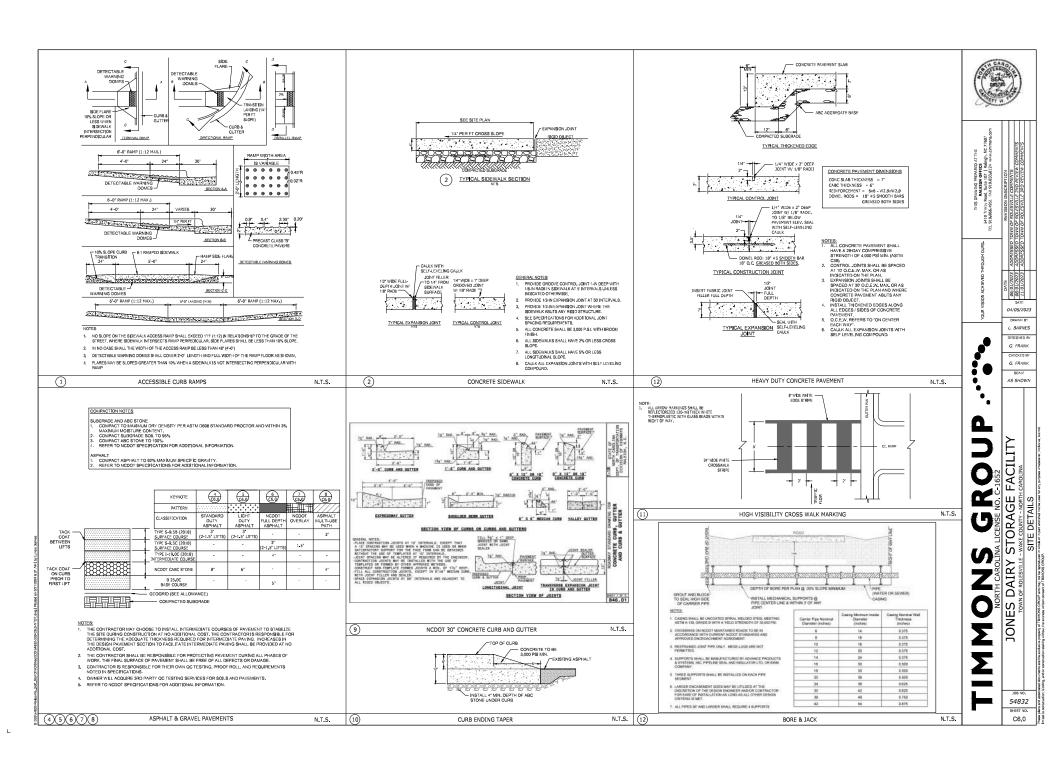
AS SHOWN

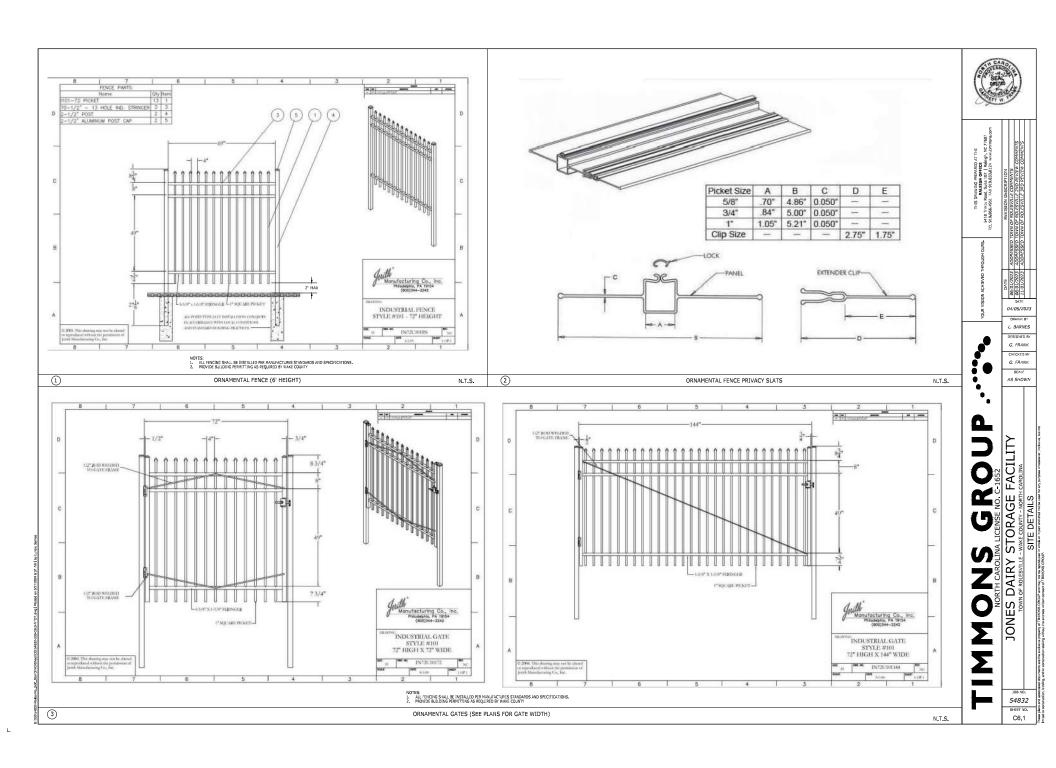
Δ

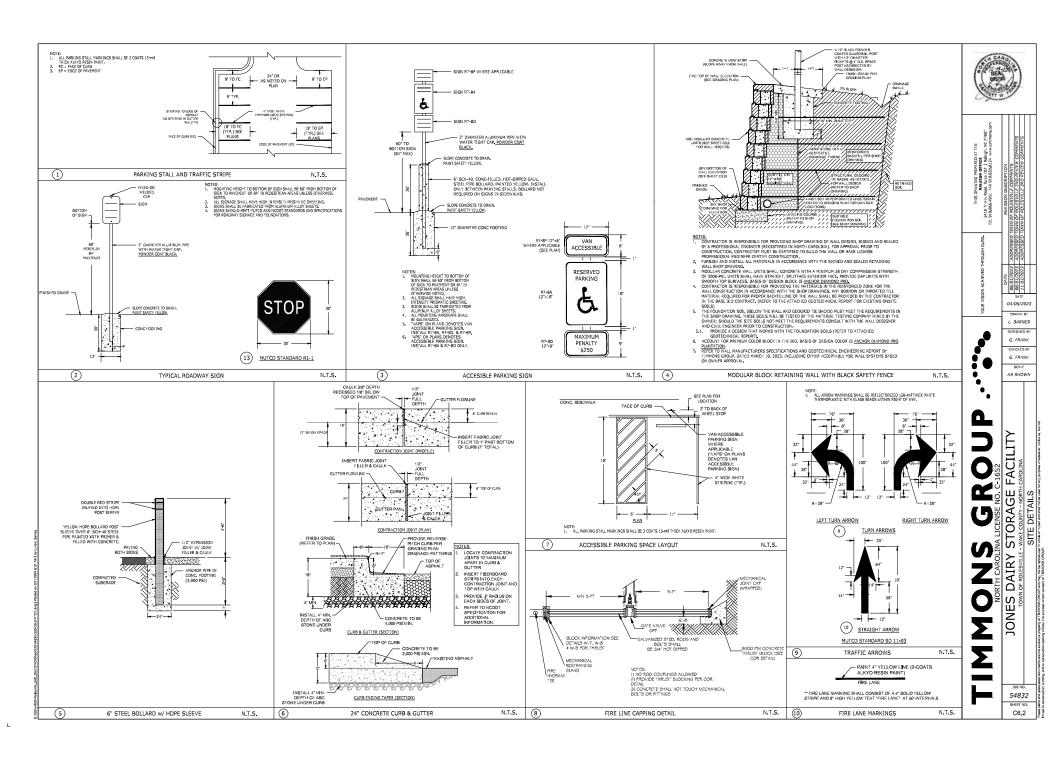
JONES DAIRY STORAGE FACILITY
TOWN OF ROLESTILE - WAKE COURTY- MORTH CAROLINA
EROSION & SEDIMENATION CONTROL PLAN - PH 1
9 department of the person of the pe

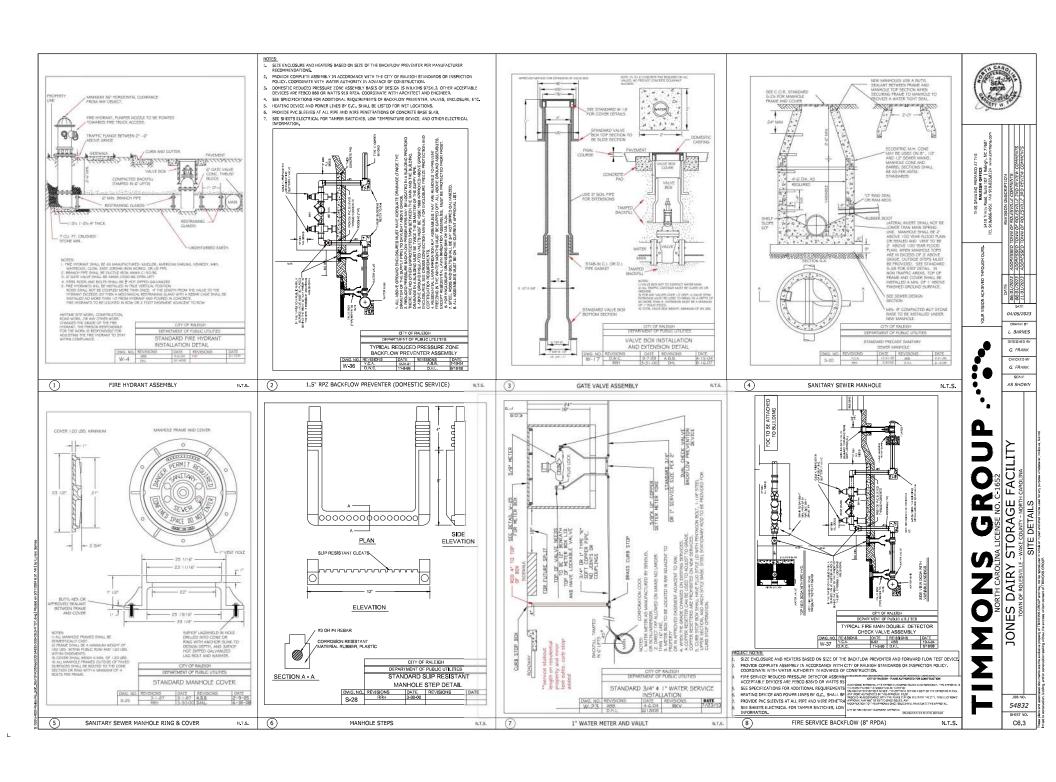
54832 SHEET NO. C5.0

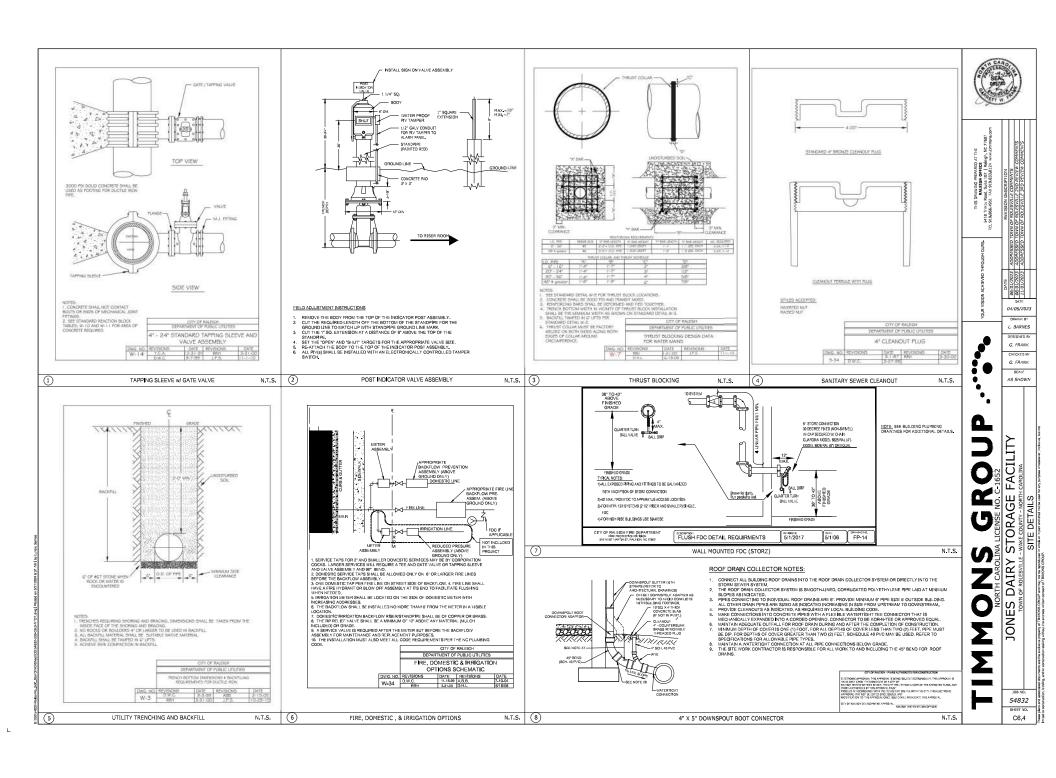


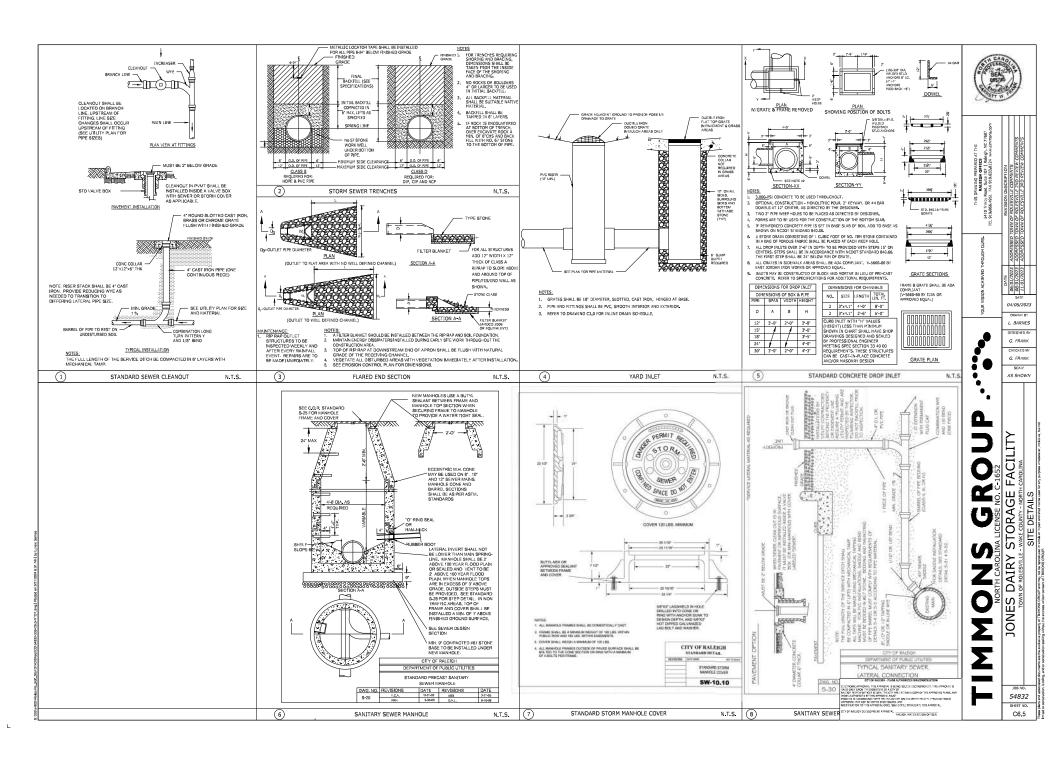


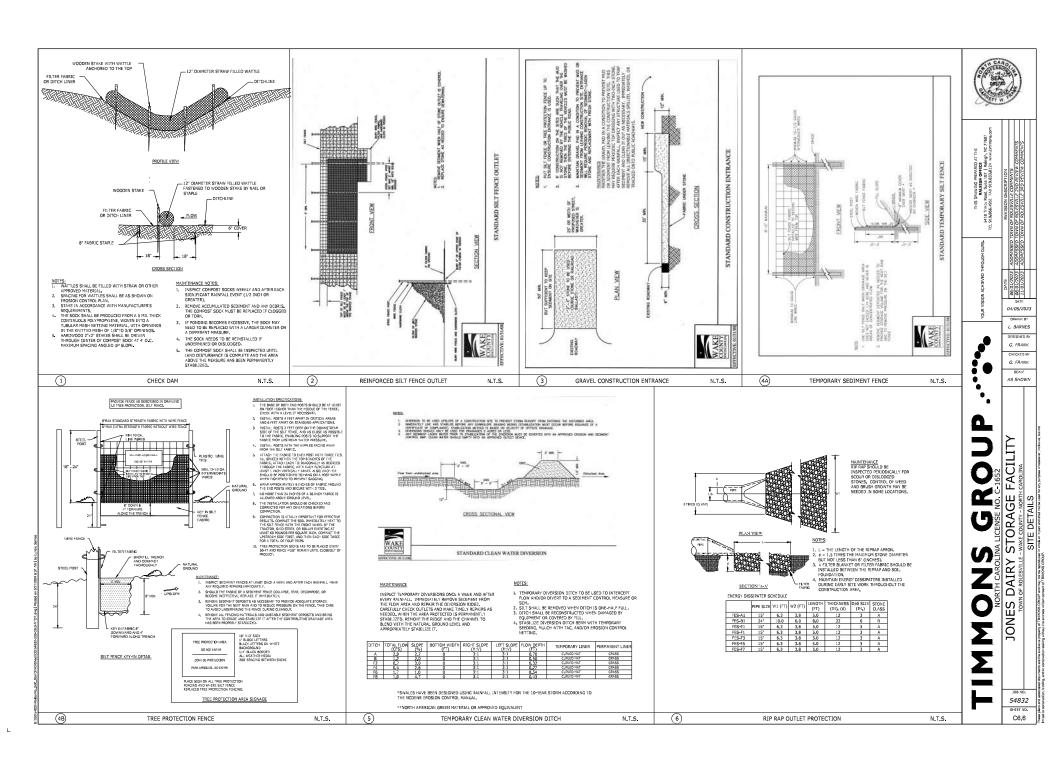


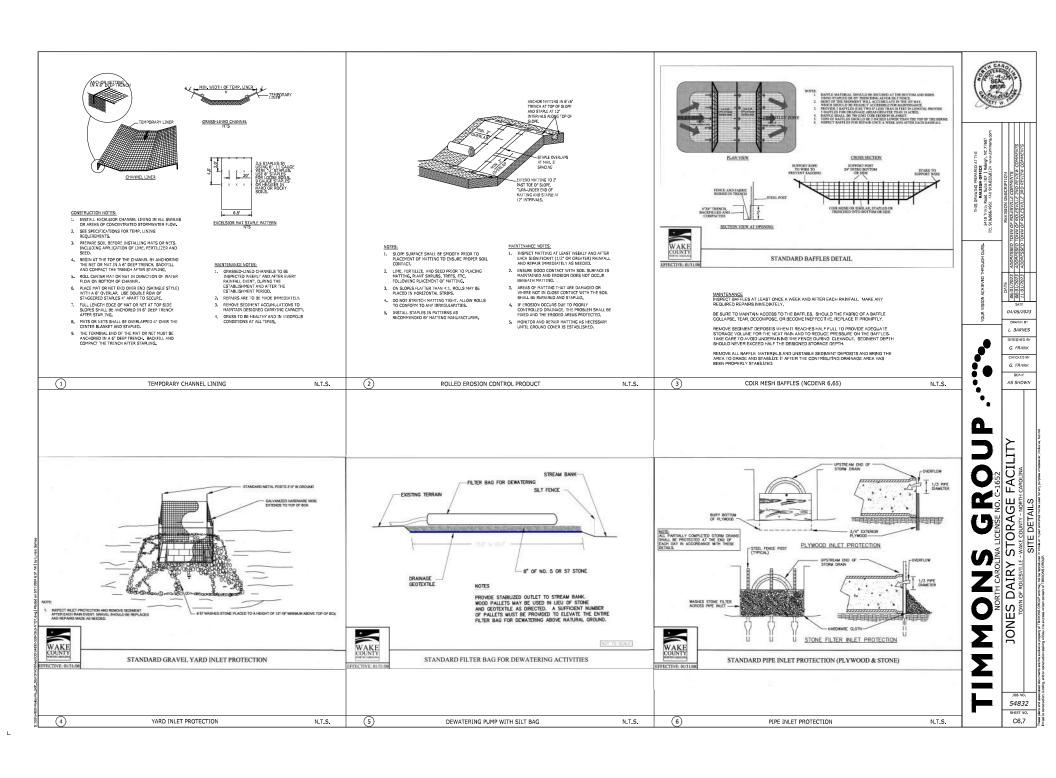


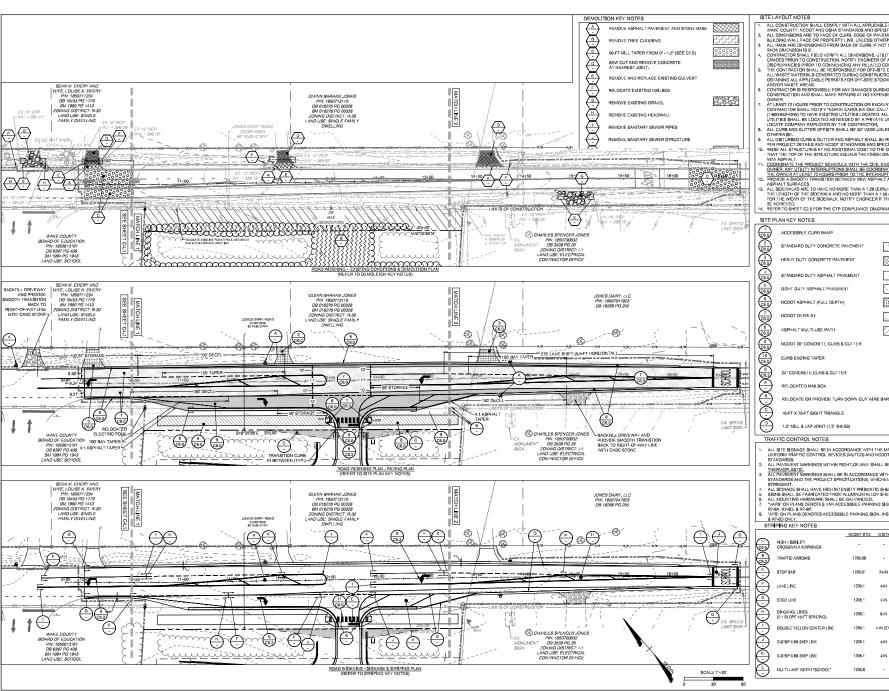












SITE LAYOUT NOTES

- ALL CONSTRUCTION SHALL COMPLY WITH ALL APPLICABLE ROLESVILLE, WAKE COUNTY, NCDOT AND OSHA STANDARDS AND SPECIFICATIONS.

- WARE COUNTY, NODOTAIN DISHA STANDARDS AND SPECIFICATIONS.
 ALL DIMENSIONS ARE TO PACE OF CIPIE, BODGE OF PRACTICATIONS.
 BULDING WALL FACE OF PROPERTY LINE; DILLESS OTHERWISE NOTED.
 BULDING WALL FACE OF PROPERTY LINE; DILLESS OTHERWISE NOTED.
 BULDING WALL FACE OF PROPERTY ALL DIMENSIONS, UTILITIES AND
 CONTRACTOR SHALL FEELD VERBEY ALL DIMENSIONS, UTILITIES AND
 DISCORPINACION CONSTRUCTION LOTTE FENDINGER OF STATED,
 DISCORPINACION CONTRACTOR SHALL FEELD VERBEY AND THE MORROW FOR THE STOCKPILES
 AND THE MORROW FOR THE MORROW FOR THE STOCKPILES
 AND THE MORROW FOR THE MORROW FOR THE STOCKPILES
 AND THE MORROW FOR THE MORROW FOR THE STOCKPILES
 AND THE MORROW FOR THE MORROW FOR THE STOCKPILES
 AND THE MORROW FOR THE MORROW FOR THE STOCKPILES
 AND THE MORROW FOR THE MORROW FOR THE STOCKPILES
 AND THE MORROW FOR THE MORROW FOR THE STOCKPILES
 AND THE MORROW FOR THE MORROW FOR THE STOCKPILES
 AND THE MORROW FOR THE MORROW FOR THE STOCKPILES
 AND THE MORROW FOR THE MORROW FOR THE STOCKPILES
 AND THE MORROW FOR THE MORROW FOR THE STOCKPILES
 AND THE MORROW FOR THE MORROW FOR THE STOCKPILES
 AND THE MORROW FOR THE MORROW FOR THE STOCKPILES
 AND THE MORROW FOR THE MORROW FOR THE STOCKPILES
 AND THE MORROW FOR THE MORROW FOR THE STOCKPILES
 AND THE MORROW FOR TH
- CONTRACTOR IS RESPONSIBLE FOR ANY DAMAGES DURING CONSTRUCTION AND SHALL MAKE REPAIRS AT NO EXPENSE TO THE
- OWNER.
 AT LEAST 72 HOURS PRIOR TO CONSTRUCTION OF EXCAVATION THE
 COVIRACTOR SHALL NOTIFY "NORTH CARROLING ONE CALL" (811) OR
 (*480-633-409) TO HAVE EXISTING UTILITIES LOCATE D. ALL PRIVATE
 LITLITES SHALL BE LOCATED AS NEEDED BY A PRIVATE LITLITY
 LOCATE COMPANY CHEN CYCLED BY HE CONTRACTOR.
 ALL CLIRE AND GUTTER OFFSITE SHALL BE 30" WIDE UNLESS NOTED
 OTHERWISE.
- OTHERWISE. ALL DISTURBED CURB & GUTTER AND ASPHALT SHALL BE REPLACED PER PROJECT DETAILS AND NODOT STANDARDS AND SPECIFICATIONS
- PER PROJECT DETAILS AND NOODT STANDARDS AND SPECIFICATIONS. RAISE ALL STRUCTURES AT NO ADDITIONAL COST TO THE OWNER SO THAT THE TOP OF THE STRUCTURE EQUALS THE FINISH, GRADE OF THE NEW ASPHALT. COORDINATE THE PROJECT SCHEDULE WITH THE CIVIL ENGINEER AND OWNER. ANY UTILITY INTERRUPTIONS SHALL BE COOPERINATED WITH THE OWNER AT LEAST 72 HOURS PRIOR TO THE INTERRUPTION. PROVIDE A SHOOTH TRANSITION BETWEEN THE WAS THAT AND EXISTING PROVIDE A SHOOTH TRANSITION BETWEEN THE WAS THAT LAND EXISTING THE OWNER AT LEAST 72 HOURS PRIOR TO THE INTERRUPTION.

- PROVIDE A SMOOTH TRANSHIPS SET THE ASPHALT SUPPACES.
 ALL SIDEWALKS ARE TO HAVE NO MORE THAN A 1:20 (5.0%) SLOPE FOR THE LENGTH OF THE SIDEWALK AND NO MORE THAN A 1:50 (2.0%) SLOPE FOR THE WIDTH OF THE SIDEWALK NOTIFY ENGINEER IF THIS CANNOT OF ADMICTS.
- BE ACHIEVED. REFER TO SHEET C2.0 FOR THE CTP COMPLIANCE DIAGRAM.



TRAFFIC CONTROL NOTES

10-FT X 70-FT SIGHT TRIANGLE

- ALL SITE SIGNAGE SHALL BE IN ACCORDANCE WITH THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (MUTCD) AND NODOT STANDARDS.
- PALE FAVEMENT MARKINGS WITHIN RIGHT-OF-WAY SHALL BE THERWOPLASTIC. ALL PAVEMENT MARKINGS SHALL BE IN ACCORDANCE WITH NODOT STANDARDS AND THE PROJECT SPECIFICATIONS, WHICHEVER IS MOR STRINGENT.
- STRINGENT. ALL SIGNAGE SHALL HAVE HIGH INTENSITY PRISMATIC SHEETING. ALL SIGNACE SHALL HAVE HICH INTENSITY PRISMATIG SHEETING. SIGNS SHALL BE FABRICATED FROM ALLIMINUM ALLOY S-EETS. ALL MOUNTING HARDWARE SHALL BE GALVANIZED. "VARS" ON FLAND SIENOTES AN ACCESSIBLE PARKING SIGN. INSTALL R.P.B., R.P.B.), & R.P.B.P. "APS" ON PLANS DENOTES ACCESSIBLE PARKING SIGN. INSTALL R7-BA

STRIPING KEY NOTES

		NCDOT STD.	WIDTH	COLOR
)	HIGH VISIBILITY CROSSWALK MARKINGS	-	-	WHITE
)	TRAFFIC ARROWS	1205.08	-	WHITE
)	STOP BAR	1205.01	24-IN	WHITE
)	LANE LÍNE	1205.1	4-IN	WHITE
)	EDGE LINE	1205.1	4-IN	WHITE
)	DIAGANOL LINES (2:1 SLOPE 45-FT SPACING)	1205.1	8-IN	YELLOW
)	DOUBLE YELLOW CENTER LINE	1206.1	4-IN (EA)	YELLOW
)	2-6/SP MINI SKIP LINE	1205.1	4-IN	WHITE
)	3'-9'/SP MINI SKIP LINE	1205.1	4-IN	WHITE
)	MULTI-LANE WIDTH "SCHOOL"	1205.8	-	WHITE



5410 Trin 919.866

DATE 06/01/2023 08/01/2023 11/01/2023

04/05/2023 L. BARNES

DESIGNED B G. FRANK CHECKED BY G. FRANK

SCALE AS SHOWN

PLANS

STRIPING FACILITY ু ∞

STORAGE FAC.

- - WAKE COUNTY - NORTH CAROLINA

- , SITE AND SIGNAGE &

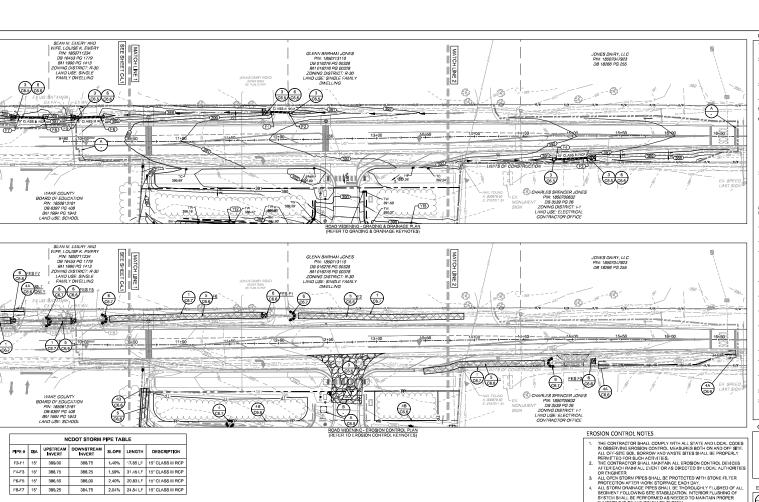
- OR PROPERTY - NORTH CAROLINA

- OR PROPERTY - NORTH DEMO, S DAIRY

ROAD WIDENING PLAN -JONES

0

54832 C7.0



- SYSTEM SHALL BE PERFORMED AS RECEIDED TO MAINTAINE PROPER FUNCTIONING OF HED DRAINED SYSTEM. OR SHALL PROPER THE INDICATED STAGING AREA IS INTENDED AND OR OTHER ERODIBLE MOLERCHAPE MATERIALS OWN. NO SOIL, SAN DOR OTHER ERODIBLE FIFE GRANED MATERIALS, SHALL BE STORED OUTSIDE OF THE LIMITS OF THE SITE PROTECTED BY SEDMENT AND EROSION CONTROL DEVICES.
- AND MEASURES
- AND MEASURES.
 SOIL AND OTHER NATERIALS SHALL ONLY BE TEMPORABILY STOCKPILED WITHIN THE CONSTRUCTION LIMITS REVOCECTED BY SEDMENT AND THE PROTECTION INSPECTION SHALL BE COMPLETED PRIOR TO INSTALLING EROSION CONTROL DEVICES.
 ALL APPLICABLE LESS CONTROL MEASURES ARE TO BE PROPERLY
- MAINTAINED UNTIL PERMANENT VEGETATION IS ESTRAISHED.
 PERMANENT GROUNDCOVER SHALL BE PROVIDED FOR ALL DISTURBED AREAS WITHIN 15 WORKING DAYS OR NO MORE THAN 90 CALENDAY DAYS (WHICHEVER IS SHORTER).

 10. TOTAL DISTURBED AREA. 583 AC.

NCDOT STORM STRUCTURE TABLE						
STRUCTURE # TOP STRUCTURE HEIGHT DESCRIPTION						
F1	390.44	N/A	15" FES			
F2	390.19	N/A	15° FES			
F3	389.69	N/A	15° FES			
F4	390.19	N/A	15° FES			
F5	387.44	N/A	15° FES			
F6	387.94	N/A	15' FES			
F7	386.19	N/A	15' FES			
F8	386.69	N/A	15" FES			

GRADING & DRAINAGE NOTES

- COORDINATE THE PROJECT SCHEDULE WITH THE OWNER, AND COORDINATE THE PROJECT SCHEDULE WITH THE OWNER, AND ADJACENT USERS OF THE PROPERTY, MAINTAN TRAFFICE FLOW AND DO OPERATIONS OF ADJACENT SITES AND FACILITIES AND OR THEIR OWNERS ONGOING OPERATIONS.

 ALL EXISTING VALUES, MANHOLES, STORM DRAIN STRUCTURES, VALVE BOXES, CLEANOUTS, ETC. SHALL BE ADJUSTED AN BEGED TO MATCH
- BOXES, CLEMOUTS, ETC. SHALL BE ADJUSTED AS NEEDED TO HATCH FINISHED CRADE, PRINCED CRADE, SINISHING HEAD FROM THE PRINCED TO HATCH FINISH CRADE AND THE CONNERS INDEPENDENT TESTING LAGORATORY. ALL SPOT LEXATIONS INDICATED ARE AT TOP OF CURB UNLESS NOTED OTHERWISE.

 SPOT CRADE ABSREVIATIONS.

- POT GRADE ABBREVIATIONS:
 TC. TOP OF CURB
 EP. EDGE OF PAVEMENT
 HIP HIGH POINT
 SVK. SIDEWALK
 FFE: FINISHED FLOOR ELEVATION
 TW: TOP OF WALL
 BW: BOTTOM WALL
 FL. FLOW LINE
 GND. GROUND
- EX GND: EXISTING GROUND
- 4.10. LX OND EXISTING ORGUND
 7. SPOT ELEXATIONS ARE GIVEN AT THE MAJORITY OF THE MAJOR SPEAK POINTS BUT IT SHOULD NOT BE ASSUMED THAT ALL NECESSARY SPOT ELEXATIONS ARE SHOWN. DUE TO SPACE LIMITATIONS. THERE MAY BE CONSIDERATED AND ALL STATEMENT OF SHALL REVIEW THE GRADING PLAN IN DETAIL AND SHALL ENSIGN THAT ALL CRITICAL WARDE POINTS ARE SHALL SHOWN THAT ALL CRITICAL WARDE POINTS ARE SHALL SHOWN THE ORGANICAL PLAN FOR THE SHALL BUT OF THE CONTRACTOR SHALL BUT ON OR CALL (BIT) TO LOOTE ALL NUBBERGOUND UTILITIES, REVIATE UTILITIES SHALL BE LOCATED BY A PRIVATE LOCATE SHALL BE DEPOSED OF THE CONTRACTOR.

 10. PRIVATE LOCATE SHALL BE THE EXPENSE OF THE CONTRACTOR.

 10. CROSSING UTILITIES AS NOILATED IN THE DRIVINGS AND SPECIFICATION.

- SPECIFICATIONS.

 PROVIDE HALF-SENCH CONCRETE INLET SHAPING FOR ALL CONCRETE STORM SEWER STRUCTURES.

 ALL ROOF DRAINS FROM BUIDLING A' SHALL BE 8" PVC (SCH 40) @ 1.04%. MILL ROOF DRAINS FROM BUILDING A STACLE DE PVC (SCH 30) BY 1004 MIN. SLOPE UNLESS INDICATED OTHERWISE. ALL ROOF DRAINS FROM BUILDING 'S', C', AND 'D', SHALL BE 6' PVC (SCH 40) <u>6</u>1.04% MIN. SLOPE UNLESS INDICATED OTHERWISE. USE DUTLE IRON WHEN COVER IS LESS THAN 24-IN, MATCH PIPE CROWNS WITH CONNECTION TO DROP MILET.
- INLET.
 PVC ROOF DRAIN PIPING UNDER PAVEMENT SHALL HAVE 24-IN MINIMUM
 COVER, IF ROOF DRAIN PIPING UNDER PAVEMENT HAS LESS THAN 24-IN
 COVER, ROOF DRAIN PIPING SHALL BE 8'D IF UN LEU OF PVC).
 JOINT FILL AND CALLK EACH CONCRETE EXPANSION JOINT AND VHERE
 CONCRETE PAVEMENT ABUTS OTHER PAVEMENTS, SIDEWALKS, OR

- JOHN THE AND CALLY EAGH CONCRETE CPANISON, JOHN THEO WHEELE CONCRETE PRAYED AREAS TO THE TWENEVERS, BOYALMAS, OR ROCKERS FALLY REPORT AND THE AND THE
- SYSTEM. IN DISTURBED AREAS, AMEND THE TOP 6 INCHES OF LAWN AREAS WITH TOPSOIL FROM THE SITE

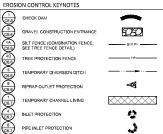
- 7. N. DISTURBED. AREA, MEND THE TOP 6 INCHES OF LAWN AREAS WITH TOPROIL FROM THE SITE.

 18. ALL SIDEWALKS ARE TO HAVE NO MORE THAN A 1-20 (5-05), SLOPE FOR THE LENDTH OF THE SIDEWALK AND NO MORE THAN A 1-20 (5-05), SLOPE FOR THE HIDTH OF THE SIDEWALK.

 19. ALL SIDEWALKS ARE TO HAVE NO MORE THAN A 1-20 (5-05), SLOPE FOR THE HIDTH OF THE SIDEWALK.

 19. ALL SIDEWALKS AREA SIDEWALKS SERVICES IN ANY OF THESE SLOPE FOR SIDEWALKS AND STORMER S CONTRACTOR AT NO ADDITIONAL COST TO THE OWNER

GRADING	& DRAINAGE KEY NOTES
3 C6.5	FLARED END SECTION
6 C8.6	RIP RAP OUTLET PROTECTION (SEE EROSION CONTROL PLAN)
$\overline{\triangle}$	PROVIDE SMOOTH PAVING TRANSITION



CONCRETE WASHOUT AREA

BAIN GALIGE AND PERMIT RECORDS BOX

LIMITS OF CONSTRUCTION EXISTING CONTOUR

(A)

FINISHED CONTOUR DUST CONTROL



짱

FACILITY EROSION : ច STORA(GRADING, DAIRY 1 OF ROLESVILLE ഗ്ളി **JONE**

DATE 06/01/2023 08/01/2023 11/01/2023

04/05/2023

L. BARNES DESIGNED B G. FRANK CHECKED BY

G. FRANK

SCALE AS SHOWN PLAN

CONTROL

ROAD \ 54832 C7.1

WIDENING





(c)

(W)

WEDGING DETAIL

G)

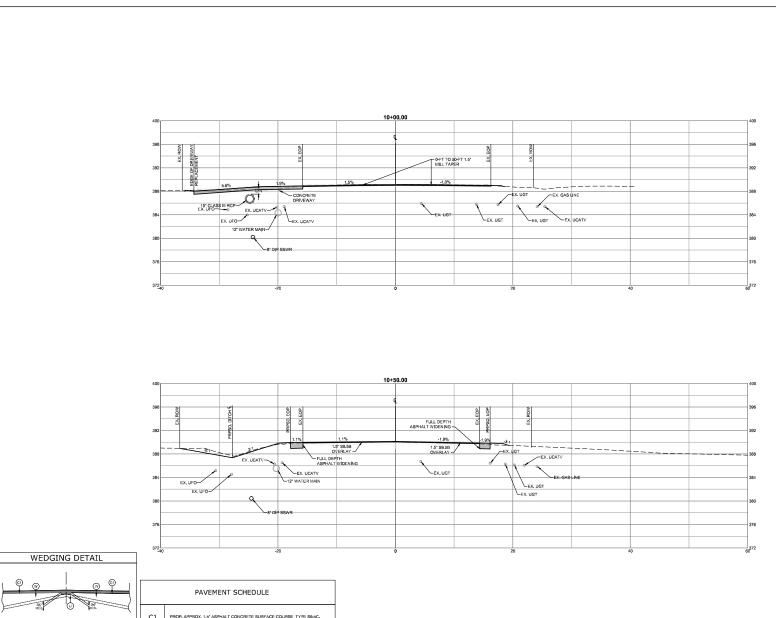


NOTE: MILL OR OVERLAY EXISTING PAVEMENT AS

NOTE: MILL OR OVERLAY EXISTING PAVEMENT AS NECESSARY TO A OHIEVE REQUIRED CROSS S.OPE PER CROSS SECTIONS AND SUPERELEVATION TRANSITIONS, BUILD UP WITH ASPIRALT CONCRETE INTERNACTION, TYPE 1180 AT VARIABLE DEPTH DEPENDING ON EXISTING CIRCUMSTANCES. REFER TO NCDO'T ROAD DESIGN MANUAL FOR MINIMUM PAVEMENT LIFTS AND WEDGING PEGUIREMENTS.

ON MET STOP THEFTED IN NOT THE EXPECTION FOR MEN THAT IS BEINGTON AT A TIME.

SEED OF THE THEFTED IN NOTION TO BE A STATE OF THE STATE STOPLMEPER ASLANDAMENTAL TELEFORM SOME LIFET PAGE BUSINESS. U EXISTING PAVEMENT E REPORT TO DOOR BUTTO, CHAPTER &, FOR FLAGUES CONTROL, ENQUERMENTS, AND PROCESSE © ON NET DESCRIPA 1, FOR A LANGE CORRESS LANGES LANGES CONTROLED BROWN IN THE THE CH





THIS DRAWING PREPARED ATTHE RALEIGH OFFICE 5410 Trinty Road, Suite 102 | Paleigh, NC 27607 TEI 919,866,4951 FAX 919,833,8124 www.timmons.

DATE 06/01/2023 9 08/01/2023 11/01/2023

04/05/2023 L. BARNES DESIGNED BY

G. FRANK CHECKED BY G. FRANK SCALE

AS SHOWN

JONES DAIRY STORAGE FACILITY
TOWN OF ROLES/ILE - WAKE COUNTY - NORTH CAROLINA
JONES DAIRY ROAD - CROSS SECTIONS STA. 10+00 - 10+50
A personal number of principal personal per

54832 C7.2

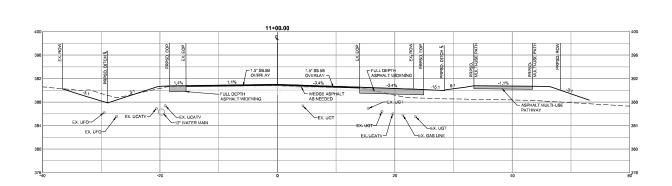
C1 PROP. APPROX. 1.6" ASPHALT CONCRETE SURFACE COURSE, TYPE \$9.5C.

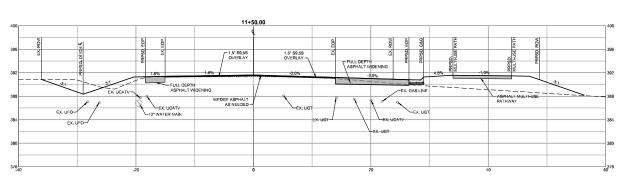
W PROP, VAR. DEPTH ASPHALT CONCRETE SURFACE COURSE, TYPE \$9.5C.

NOTE: MILL OR OVERLAY EXISTING PAVEMENT AS NECESSARY TO AGNETICE REQUIRED CROSS SLOPE PER SUIT OF THE PARTY O U EXISTING PAVEMENT

DETAIL SHOWING METHOD OF WEDGING

L





WEDGING DETAIL DETAIL SHOWING METHOD OF WEDGING

NOTE: MILL OR OVERLAY EXISTING PAVEMENT AS NECESSARY TO ACHIEVE REQUIRED CROSS SLOWE PER SULLO UNITED CROSS STEPS TO NCOO! TO ACHIEVE STEPS AND ACTIVITY OF MINIMAL PAVEMENT LET'S AND WEDDING REQUIRED CROSS SERVENT OF THE SAME VEDDING REQUIRED CROSS SLOWE PAVEMENT LET'S AND WEDDING REQUIRED CROSS SERVENT SLOWE PAVEMENT LET'S AND WEDDING REQUIRED CROSS SERVENT SLOWE PAVEMENT LET'S AND WEDDING REQUIRED CROSS SERVENT SERV

L

	PAVEMENT SCHEDULE
C1	PROP. APPROX. 1.5" ASPHALT CONCRETE SURFACE COURSE, TYPE \$9.5C.
W	PROP. VAR. DEPTH ASPHALT CONCRETE SURFACE COURSE, TYPE \$9.5C.
U	EXISTING PAVEMENT



THIS DRAWING REPARED ATTHE RALEGH OFFICE 5410 Trinty Road, Suite 102 | Raleigh, MC 27607 TE, 919,866,4951 FAX 919,833,8124 www.immens.

DATE 06/01/2023 9 08/01/2023 11/01/2023 04/05/2023

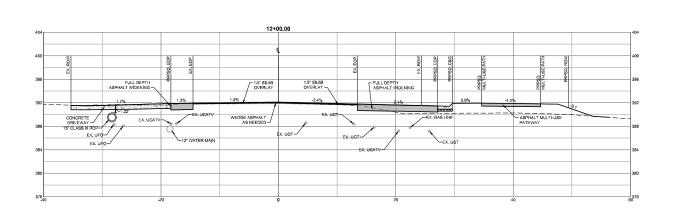
> L. BARNES DESIGNED BY G. FRANK CHECKED BY

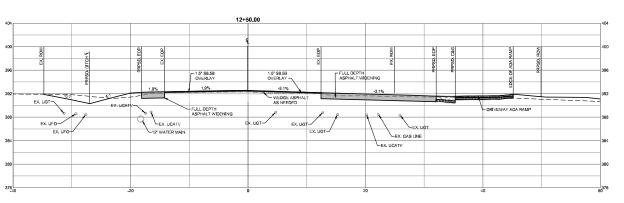
G. FRANK SCALE

AS SHOWN

JONES DAIRY STORAGE FACILITY
TOWN OF DOLES/ILLE - WARE COUNTY - HORTH CAROLLINA
JONES DAIRY ROAD - CROSS SECTIONS STA. 11+600 - 11+50
Specime and prediction property of the p

54832 SHEET NO. C7.3





WEDGING DETAIL DETAIL SHOWING METHOD OF WEDGING

NOTE: MILL OR OVERLAY EXISTING PAVEMENT AS NECESSARY TO ACHIEVE REQUIRED CROSS SLOWE PER SULLO UNITED ACHIEVE REQUIRED CROSS SLOWE PER SULLO UP WITH ASPHALT CONCRETE INTERMEDIATE TYPE ITSIGA AT VARIABLE DEPTH DEPENDING OF LOSSITING CIRCUMSTANCES, SEFER TO NCDO'S ROAD DESIGN MANUAL FOR MINIMUM PAVEMENT LET'S AND WEDDING REQUIRED.

L

	PAVEMENT SCHEDULE			
C1	PROP. APPROX. 1.5" ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5C.			
w	PROP. VAR. DEPTH ASPHALT CONCRETE SURFACE COURSE, TYPE \$9.5C.			
U	EXISTING PAVEMENT			



THIS DRAWING REPARED ATTHE RALEGH OFFICE 5410 Trinty Road, Suite 102 | Raleigh, MC 27607 TE, 919,866,4951 FAX 919,833,8124 www.immore.

DATE 06/01/2023 9 08/01/2023 11/01/2023 04/05/2023

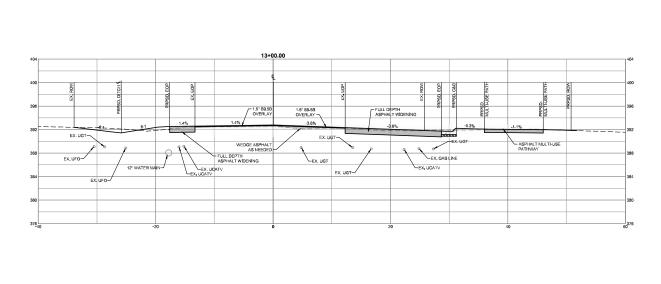
L. BARNES DESIGNED BY G. FRANK

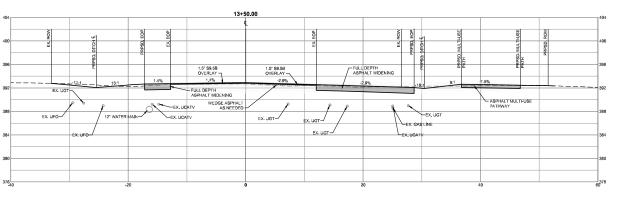
CHECKED BY G. FRANK SCALE

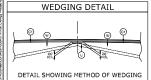
AS SHOWN

JONES DAIRY STORAGE FACILITY
TOWN OF ROLESCILLE - WARE COUNTY - NORTH CAROLINA
JONES DAIRY ROAD - CROSS SECTIONS STA. 12+00 - 12+50
Recommendation where y referred from any and a second or proper control and a second

54832 C7.4







NOTE: MILL OR OVERLAY EXISTING PAVEMENT AS NECESSARY TO ACHIEVE REQUIRED CROSS SLOWE PER SULD UP WITH SAPILAT CONCEPT IN TERMEDIATE TYPE ITSO, AT VARIABLE DEPTH DEPENDING OR EXISTING CROUNTSTAND FOR THE SAPILAT CONCEPT ON CODE TO ACHIEVE THE SAPILATION OF WINDLAM PAVEMENT LIFTS AND VERSIONS OF ECUITABLE OF WINDLAM PAVEMENT LIFTS AND VERSIONS OF WINDLAM PAVEMENT LIFTS OF WINDLAM P

L

	PAVEMENT SCHEDULE			
C1	PROP. APPROX. 1.5" ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5C.			
W	PROP. VAR. DEPTH ASPHALT CONCRETE SURFACE COURSE, TYPE \$9.5C.			
U	EXISTING PAVEMENT			



THIS DRAWING REPARED ATTHE RALEGH OFFICE 5410 Trinty Road, Suite 102 | Raleigh, MC 27607 TE, 919,866,4951 FAX 919,833,8124 www.immens.

DATE 06/01/2023 9 08/01/2023 11/01/2023

04/05/2023 L. BARNES

DESIGNED BY G. FRANK CHECKED BY G. FRANK

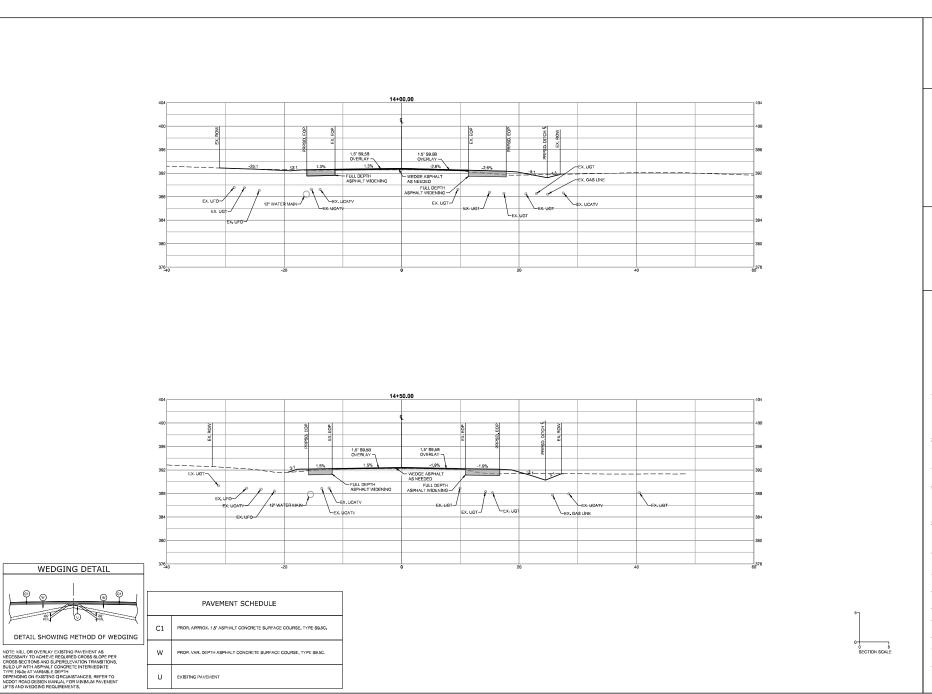
SCALE

AS SHOWN

JONES DAIRY STORAGE FACILITY
TOWN OF ROLESTILE - WAKE COUNTY - NORTH CAROLINA
JONES DAIRY ROAD - CROSS SECTIONS STA. 13+00 - 13+50
Seconda shirk-langing (TRINSING Persipa, et al. 1800) per representation consequence in the consequence of the

308 NO.

54832 C7.5



L

THIS DRAWING REPARED ATTHE RALEGH OFFICE 5410 Trinty Road, Suite 102 | Ralegh, NC 27607 TE, 919,865,4951 FAX 919,833,8124 www.immens.

DATE 06/01/2023 9 08/01/2023 11/01/2023 04/05/2023

L. BARNES DESIGNED BY G. FRANK

CHECKED BY G. FRANK

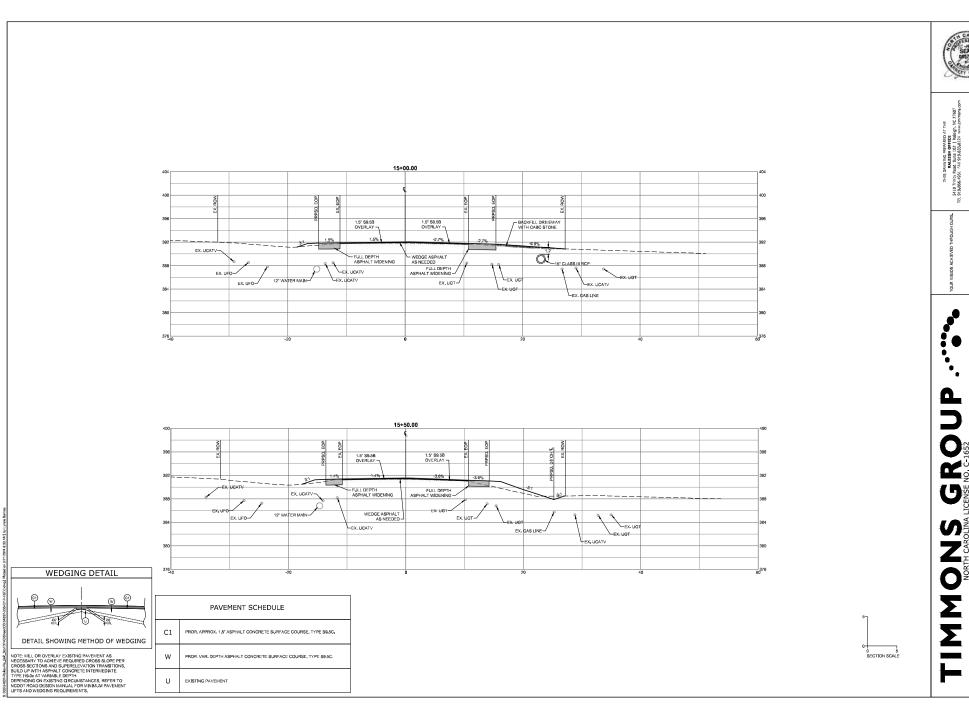
SCALE

AS SHOWN

JONES DAIRY STORAGE FACILITY
TOWN OF ROLESULE - WARE COUNTY - NORTH CAROLINA
JONES DAIRY ROAD - CROSS SECTIONS STA. 14+00 - 14+50
secure astrone large (TURISC) per may use progress the proper values from the 17 per enter of th

54832

SHEET NO. C7.6



L

04/05/2023

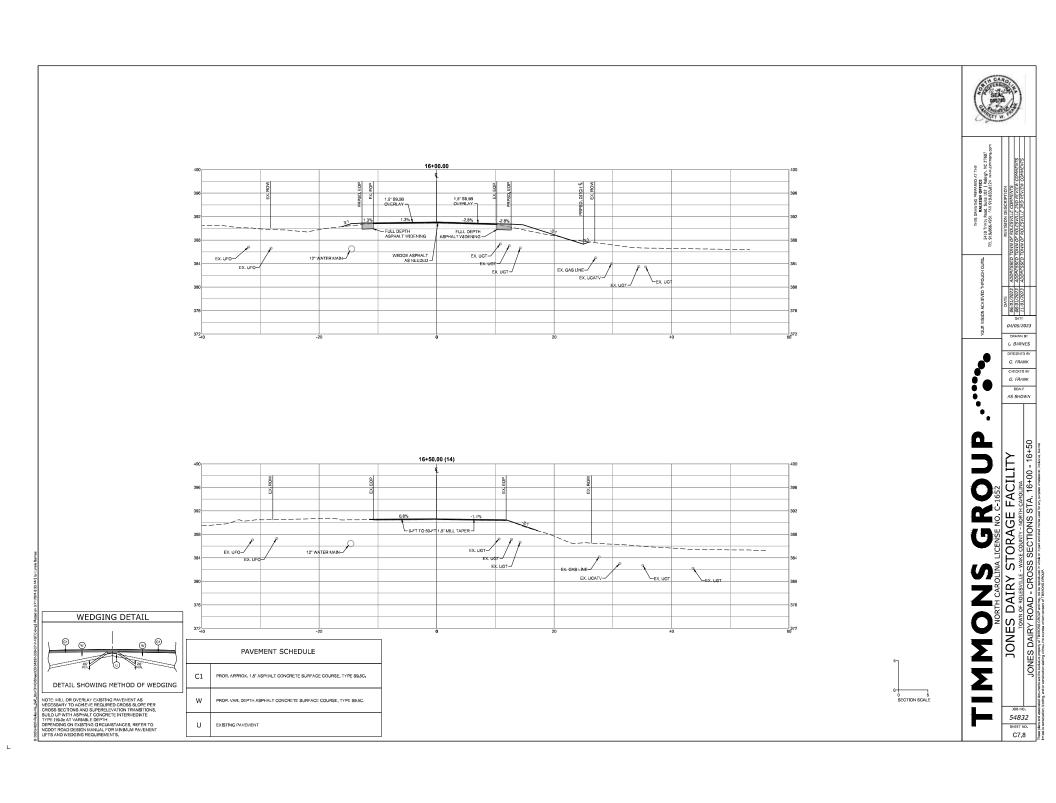
L. BARNES DESIGNED BY

G. FRANK CHECKED BY G. FRANK SCALE

AS SHOWN

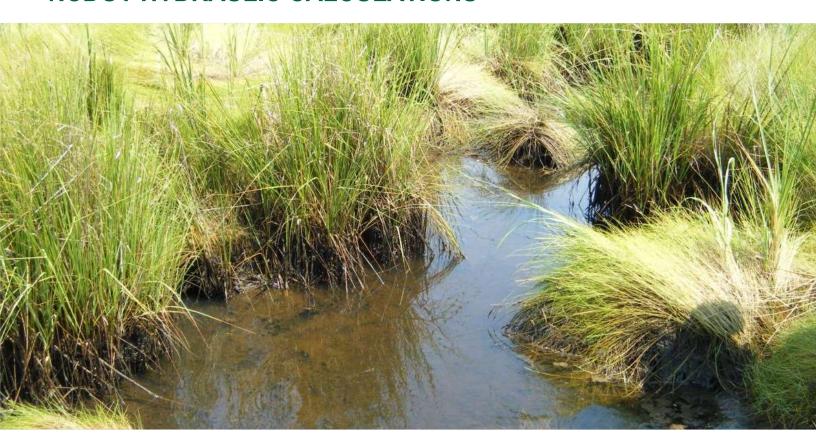
JONES DAIRY STORAGE FACILITY
TOWN OF ROLES/ILLE - WARK COUNTY - NORTH CAROLINA
JONES DAIRY ROAD - CROSS SECTIONS STA. 15+00 - 15+50
Decreased in the company of transcription of the company of the compa

54832 C7.7





JONES DAIRY STORAGE FACILITY NCDOT HYDRAULIC CALCULATIONS



JANUARY 25, 2023

PREPARED BY:

Garrett Frank, PLA, PE 5410 Trinity Road, Suite 102 Raleigh, North Carolina 27607 919.866.4503 phone 919.859.5663 fax Garrett.frank@timmons.com www.timmons.com





RIP RAP OUTLET DESIGN



OUTLET PROTECTION	DATE
DESIGN	07/31/2023
PROJECT NAME	PROJECT NO
Rolesville Self Storage Facility	54832
LOCATION	BY
Rolesville, NC	LMB



RIPRAP FES F1

Pipe Diameter: 15.00 in 389.00 Lower Invert: 388.75 Pipe Length: 17.45 ft

Pipe Slope: 0.014 Discharge:

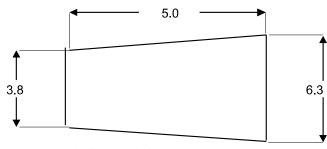
Pipe Material: RCP

Q10 = 0.47 cfs Qfull = 7.74 cfs Vfull = 6.31 fps Q10/Qfull = 0.06 V/Vfull = 0.52 V = 3.29 fps

0.47 cfs

Dissipator Dimensions * Zone = 1
Stone Filling Class = A

Entry Width $(3 \times D_0)$ = 3.8 ft Length $(4 \times D_0)$ = 5.0 ft Width $(La + D_0)$ = 6.3 ft Min. Thickness = 12 inches Stone Diameter (D_{50}) = 3 inches



*Discharge from Storm Sewers

* All units are in feet

8/3/2023 Page 4 of 5

^{**} Dissipator pad designed for full flow of pipe

OUTLET PROTECTION	DATE
DESIGN	07/31/2023
PROJECT NAME	PROJECT NO
Rolesville Self Storage Facility	54832
LOCATION	ВҮ
Rolesville, NC	LMB



RIPRAP FES F3

15.00 Pipe Diameter: in Upper Invert: 389.00 Lower Invert: 388.50 Pipe Length: 31.45

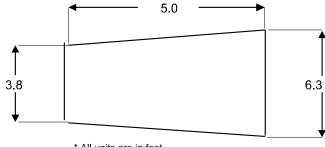
*Discharge from Storm Sewers Pipe Slope: 0.016 Discharge: 0.32 cfs

Pipe Material: RCP

Q10 = 0.32 cfs $Q_{10}/Q_{full} =$ 0.04 Qfull = V/Vfull = 0.42 8.16 cfs 2.81 fps Vfull = 6.65 fps V =

Dissipator Dimensions * Zone = Stone Filling Class = Α

Entry Width ($3 \times D_0$) = 3.8 ft Length ($4 \times D_0$) = 5.0 ft Width (La + D_0) = 6.3 ft Min. Thickness = 12 inches Stone Diameter (D₅₀)= 3 inches



* All units are in feet

8/3/2023 Page 5 of 5

^{**} Dissipator pad designed for full flow of pipe

OUTLET PROTECTION	DATE
DESIGN	01/19/2024
PROJECT NAME	PROJECT NO
Rolesville Self Storage Facility	54832
LOCATION	ВҮ
Rolesville, NC	LMB



0.15

0.72

6.51 fps

RIPRAP FES F5

 Pipe Diameter:
 15.00 in

 Upper Invert:
 387.00

 Lower Invert:
 386.40

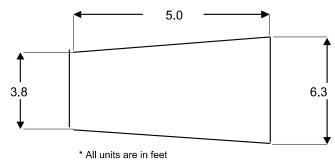
 Pipe Length:
 20.35 ft

Pipe Length: 20.35 ft *Discharge from Storm Sewers
Pipe Slope: 0.029 triangle to the storm Sewers
Discharge: 1.70 cfs

Pipe Material: RCP

 $Q_{10} = 1.70 \text{ cfs}$ $Q_{10}/Q_{full} = Q_{full} = 11.11 \text{ cfs}$ $V/V_{full} = V_{full} = 9.05 \text{ fps}$ V = 0.00

Dissipator Dimensions * Zone = $\frac{1}{A}$ Stone Filling Class = $\frac{1}{A}$ Entry Width $(3 \times D_0)$ = 3.8 ft



** Dissipator pad designed for full flow of pipe

1/19/2024 Page 6 of 7

OUTLET PROTECTION	DATE
DESIGN	01/19/2024
PROJECT NAME	PROJECT NO
Rolesville Self Storage Facility	54832
LOCATION	ву
Rolesville, NC	LMB



0.21

0.78

5.88 fps

RIPRAP FES F7

15.00 Pipe Diameter: in Upper Invert: 385.00 Lower Invert: 384.50 Pipe Length: 24.50

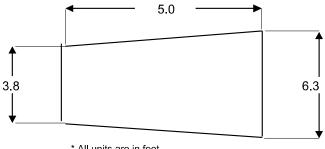
*Discharge from Storm Sewers Pipe Slope: 0.020 Discharge: 1.90 cfs

Pipe Material: RCP

Q10 = 1.90 cfs Q10/Qfull = Qfull = V/Vfull = 9.24 cfs Vfull = 7.53 fps V =

Dissipator Dimensions * Zone = Stone Filling Class = Α

Entry Width ($3 \times D_0$) = 3.8 ft Length ($4 \times D_0$) = 5.0 ft Width (La + D_0) = 6.3 ft Min. Thickness = 12 inches Stone Diameter (D₅₀)= 3 inches



* All units are in feet

1/19/2024 Page 7 of 7

^{**} Dissipator pad designed for full flow of pipe

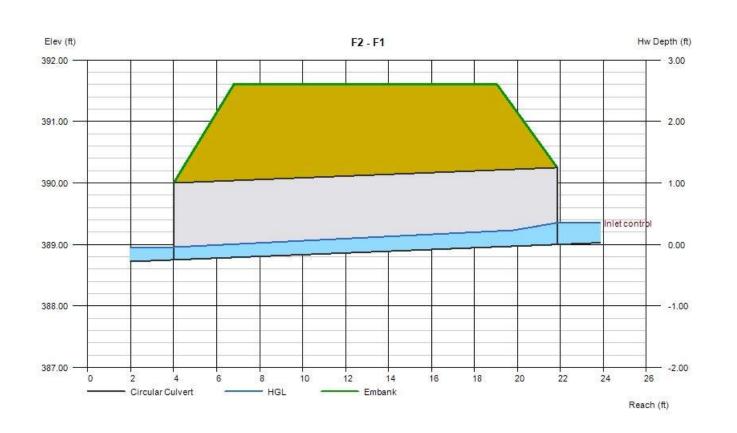
CULVERT DESIGN



Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

F2 - F1

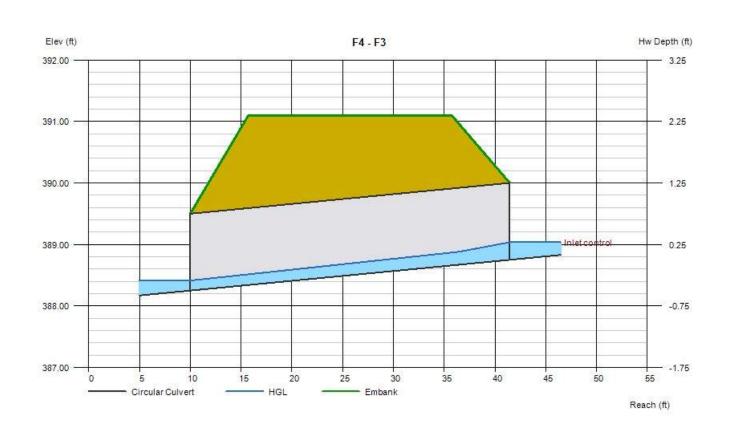
= 388.75	Calculations	
= 17.85	Qmin (cfs)	= 0.47
= 1.40	Qmax (cfs)	= 0.47
= 389.00	Tailwater Elev (ft)	= Normal
= 15.0	` '	
= Circular	Highlighted	
= 15.0		= 0.47
= 1	,	= 0.47
= 0.012	Qovertop (cfs)	= 0.00
= Circular Concrete	Veloc Dn (ft/s)	= 3.57
= Square edge w/headwall (C)	Veloc Up (ft/s)	= 2.45
= 0.0098, 2, 0.0398, 0.67, 0.5	HGL Dn (ft)	= 388.96
	HGL Up (ft)	= 389.27
	Hw Elev (ft)	= 389.35
= 391.60	Hw/D (ft)	= 0.28
= 12.25	Flow Regime	= Inlet Control
= 12.25	-	
	= 17.85 = 1.40 = 389.00 = 15.0 = Circular = 15.0 = 1 = 0.012 = Circular Concrete = Square edge w/headwall (C) = 0.0098, 2, 0.0398, 0.67, 0.5 = 391.60 = 12.25	= 17.85 Qmin (cfs) = 1.40 Qmax (cfs) = 389.00 Tailwater Elev (ft) = 15.0 = Circular Highlighted = 15.0 Qtotal (cfs) = 1 Qpipe (cfs) = 0.012 Qovertop (cfs) Circular Concrete Veloc Dn (ft/s) = Square edge w/headwall (C) Veloc Up (ft/s) = 0.0098, 2, 0.0398, 0.67, 0.5 HGL Dn (ft) HGL Up (ft) HW Elev (ft) HW/D (ft) = 391.60 Hw/D (ft) Flow Regime



Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

F4 - F3

= 388.25	Calculations	
= 31.45	Qmin (cfs)	= 0.32
= 1.59	Qmax (cfs)	= 0.32
= 388.75	Tailwater Elev (ft)	= Normal
= 15.0		
= Circular	Highlighted	
= 15.0	Qtotal (cfs)	= 0.32
= 1	Qpipe (cfs)	= 0.32
= 0.012	Qovertop (cfs)	= 0.00
Circular Concrete	Veloc Dn (ft/s)	= 3.38
Square edge w/headwall (C)	Veloc Up (ft/s)	= 2.21
= 0.0098, 2, 0.0398, 0.67, 0.5	HGL Dn (ft)	= 388.41
	HGL Up (ft)	= 388.97
	Hw Elev (ft)	= 389.04
= 391.10	Hw/D (ft)	= 0.23
= 20.00	Flow Regime	= Inlet Control
= 20.00	-	
	= 31.45 = 1.59 = 388.75 = 15.0 = Circular = 15.0 = 1 = 0.012 = Circular Concrete = Square edge w/headwall (C) = 0.0098, 2, 0.0398, 0.67, 0.5 = 391.10 = 20.00	= 31.45 Qmin (cfs) = 1.59 Qmax (cfs) = 388.75 Tailwater Elev (ft) = 15.0 = Circular Highlighted = 15.0 Qtotal (cfs) = 1 Qpipe (cfs) = 0.012 Qovertop (cfs) = Circular Concrete Veloc Dn (ft/s) = Square edge w/headwall (C) Veloc Up (ft/s) = 0.0098, 2, 0.0398, 0.67, 0.5 HGL Dn (ft) HGL Up (ft) HW Elev (ft) = 391.10 Hw/D (ft) = 20.00 Flow Regime



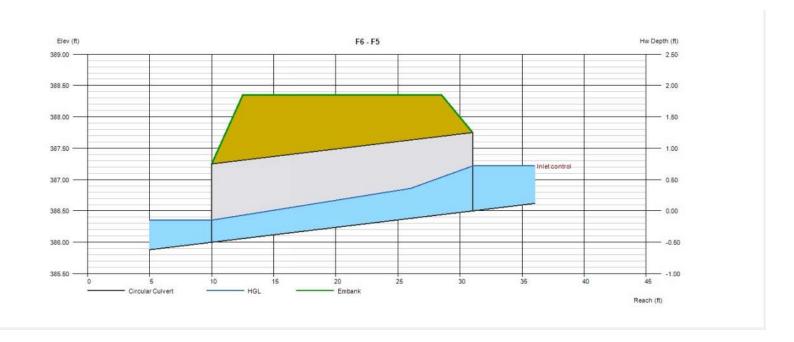
Culvert Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Friday, Jan 26 2024

F6 - F5

Invert Elev Dn (ft)	= 386.00	Calculations	
Pipe Length (ft)	= 21.01	Qmin (cfs)	= 1.70
Slope (%)	= 2.38	Qmax (cfs)	= 1.70
Invert Elev Up (ft)	= 386.50	Tailwater Elev (ft)	= Normal
Rise (in)	= 15.0		
Shape	= Circular	Highlighted	
Span (in)	= 15.0	Qtotal (cfs)	= 1.70
No. Barrels	= 1	Qpipe (cfs)	= 1.70
n-Value	= 0.012	Qovertop (cfs)	= 0.00
Culvert Type	Circular Concrete	Veloc Dn (ft/s)	= 5.98
Culvert Entrance	= Square edge w/headwall (C)	Veloc Up (ft/s)	= 3.55
Coeff. K,M,c,Y,k	= 0.0098, 2, 0.0398, 0.67, 0.5	HGL Dn (ft)	= 386.35
		HGL Up (ft)	= 387.02
Embankment		Hw Elev (ft)	= 387.22
Top Elevation (ft)	= 388.35	Hw/D (ft)	= 0.57
Top Width (ft)	= 16.00	Flow Regime	= Inlet Control
Crest Width (ft)	= 16.00		



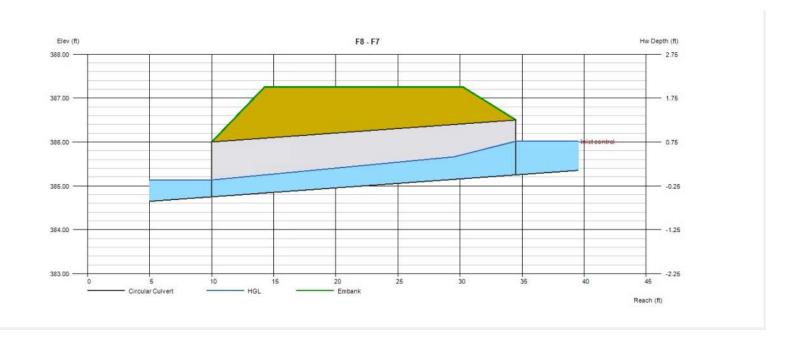
Culvert Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Friday, Jan 26 2024

F8 - F7

Invert Elev Dn (ft)	= 384.75	Calculations	
Pipe Length (ft)	= 24.50	Qmin (cfs)	= 1.89
Slope (%)	= 2.04	Qmax (cfs)	= 1.89
Invert Elev Up (ft)	= 385.25	Tailwater Elev (ft)	= Normal
Rise (in)	= 15.0		
Shape	= Circular	Highlighted	
Span (in)	= 15.0	Qtotal (cfs)	= 1.89
No. Barrels	= 1	Qpipe (cfs)	= 1.89
n-Value	= 0.012	Qovertop (cfs)	= 0.00
Culvert Type	Circular Concrete	Veloc Dn (ft/s)	= 5.91
Culvert Entrance	= Square edge w/headwall (C)	Veloc Up (ft/s)	= 3.66
Coeff. K,M,c,Y,k	= 0.0098, 2, 0.0398, 0.67, 0.5	HGL Dn (ft)	= 385.13
		HGL Up (ft)	= 385.80
Embankment		Hw Elev (ft)	= 386.02
Top Elevation (ft)	= 387.25	Hw/D (ft)	= 0.61
Top Width (ft)	= 16.00	Flow Regime	= Inlet Control
Crest Width (ft)	= 16.00		



CHANNEL DESIGN









PERMANENT DIVERSION DITCH: F2

This worksheet is designed to determine channel lining based on flow and ditch geometry (base width, side slope, & channel slope). This program determines depth of flow, velocity and tractive force assuming n=0.03 for grass or rip rap (n can be varied if desired). (Vary depth until z actual equals z required; v & t will be correct; select appropriate lining based on tractive force)

Watershed Data

Area (A) = **0.17** (Acres)

Coef. (C) = 0.60 (Dimensionless)

Inte. (I) = 7.19 (in/hr) Flow (Q₁₀) = 0.7 (cfs)

Known Quantities

Line channel with: 6 " rip-rap (Assume 6" even if using grass)

Manning n = 0.030 (Dimensionless) Ditch Length = 122 LF Slope = 0.030 (ft/ft) Highest Elevation = 392.6 Z Required = 0.08 Ratio Lowest Elevation = 389

Side Slope (M) = 3:1

Variable Quantities (Ditch Width & Depth)

Flow Depth (Y) = 0.32 ft 3.8 in
Bottom Width (B) = 0.00 ft 0.0 in
Freeboard = 0.00 ft 0.0 in

Compute Tractive Force

T=YxDxS

T =

Y = Weight of water (62.4 LB/CUFT)
0.59 LB/SF D=Depth of flow in channel (ft)

S=Slope of channel (ft/ft)

General Lining	LB/SF
Jute Net	0.45
Curled Mat	1.55
Class A (4"@9"THK)	2.00
Class B (8"@18"THK)	3.50
Class I (12"@22"THK)	5.00
Class II (18"@30"THK)	7.50

North American	
Green Lining	LB/SF
S75	1.55
S150	1.75
SC150	2.10
C125	2.25
P300	8.00

Determine If Z Actual Is Greater Than Z Required

New A = 0.31 SF

New P = 2.02 Wetted perimeter New R = 0.15 Hydraulic radius

Z Actual = 0.09 Must be greater than z required=> 0.08 Okay

New V = 2.43 fps

Minimum Design Geometry Permanent Liner: Grass Temporary Liner: Curled Mat Top Width = 1.92 ft Min. Ditch Depth = 0.32 ft Bottom Width (B) = 0 ft Top Width = 1.92 ft Flow Depth (Y) = 0.32 ft

Ditch Geometry						
Min. Ditch Depth (ft)	Bottom Width (ft)	Flow Depth (ft)	Top Width (ft)	Side Slope		
0.32	0.00	0.32	1.92	3:1		

Ditch Liner

Permanent Liner:Grass,Temporary Liner:Curled Mat







PERMANENT DIVERSION DITCH: F4

This worksheet is designed to determine channel lining based on flow and ditch geometry (base width, side slope, & channel slope). This program determines depth of flow, velocity and tractive force assuming n=0.03 for grass or rip rap (n can be varied if desired). (Vary depth until z actual equals z required; v & t will be correct; select appropriate lining based on tractive force)

Watershed Data

Area (A) = 0.09 (Acres)

0.60 (Dimensionless) Coef. (C) =

Inte. (I) = **7.19** (in/hr) Flow $(Q_{10}) =$ **0.4** (cfs)

Known Quantities

Line channel with: 6 " rip-rap (Assume 6" even if using grass)

Manning n = 0.030 (Dimensionless) Ditch Length = 132 LF Slope = 0.026 (ft/ft) Highest Elevation = 392.2 Z Required = 0.05 Ratio Lowest Elevation = 388.75

Side Slope (M) = 3:1

Variable Quantities (Ditch Width & Depth)

Flow Depth (Y) = 3.2 in **0.27** ft Bottom Width (B) = 0.00 ft 0.0 in Freeboard = 0.00 ft 0.0 in

Compute Tractive Force

T=YxDxS

Y = Weight of water (62.4 LB/CUFT) T = 0.44 LB/SF D=Depth of flow in channel (ft) S=Slope of channel (ft/ft)

General Lining LB/SF Jute Net 0.45 Curled Mat 1.55 Class A (4"@9"THK) 2.00 Class B (8"@18"THK) 3.50 Class I (12"@22"THK) 5.00 Class II (18"@30"THK) 7.50

North American	
Green Lining	LB/SF
S75	1.55
S150	1.75
SC150	2.10
C125	2.25
P300	8.00

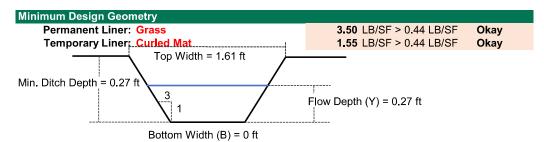
Determine If Z Actual Is Greater Than Z Required

New A = 0,22 SF

New P = 1.70 Wetted perimeter New R = 0.13 Hydraulic radius

Z Actual = **0.06** Must be greater than z required=> **0.05** Okay

New V =



Ditch Geometry						
Min. Ditch Depth (ft)	Bottom Width (ft)	Flow Depth (ft)	Top Width (ft)	Side Slope		
0.27	0.00	0.27	1.61	3:1		

Ditch Liner

Permanent Liner: Grass, Temporary Liner: Curled Mat







PERMANENT DIVERSION DITCH: F6

This worksheet is designed to determine channel lining based on flow and ditch geometry (base width, side slope, & channel slope). This program determines depth of flow, velocity and tractive force assuming n=0.03 for grass or rip rap (n can be varied if desired). (Vary depth until z actual equals z required; v & t will be correct; select appropriate lining based on tractive force)

Watershed Data

Area (A) = **0.39** (Acres)

Coef. (C) = 0.60 (Dimensionless)

Inte. (I) = 7.19 (in/hr) Flow (Q_{10}) = 1.7 (cfs)

Known Quantities

Line channel with: 6 " rip-rap (Assume 6" even if using grass)

Manning n =0.030 (Dimensionless)Ditch Length =170 LFSlope =0.010 (ft/ft)Highest Elevation =388.75Z Required =0.34 RatioLowest Elevation =387

Side Slope (M) = 3:1

Variable Quantities (Ditch Width & Depth)

Flow Depth (Y) = 0.54 ft 6.5 in
Bottom Width (B) = 0.00 ft 0.0 in
Freeboard = 0.00 ft 0.0 in

Compute Tractive Force

T=YxDxS

XS Y = Weight of water (62.4 LB/CUFT)
T = 0.35 LB/SF D=Depth of flow in channel (ft)

S=Slope of channel (ft/tt)

S=Slope of channel (ft/ft)

General Lining	LB/SF
Jute Net	0.45
Curled Mat	1.55
Class A (4"@9"THK)	2.00
Class A (4"@9"THK) Class B (8"@18"THK) Class I (12"@22"THK) Class II (18"@30"THK)	3.50
Class I (12"@22"THK)	5.00
Class II (18"@30"THK)	7.50

North American	
Green Lining	LB/SF
S75	1.55
S150	1.75
SC150	2.10
C125	2.25
P300	8.00

Determine If Z Actual Is Greater Than Z Required

New A = 0.87 SF

New P = 3.41 Wetted perimeter New R = 0.26 Hydraulic radius

Z Actual = 0.35 Must be greater than z required=> 0.34 Okay

New V = 2.03 fps

Minimum Design Geometry Permanent Liner: Grass Temporary Liner: Curled Mat. Top Width = 3.23 ft Min. Ditch Depth = 0.54 ft Bottom Width (B) = 0 ft

Ditch Geometry						
Min. Ditch Depth (ft)	Bottom Width (ft)	Flow Depth (ft)	Top Width (ft)	Side Slope		
0.54	0.00	0.54	3.23	3:1		

Ditch Liner

Permanent Liner:Grass,Temporary Liner:Curled Mat







PERMANENT DIVERSION DITCH: F8

This worksheet is designed to determine channel lining based on flow and ditch geometry (base width, side slope, & channel slope). This program determines depth of flow, velocity and tractive force assuming n=0.03 for grass or rip rap (n can be varied if desired). (Vary depth until z actual equals z required; v & t will be correct; select appropriate lining based on tractive force)

Watershed Data

Area (A) = **0.44** (Acres)

Coef. (C) = 0.60 (Dimensionless)

Inte. (I) = 7.19 (in/hr) Flow (Q₁₀) = 1.9 (cfs)

Known Quantities

Line channel with: 6 " rip-rap (Assume 6" even if using grass)

Manning n =0.030 (Dimensionless)Ditch Length =30 LFSlope =0.047 (ft/ft)Highest Elevation =386.4Z Required =0.18 RatioLowest Elevation =385

Side Slope (M) = 3:1

Variable Quantities (Ditch Width & Depth)

Flow Depth (Y) = 0.43 ft 5.1 in
Bottom Width (B) = 0.00 ft 0.0 in
Freeboard = 0.00 ft 0.0 in

Compute Tractive Force

T=YxDxS

XS Y = Weight of water (62.4 LB/CUFT)
T = 1.25 LB/SF D=Depth of flow in channel (ft)
S=Slope of channel (ft/ft)

General Lining	LB/SF
Jute Net	0.45
Curled Mat	1.55
Class A (4"@9"THK) Class B (8"@18"THK)	2.00
Class B (8"@18"THK)	3.50
Class I (12"@22"THK)	5.00
Class II (18"@30"THK)	7.50

North American	
Green Lining	LB/SF
S75	1.55
S150	1.75
SC150	2.10
C125	2.25
P300	8.00

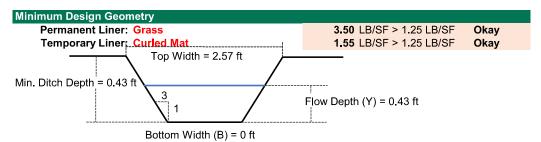
Determine If Z Actual Is Greater Than Z Required

New A = 0.55 SF

New P = 2.71 Wetted perimeter New R = 0.20 Hydraulic radius

Z Actual = 0.19 Must be greater than z required=> 0.18 Okay

New V = 3.71 fps



Ditch Geometry						
Min. Ditch Depth (ft)	Bottom Width (ft)	Flow Depth (ft)	Top Width (ft)	Side Slope		
0.43	0.00	0.43	2.57	3:1		

Ditch Liner

Permanent Liner:Grass,Temporary Liner:Curled Mat

GUTTER SPREAD



Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Monday, Jan 15 2024

Gutter Spread Plan East

Outlei		
Cross SI, Sx (ft/ft)	=	0.034
Cross SI, Sw (ft/ft)	=	0.020
Gutter Width (ft)	=	2.00
Invert Elev (ft)	=	391.72
Slope (%)	=	1.50
N-Value	=	0.013

Calculations

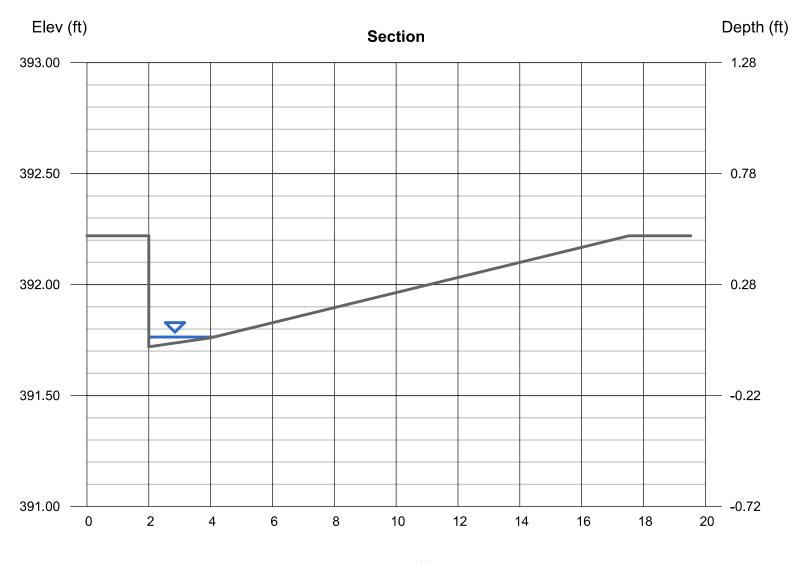
Gutter

Compute by: Known Q Known Q (cfs) = 0.06

Highlighted Depth (ft) = 0.04 Q (cfs) = 0.060 Area (sqft) = 0.05 Velocity (ft/s) = 1.24

Velocity (ft/s) = 1.24Wetted Perim (ft) = 2.16Crit Depth, Yc (ft) = 0.06Spread Width (ft) = 2.12

EGL (ft) = 0.07



Reach (ft)

Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

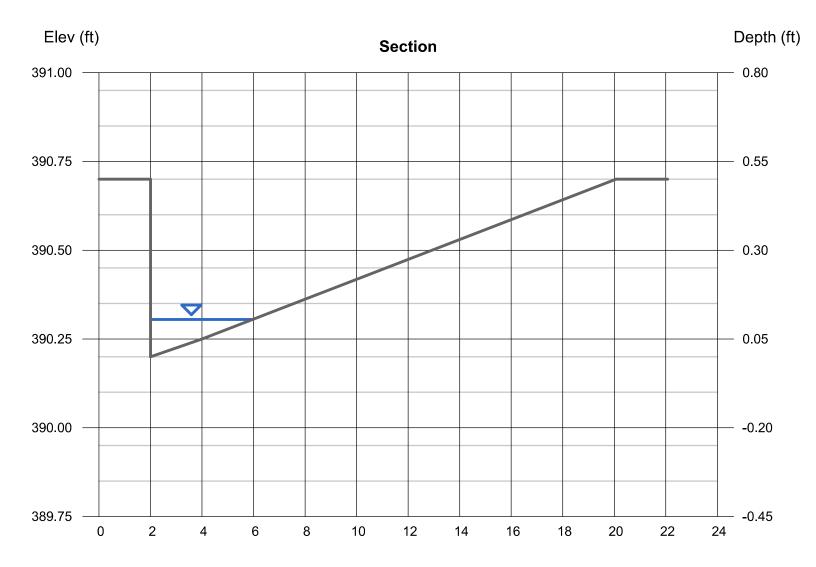
Monday, Jan 15 2024

Gutter Spread Plan West

Gutter		
Cross SI, Sx (ft/ft)	=	0.028
Cross SI, Sw (ft/ft)	=	0.025
Gutter Width (ft)	=	2.00
Invert Elev (ft)	=	390.20
Slope (%)	=	1.25
N-Value	=	0.013

Calculations

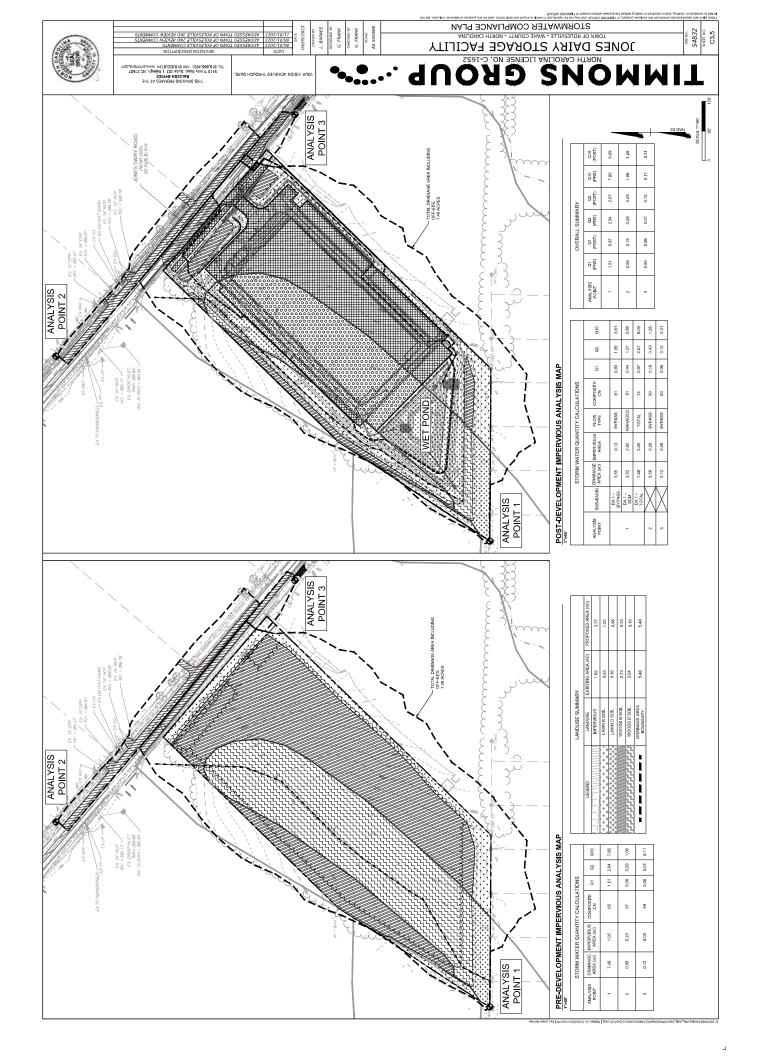
Compute by: Known Q Known Q (cfs) = 0.46 Highlighted Depth (ft) = 0.11Q (cfs) = 0.460Area (sqft) = 0.21Velocity (ft/s) = 2.15Wetted Perim (ft) = 4.07Crit Depth, Yc (ft) = 0.13Spread Width (ft) = 3.96EGL (ft) = 0.18



Reach (ft)

PRE/POST STORMWATER MAP





PRE/POST DRAINAGE AREA CALCULATIONS



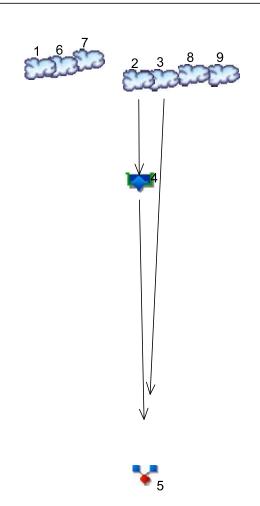
Hydraflow Table of Contents

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Monday, 01 / 29 / 2024

Hydrograph Return Period Recap	2
1 - Year	
Summary Report	3
Hydrograph Reports	4
Hydrograph No. 1, SCS Runoff, Analysis Point 1_PRE	4
TR-55 Tc Worksheet	5
Hydrograph No. 2, SCS Runoff, Analysis Point 1_POST SCM	6
Hydrograph No. 3, SCS Runoff, Analysis Point 1_POST BYPASS	. 7
Hydrograph No. 4, Reservoir, Wetpond	
Hydrograph No. 5, Combine, POST SITE	
Hydrograph No. 6, SCS Runoff, Analysis Point 2_PRE	
Hydrograph No. 7, SCS Runoff, Analysis Point 3_PRE	
Hydrograph No. 8, SCS Runoff, Analysis Point 2_POST	12
Hydrograph No. 9, SCS Runoff, Analysis Point 3_POST	13
2 - Year	4 4
Summary Report	14 15
Hydrograph Reports	15 15
Hydrograph No. 2, SCS Runoff, Analysis Point 1_POST SCM	16
Hydrograph No. 3, SCS Runoff, Analysis Point 1_POST BYPASS	17
Hydrograph No. 4, Reservoir, Wetpond	
Hydrograph No. 5, Combine, POST SITE	
Hydrograph No. 6, SCS Runoff, Analysis Point 2_PRE	
Hydrograph No. 7, SCS Runoff, Analysis Point 3 PRE	21
Hydrograph No. 8, SCS Runoff, Analysis Point 2_POST	
Hydrograph No. 9, SCS Runoff, Analysis Point 3_POST	
1 yaragraph 116. 6, 666 hanon, / waryolo 1 emit 6_1 66 him.	
10 - Year	
Summary Report	
Hydrograph Reports	25
Hydrograph No. 1, SCS Runoff, Analysis Point 1_PRE	
Hydrograph No. 2, SCS Runoff, Analysis Point 1_POST SCM	
Hydrograph No. 3, SCS Runoff, Analysis Point 1_POST BYPASS	
Hydrograph No. 4, Reservoir, Wetpond	
Hydrograph No. 5, Combine, POST SITE	
Hydrograph No. 6, SCS Runoff, Analysis Point 2_PRE	
Hydrograph No. 7, SCS Runoff, Analysis Point 3_PRE	
Hydrograph No. 8, SCS Runoff, Analysis Point 2_POST	
Hydrograph No. 9, SCS Runoff, Analysis Point 3_POST	33
IDF Report 3	34

Watershed Model Schematic



Legend

<u>Hyd.</u>	<u>Origin</u>	<u>Description</u>
1	SCS Runoff	Analysis Point 1_PRE
2	SCS Runoff	Analysis Point 1_POST SCM
3	SCS Runoff	Analysis Point 1_POST BYPASS
4	Reservoir	Wetpond
5	Combine	POST SITE
6	SCS Runoff	Analysis Point 2_PRE
7	SCS Runoff	Analysis Point 3_PRE
8	SCS Runoff	Analysis Point 2_POST
9	SCS Runoff	Analysis Point 3_POST

Project: Rolesville Stormwater Compliance_NCDOT.gpw

Monday, 01 / 29 / 2024

Hydrograph Return Period Recap

	Hydrograph	Inflow hyd(s)	Peak Outflow (cfs)							Hydrograph		
No.	type (origin)		1-yr	2-yr	3-yr	5 - yr	10-yr	25-yr	50 - yr	100 - yr	Description	
1	SCS Runoff		2.063	4.009			10.71			23.44	Analysis Point 1_PRE	
2	SCS Runoff		10.85	13.85			21.66			33.79	Analysis Point 1_POST SCM	
3	SCS Runoff		0.704	1.658			5.251			12.38	Analysis Point 1_POST BYPASS	
4	Reservoir	2	0.437	1.213			2.863			10.53	Wetpond	
5	Combine	3, 4	0.745	2.255			7.737			20.55	POST SITE	
3	SCS Runoff		0.089	0.289			1.062			2.617	Analysis Point 2_PRE	
7	SCS Runoff		0.001	0.005			0.105			0.370	Analysis Point 3_PRE	
8	SCS Runoff		0.178	0.432			1.279			2.923	Analysis Point 2_POST	
9	SCS Runoff		0.061	0.120			0.310			0.667	Analysis Point 3_POST	

Proj. file: Rolesville Stormwater Compliance_NCDOT.gpw

Monday, 01 / 29 / 2024

Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

2 SCS Runoff 10.85 2 720 28,557 Analysis Point 1_POST SCM 3 SCS Runoff 0.704 2 728 4,036 Analysis Point 1_POST BYPASS 4 Reservoir 0.437 2 836 18,230 2 376.98 19,520 Wetpond 5 Combine 0.745 2 728 22,267 3,4 POST SITE 6 SCS Runoff 0.089 2 720 405 Analysis Point 2_PRE 7 SCS Runoff 0.001 2 902 22 Analysis Point 2_POST 8 SCS Runoff 0.178 2 720 561 Analysis Point 2_POST	Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
3 SCS Runoff 0.704 2 728 4,036 Analysis Point 1_POST BYPASS 4 Reservoir 0.437 2 836 18,230 2 376.98 19,520 Wetpond 5 Combine 0.745 2 728 22,267 3,4 POST SITE 6 SCS Runoff 0.089 2 720 405 Analysis Point 2_PRE 7 SCS Runoff 0.001 2 902 22 Analysis Point 3_PRE 8 SCS Runoff 0.178 2 720 561 Analysis Point 2_POST	1	SCS Runoff	2.063	2	734	12,044				Analysis Point 1_PRE
4 Reservoir 0.437 2 836 18,230 2 376.98 19,520 Wetpond 5 Combine 0.745 2 728 22,267 3,4 POST SITE 6 SCS Runoff 0.089 2 720 405 Analysis Point 2_PRE 7 SCS Runoff 0.001 2 902 22 Analysis Point 3_PRE 8 SCS Runoff 0.178 2 720 561 Analysis Point 2_POST	2	SCS Runoff	10.85	2	720	28,557				Analysis Point 1_POST SCM
5 Combine 0.745 2 728 22,267 3, 4 POST SITE 6 SCS Runoff 0.089 2 720 405 Analysis Point 2_PRE 7 SCS Runoff 0.001 2 902 22 Analysis Point 3_PRE 8 SCS Runoff 0.178 2 720 561 Analysis Point 2_POST	3	SCS Runoff	0.704	2	728	4,036				Analysis Point 1_POST BYPASS
6 SCS Runoff 0.089 2 720 405 Analysis Point 2_PRE 7 SCS Runoff 0.001 2 902 22 Analysis Point 3_PRE 8 SCS Runoff 0.178 2 720 561 Analysis Point 2_POST	4	Reservoir	0.437	2	836	18,230	2	376.98	19,520	Wetpond
7 SCS Runoff 0.001 2 902 22 Analysis Point 3_PRE 8 SCS Runoff 0.178 2 720 561 Analysis Point 2_POST	5	Combine	0.745	2	728	22,267	3, 4			POST SITE
8 SCS Runoff 0.178 2 720 561 Analysis Point 2_POST	6	SCS Runoff	0.089	2	720	405				Analysis Point 2_PRE
	7	SCS Runoff	0.001	2	902	22				Analysis Point 3_PRE
9 SCS Runoff 0.061 2 718 153 Analysis Point 3_POST	8	SCS Runoff	0.178	2	720	561				Analysis Point 2_POST
	9	SCS Runoff	0.061	2	718	153				Analysis Point 3_POST

Hydrograph Report

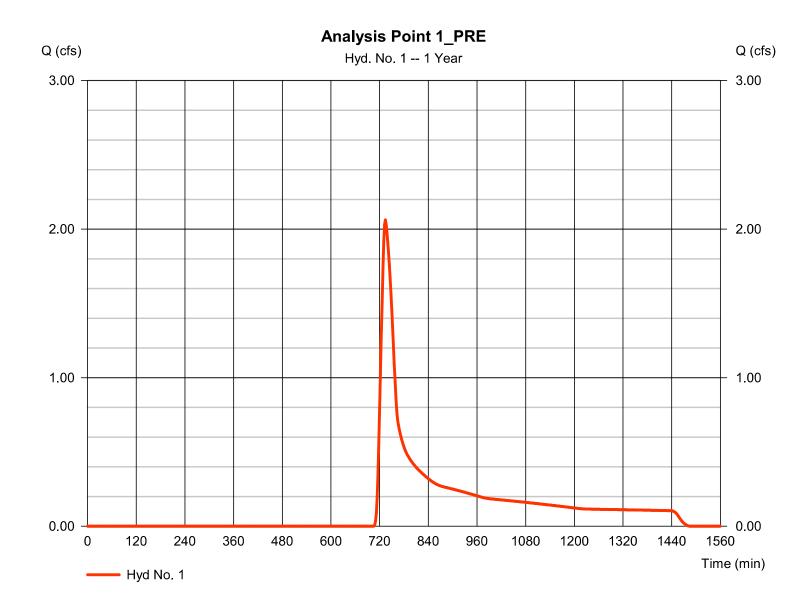
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Monday, 01 / 29 / 2024

Hyd. No. 1

Analysis Point 1_PRE

Hydrograph type Peak discharge = 2.063 cfs= SCS Runoff Storm frequency Time to peak = 734 min = 1 yrsTime interval = 2 min Hyd. volume = 12,044 cuft = 7.480 acCurve number Drainage area = 65 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc) = 28.10 min Tc method = TR55 Total precip. = 2.86 inDistribution = Type II Storm duration = 24 hrs = 484 Shape factor



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 1Analysis Point 1_PRE

<u>Description</u>	A		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.46 = 2.93 = 17.73	+	0.011 0.0 0.00 0.00	+	0.011 0.0 0.00 0.00	=	17.73
, ,	- 17.70	•	0.00	•	0.00	_	17.70
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 1315.00 = 1.71 = Unpaved =2.11	i	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 10.39	+	0.00	+	0.00	=	10.39
Channel Flow							
X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015		0.00 0.00 0.00 0.015		
					0.00		
Flow length (ft)	0.0({0})		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							28.10 min

Hydrograph Report

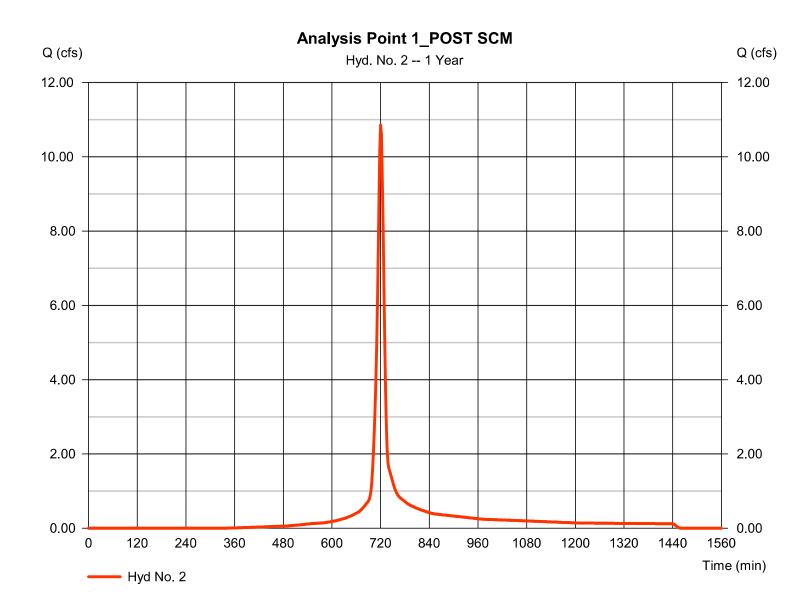
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Monday, 01 / 29 / 2024

Hyd. No. 2

Analysis Point 1_POST SCM

Hydrograph type Peak discharge = SCS Runoff = 10.85 cfsStorm frequency Time to peak = 720 min = 1 yrsTime interval = 2 min Hyd. volume = 28,557 cuft Drainage area Curve number = 3.930 ac= 91 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method Time of conc. (Tc) = 13.20 min = User Total precip. = 2.86 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Q (cfs)

Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

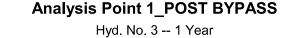
Monday, 01 / 29 / 2024

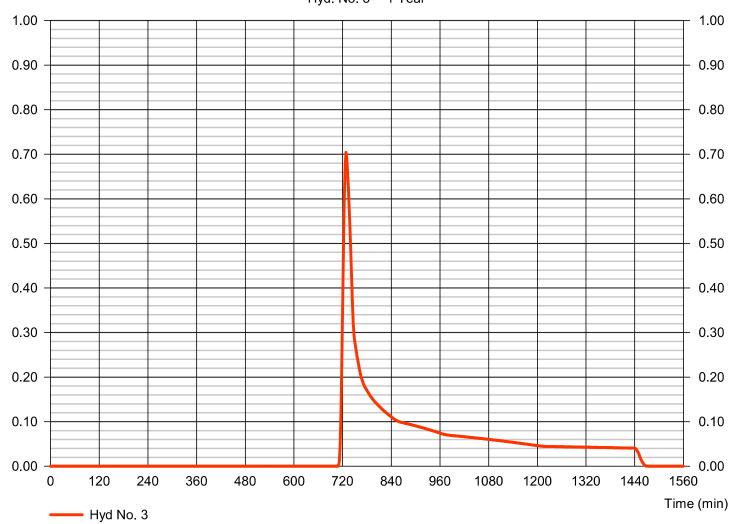
Hyd. No. 3

Q (cfs)

Analysis Point 1_POST BYPASS

Hydrograph type Peak discharge = SCS Runoff = 0.704 cfsStorm frequency Time to peak = 728 min = 1 yrsTime interval = 2 min Hyd. volume = 4.036 cuftDrainage area Curve number = 3.550 ac= 61 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method Time of conc. (Tc) $= 20.00 \, \text{min}$ = User Total precip. Distribution = Type II = 2.86 inStorm duration = 24 hrs Shape factor = 484





Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

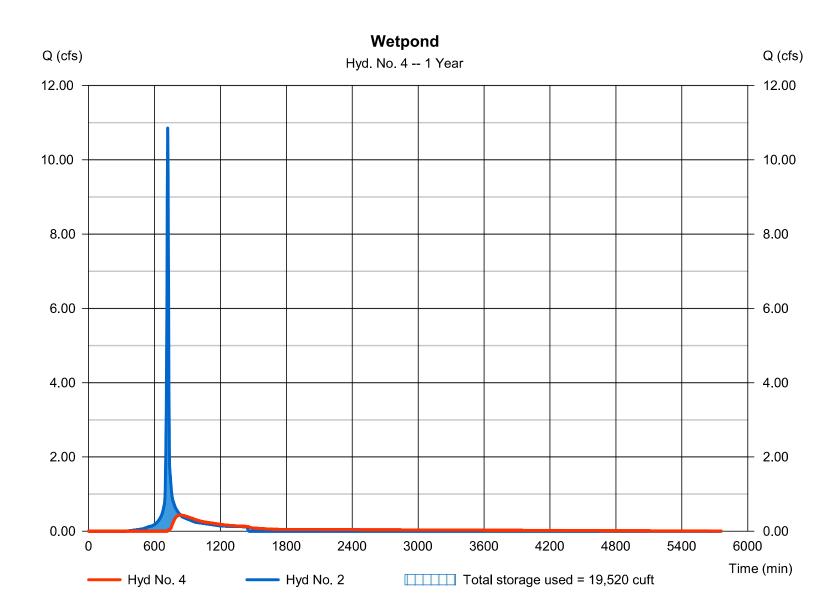
Monday, 01 / 29 / 2024

Hyd. No. 4

Wetpond

Hydrograph type Peak discharge = Reservoir = 0.437 cfsStorm frequency Time to peak = 836 min = 1 yrsHyd. volume Time interval = 2 min = 18,230 cuftInflow hyd. No. = 2 - Analysis Point 1_POST SCMax. Elevation = 376.98 ftMax. Storage = Wet Pond Reservoir name = 19,520 cuft

Storage Indication method used.



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Monday, 01 / 29 / 2024

= 0.745 cfs

= 728 min

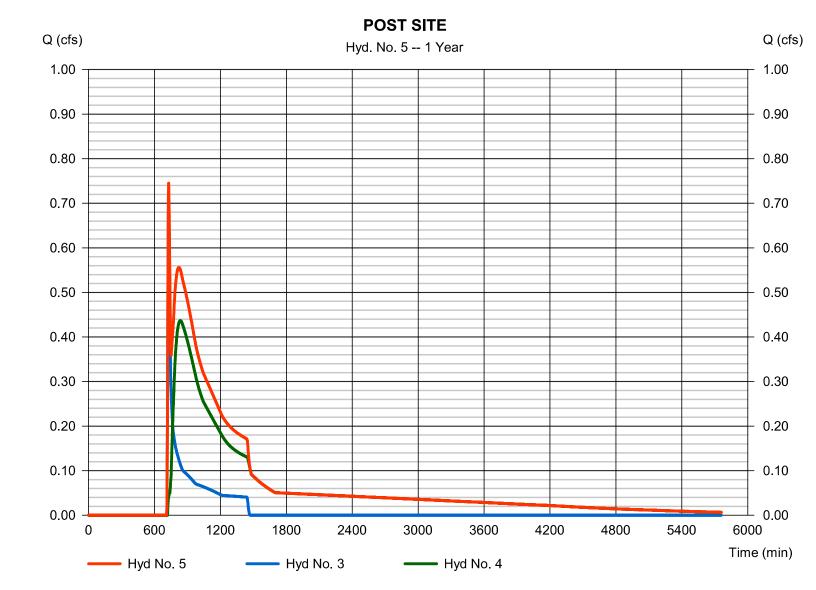
= 3.550 ac

= 22,267 cuft

Hyd. No. 5

POST SITE

Hydrograph type= CombinePeak dischargeStorm frequency= 1 yrsTime to peakTime interval= 2 minHyd. volumeInflow hyds.= 3, 4Contrib. drain. area



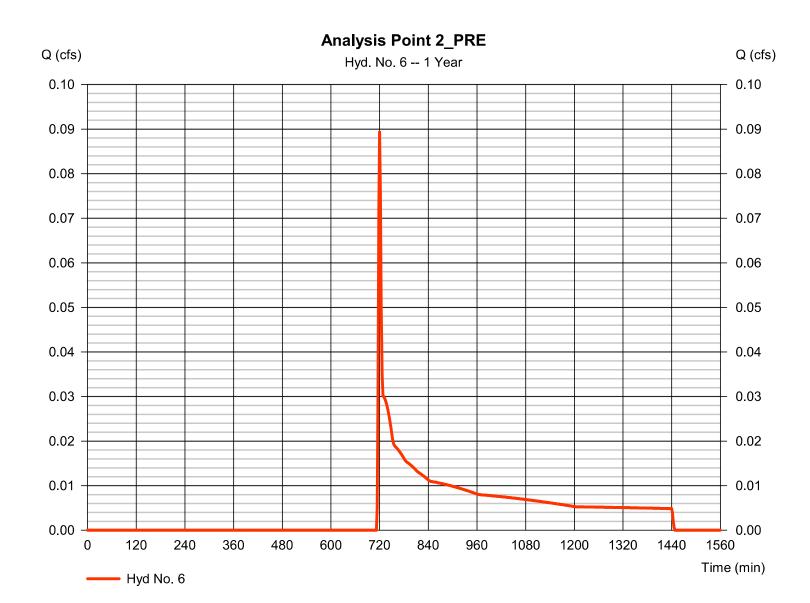
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Monday, 01 / 29 / 2024

Hyd. No. 6

Analysis Point 2_PRE

Hydrograph type Peak discharge = SCS Runoff = 0.089 cfsStorm frequency Time to peak = 720 min = 1 yrsTime interval = 2 min Hyd. volume = 405 cuft Drainage area Curve number = 0.580 ac= 57 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc) $= 5.00 \, \text{min}$ Tc method = User Total precip. Distribution = Type II = 2.86 inStorm duration = 24 hrs = 484 Shape factor



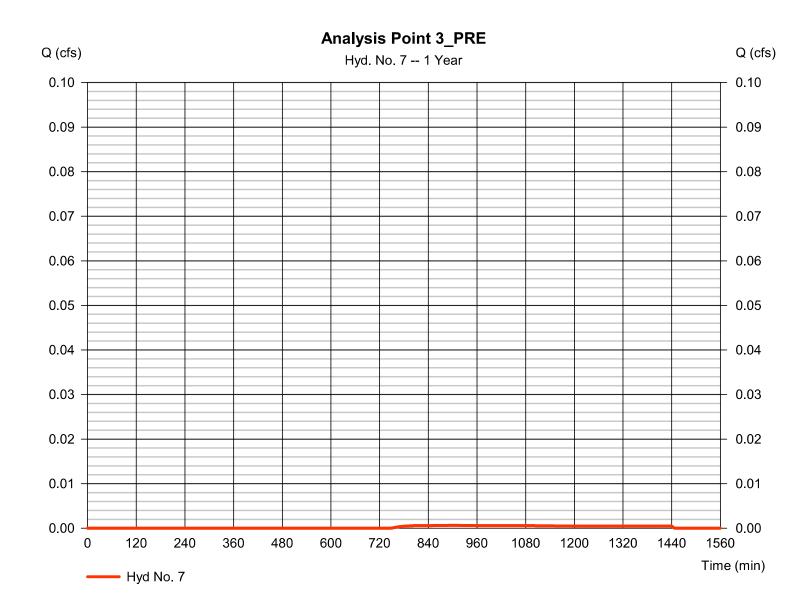
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Monday, 01 / 29 / 2024

Hyd. No. 7

Analysis Point 3_PRE

Hydrograph type Peak discharge = SCS Runoff = 0.001 cfsStorm frequency Time to peak = 902 min = 1 yrsTime interval = 2 min Hyd. volume = 22 cuft Drainage area Curve number = 0.120 ac= 49 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method Time of conc. (Tc) $= 5.00 \, \text{min}$ = User Total precip. Distribution = Type II = 2.86 inStorm duration = 24 hrs Shape factor = 484



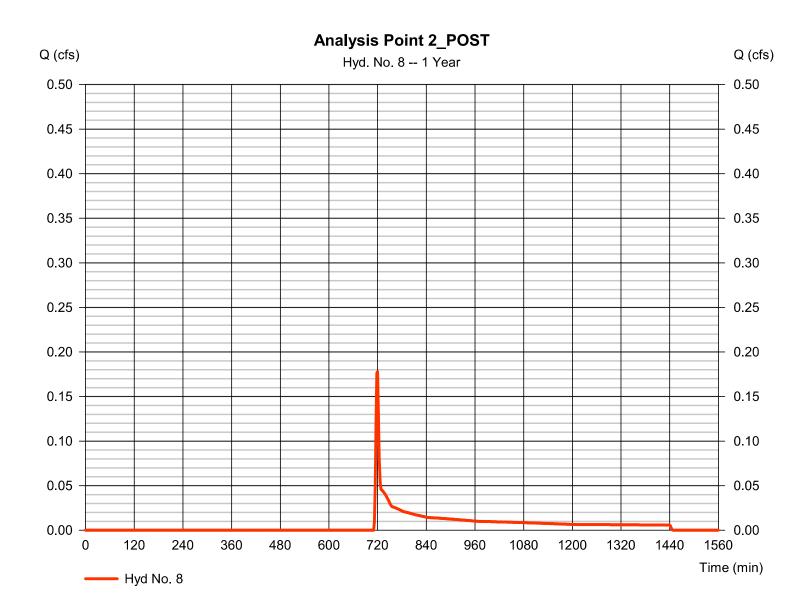
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Monday, 01 / 29 / 2024

Hyd. No. 8

Analysis Point 2_POST

Hydrograph type Peak discharge = SCS Runoff = 0.178 cfsStorm frequency Time to peak = 720 min = 1 yrsTime interval = 2 min Hyd. volume = 561 cuft Drainage area Curve number = 0.580 ac= 60 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc) $= 5.00 \, \text{min}$ Tc method = User Total precip. Distribution = Type II = 2.86 inStorm duration = 24 hrs = 484 Shape factor



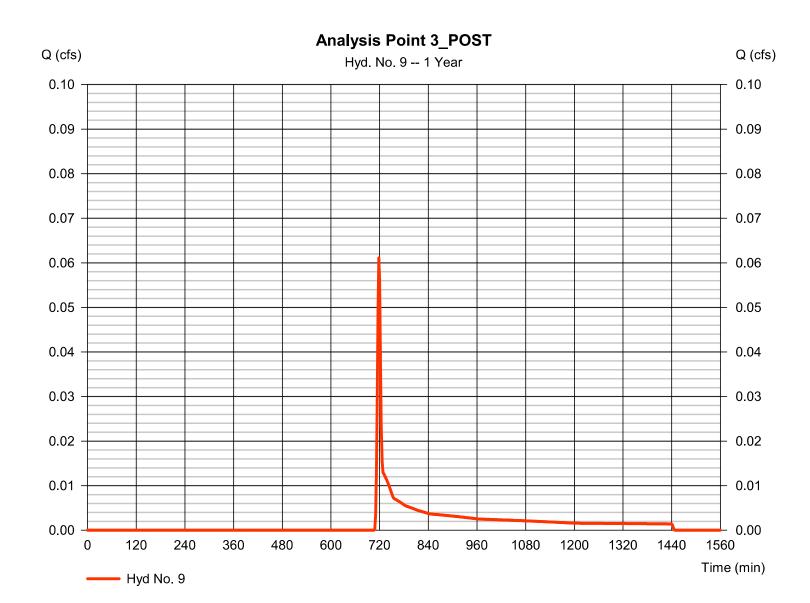
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Monday, 01 / 29 / 2024

Hyd. No. 9

Analysis Point 3_POST

Hydrograph type Peak discharge = SCS Runoff = 0.061 cfsStorm frequency Time to peak = 718 min = 1 yrsTime interval = 2 min Hyd. volume = 153 cuft Drainage area Curve number = 0.120 ac= 63 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc) $= 5.00 \, \text{min}$ Tc method = User Total precip. Distribution = Type II = 2.86 inStorm duration = 24 hrs = 484 Shape factor



Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

lyd. Io.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	4.009	2	732	19,850				Analysis Point 1_PRE
2	SCS Runoff	13.85	2	720	36,827				Analysis Point 1_POST SCM
3	SCS Runoff	1.658	2	728	7,150				Analysis Point 1_POST BYPASS
4	Reservoir	1.213	2	758	26,493	2	377.24	22,234	Wetpond
5	Combine	2.255	2	732	33,643	3, 4			POST SITE
6	SCS Runoff	0.289	2	718	791				Analysis Point 2_PRE
7	SCS Runoff	0.005	2	722	66				Analysis Point 3_PRE
8	SCS Runoff	0.432	2	718	1,015				Analysis Point 2_POST
9	SCS Runoff	0.120	2	718	261				Analysis Point 3_POST
	esville Storm	-10	<u> </u>	NODOT				Monday, 0	

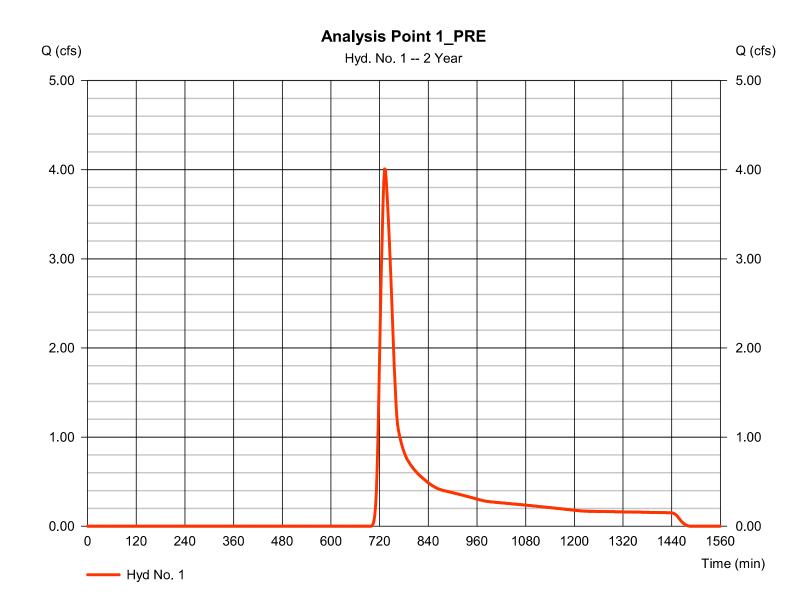
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Monday, 01 / 29 / 2024

Hyd. No. 1

Analysis Point 1_PRE

Hydrograph type = SCS Runoff Peak discharge = 4.009 cfsStorm frequency Time to peak = 732 min = 2 yrsTime interval = 2 min Hyd. volume = 19,850 cuftCurve number Drainage area = 7.480 ac= 65 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc) = 28.10 min Tc method = TR55 Total precip. Distribution = Type II = 3.46 inStorm duration = 24 hrs = 484 Shape factor



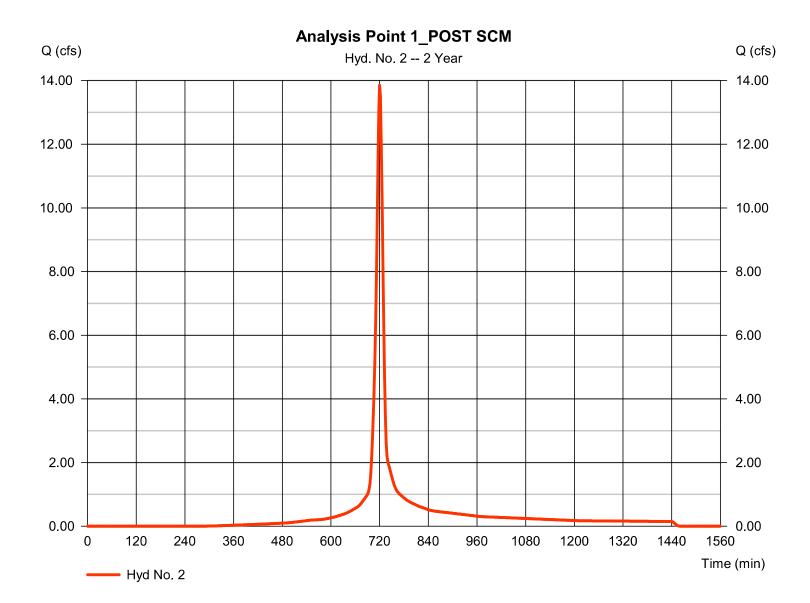
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Monday, 01 / 29 / 2024

Hyd. No. 2

Analysis Point 1_POST SCM

Peak discharge Hydrograph type = SCS Runoff = 13.85 cfsStorm frequency Time to peak = 720 min = 2 yrsTime interval = 2 min Hyd. volume = 36,827 cuft Curve number Drainage area = 3.930 ac= 91 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc) Tc method = User $= 13.20 \, \text{min}$ Total precip. = 3.46 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



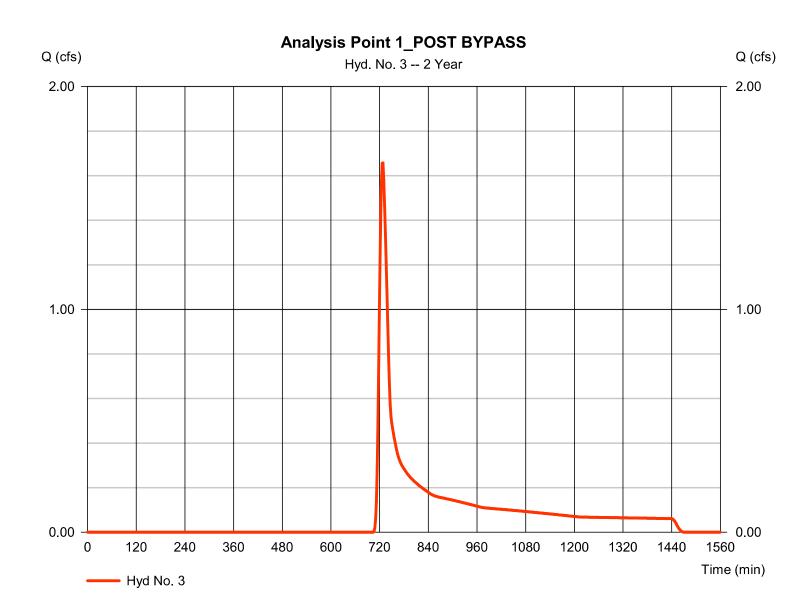
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Monday, 01 / 29 / 2024

Hyd. No. 3

Analysis Point 1_POST BYPASS

Hydrograph type Peak discharge = SCS Runoff = 1.658 cfsStorm frequency Time to peak = 728 min = 2 yrsTime interval = 2 min Hyd. volume = 7,150 cuftDrainage area = 3.550 acCurve number = 61 = 0 ftBasin Slope = 0.0 % Hydraulic length Tc method Time of conc. (Tc) $= 20.00 \, \text{min}$ = User Total precip. = 3.46 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

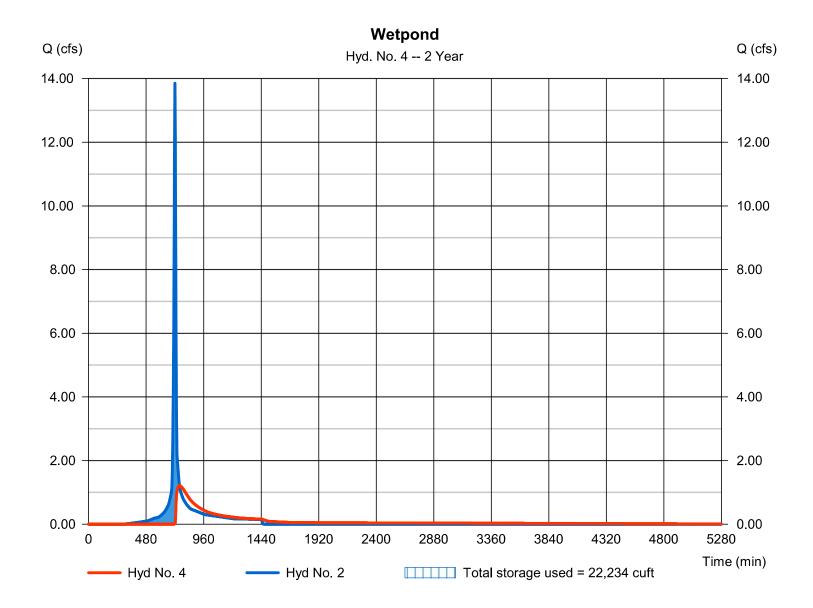
Monday, 01 / 29 / 2024

Hyd. No. 4

Wetpond

Hydrograph type = Reservoir Peak discharge = 1.213 cfsStorm frequency = 2 yrsTime to peak = 758 min Time interval = 2 min Hyd. volume = 26,493 cuft = 2 - Analysis Point 1_POST SCMax. Elevation = 377.24 ftInflow hyd. No. = Wet Pond Reservoir name Max. Storage = 22,234 cuft

Storage Indication method used.



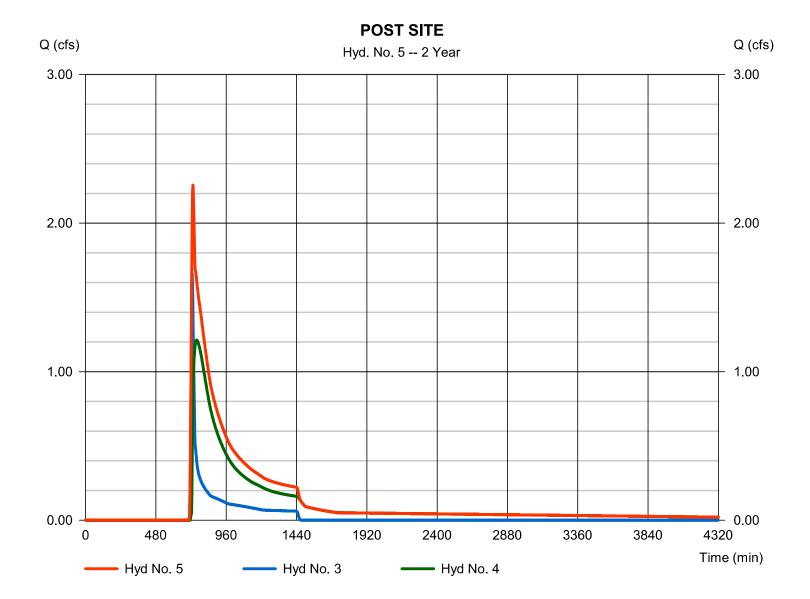
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Monday, 01 / 29 / 2024

Hyd. No. 5

POST SITE

Hydrograph type = Combine Peak discharge = 2.255 cfsStorm frequency = 2 yrs Time to peak = 732 min Time interval = 2 min Hyd. volume = 33,643 cuftInflow hyds. = 3, 4 Contrib. drain. area = 3.550 ac



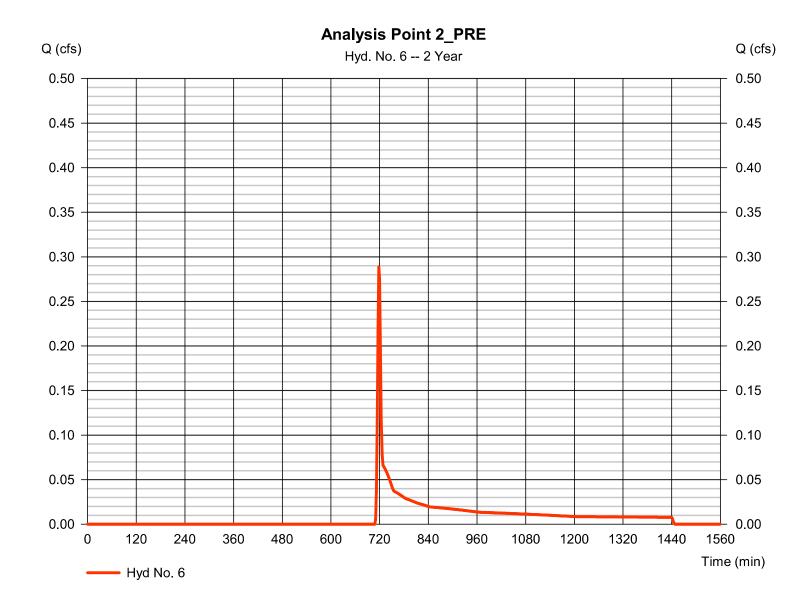
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Monday, 01 / 29 / 2024

Hyd. No. 6

Analysis Point 2_PRE

Hydrograph type Peak discharge = SCS Runoff = 0.289 cfsStorm frequency Time to peak = 718 min = 2 yrsTime interval = 2 min Hyd. volume = 791 cuft Curve number Drainage area = 0.580 ac= 57 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc) $= 5.00 \, \text{min}$ Tc method = User Total precip. Distribution = Type II = 3.46 inStorm duration = 24 hrs = 484 Shape factor



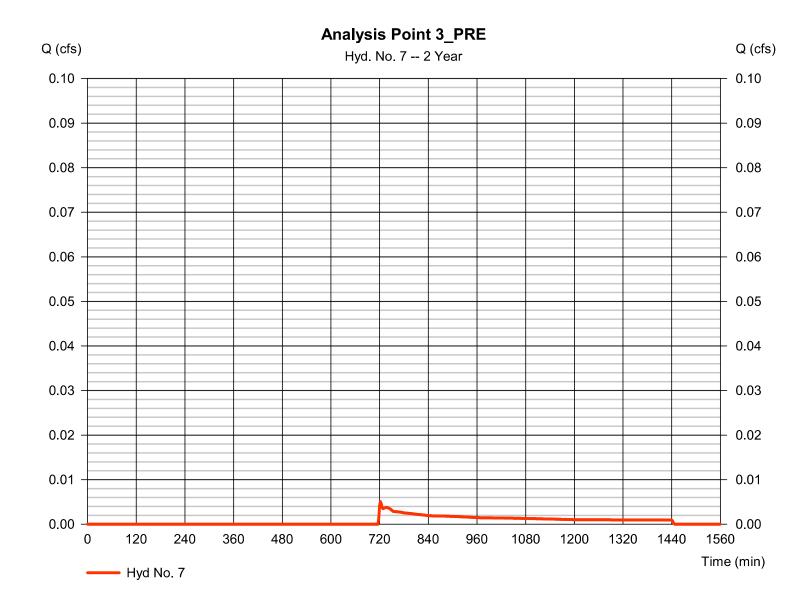
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Monday, 01 / 29 / 2024

Hyd. No. 7

Analysis Point 3_PRE

Hydrograph type Peak discharge = SCS Runoff = 0.005 cfsStorm frequency Time to peak = 722 min = 2 yrsTime interval = 2 min Hyd. volume = 66 cuft Drainage area Curve number = 0.120 ac= 49 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc) $= 5.00 \, \text{min}$ Tc method = User Total precip. Distribution = Type II = 3.46 inStorm duration = 24 hrs Shape factor = 484



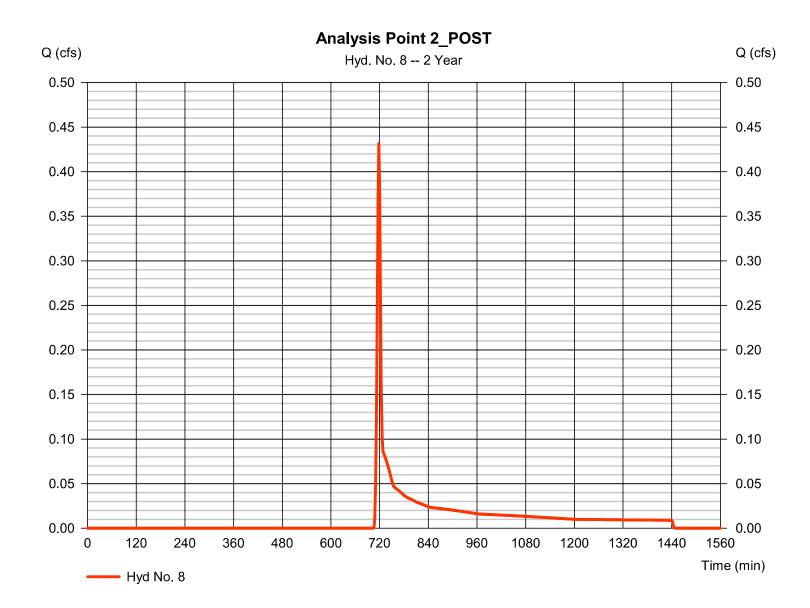
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Monday, 01 / 29 / 2024

Hyd. No. 8

Analysis Point 2_POST

Hydrograph type Peak discharge = SCS Runoff = 0.432 cfsStorm frequency Time to peak = 718 min = 2 yrsTime interval = 2 min Hyd. volume = 1,015 cuft Drainage area Curve number = 0.580 ac= 60 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc) $= 5.00 \, \text{min}$ Tc method = User Total precip. Distribution = Type II = 3.46 inStorm duration = 24 hrs = 484 Shape factor



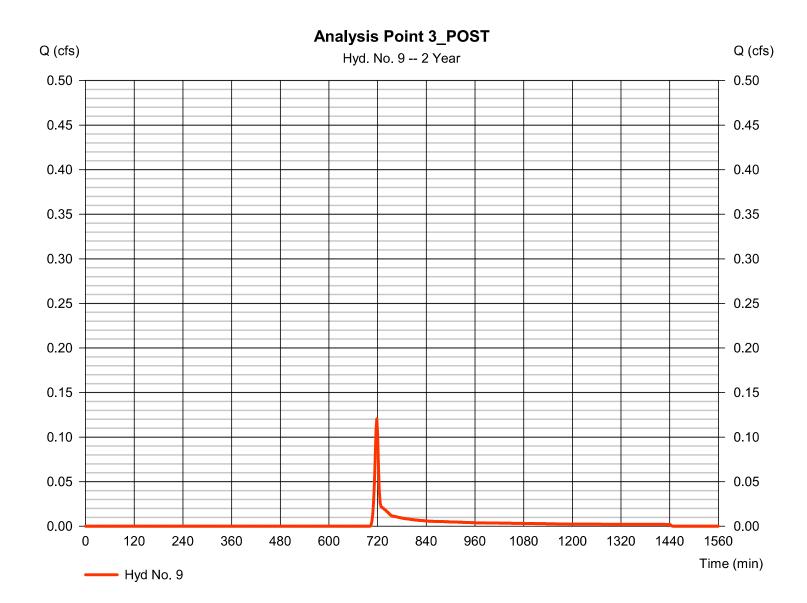
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Monday, 01 / 29 / 2024

Hyd. No. 9

Analysis Point 3_POST

Hydrograph type Peak discharge = SCS Runoff = 0.120 cfsStorm frequency Time to peak = 718 min = 2 yrsTime interval = 2 min Hyd. volume = 261 cuft Drainage area Curve number = 0.120 ac= 63 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc) $= 5.00 \, \text{min}$ Tc method = User Total precip. Distribution = Type II = 3.46 inStorm duration = 24 hrs = 484 Shape factor



Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

lyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	10.71	2	732	45,440				Analysis Point 1_PRE
2	SCS Runoff	21.66	2	720	59,012				Analysis Point 1_POST SCM
3	SCS Runoff	5.251	2	726	17,876				Analysis Point 1_POST BYPASS
4	Reservoir	2.863	2	744	48,663	2	378.33	34,009	Wetpond
5	Combine	7.737	2	726	66,539	3, 4			POST SITE
6	SCS Runoff	1.062	2	718	2,212				Analysis Point 2_PRE
7	SCS Runoff	0.105	2	718	266				Analysis Point 3_PRE
8	SCS Runoff	1.279	2	718	2,603				Analysis Point 2_POST
9	SCS Runoff	0.310	2	718	624				Analysis Point 3_POST

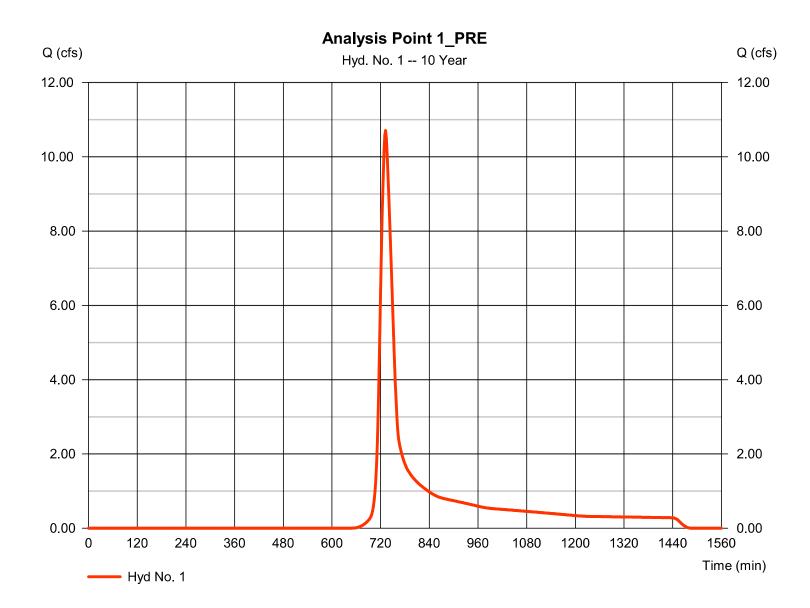
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Monday, 01 / 29 / 2024

Hyd. No. 1

Analysis Point 1_PRE

Hydrograph type = SCS Runoff Peak discharge = 10.71 cfsStorm frequency = 10 yrsTime to peak = 732 min Time interval = 2 min Hyd. volume = 45,440 cuft Drainage area = 7.480 acCurve number = 65 = 0.0 % Basin Slope Hydraulic length = 0 ftTime of conc. (Tc) = 28.10 min Tc method = TR55 Total precip. = 5.03 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



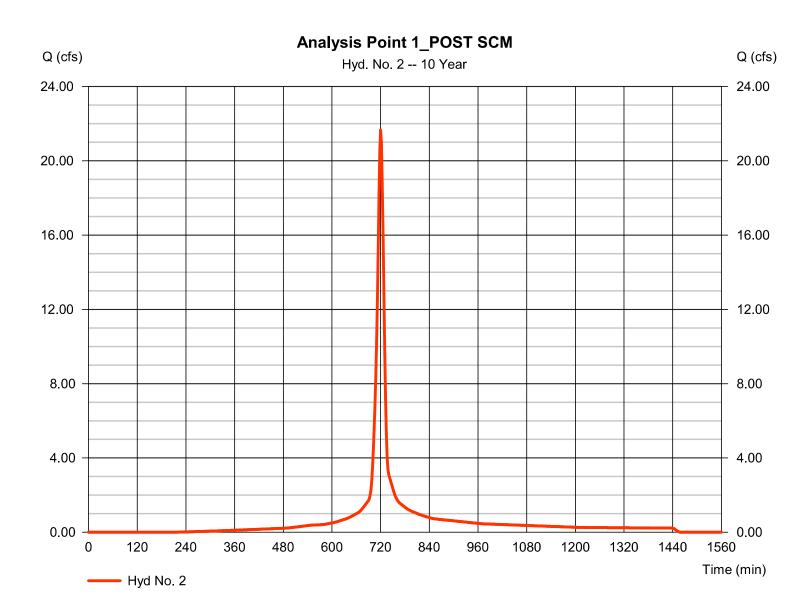
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Monday, 01 / 29 / 2024

Hyd. No. 2

Analysis Point 1 POST SCM

Peak discharge Hydrograph type = SCS Runoff = 21.66 cfsStorm frequency = 10 yrsTime to peak = 720 min Time interval = 2 min Hyd. volume = 59,012 cuftCurve number = 91 Drainage area = 3.930 acBasin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc) Tc method = User $= 13.20 \, \text{min}$ Total precip. = 5.03 inDistribution = Type II Storm duration = 24 hrs = 484 Shape factor



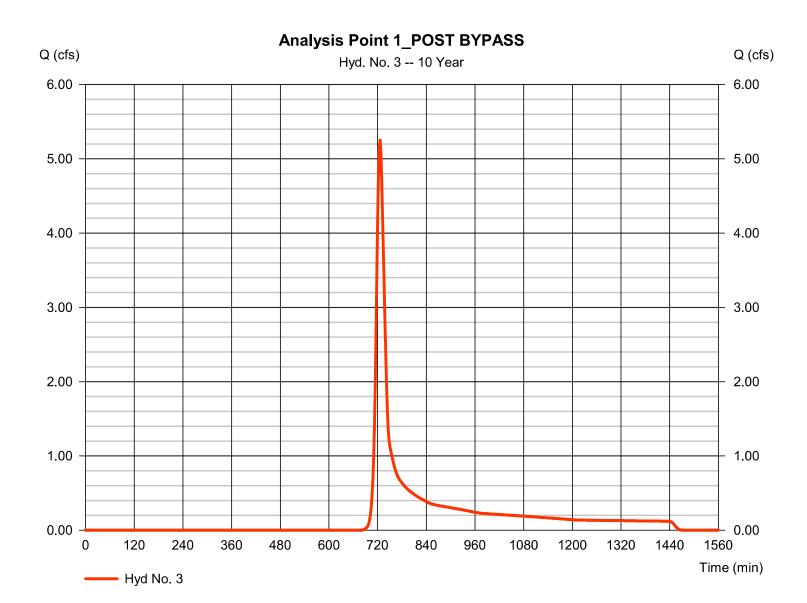
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Monday, 01 / 29 / 2024

Hyd. No. 3

Analysis Point 1_POST BYPASS

= SCS Runoff Peak discharge = 5.251 cfsHydrograph type Storm frequency Time to peak = 726 min = 10 yrsTime interval = 2 min Hyd. volume = 17,876 cuftCurve number Drainage area = 3.550 ac= 61 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc) $= 20.00 \, \text{min}$ Tc method = User Total precip. = 5.03 inDistribution = Type II Storm duration = 24 hrs = 484 Shape factor



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

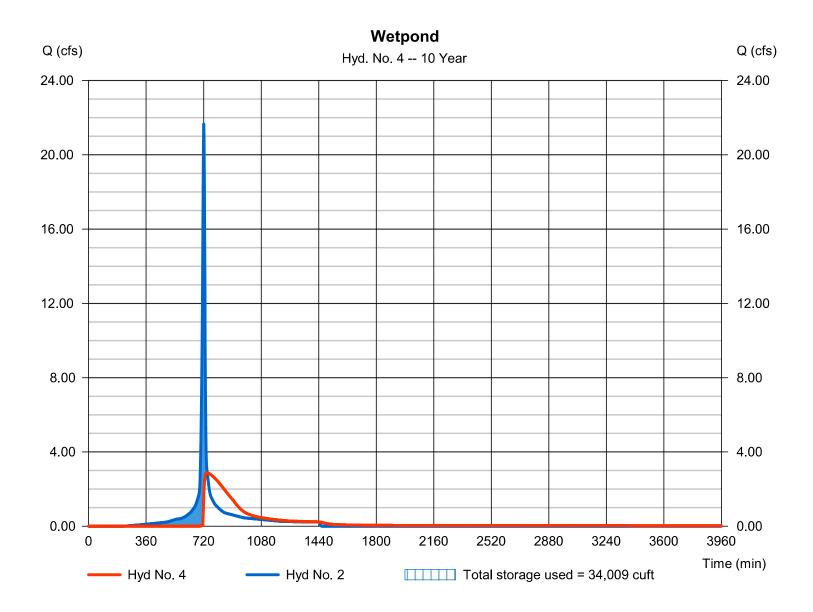
Monday, 01 / 29 / 2024

Hyd. No. 4

Wetpond

Hydrograph type = Reservoir Peak discharge = 2.863 cfsStorm frequency = 10 yrsTime to peak = 744 min Hyd. volume Time interval = 2 min = 48,663 cuft= 2 - Analysis Point 1_POST SCMax. Elevation Inflow hyd. No. = 378.33 ft= Wet Pond Reservoir name Max. Storage = 34,009 cuft

Storage Indication method used.



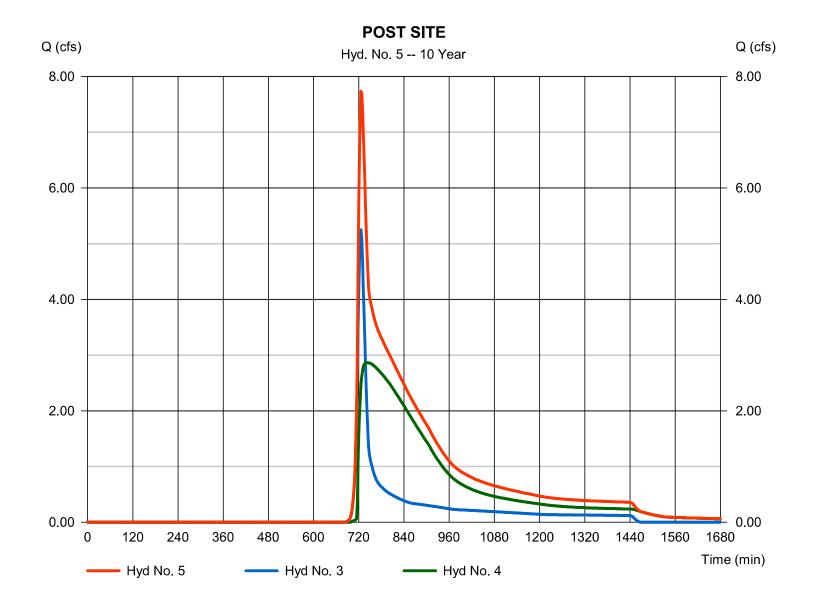
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Monday, 01 / 29 / 2024

Hyd. No. 5

POST SITE

Hydrograph type = Combine Storm frequency = 10 yrs Time interval = 2 min Inflow hyds. = 3, 4 Peak discharge = 7.737 cfs
Time to peak = 726 min
Hyd. volume = 66,539 cuft
Contrib. drain. area = 3.550 ac



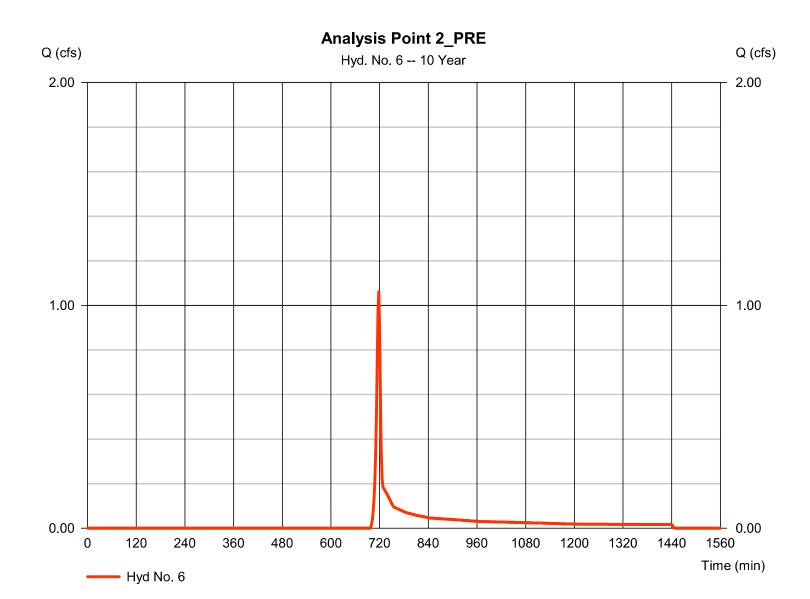
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Monday, 01 / 29 / 2024

Hyd. No. 6

Analysis Point 2_PRE

Hydrograph type = SCS Runoff Peak discharge = 1.062 cfsStorm frequency = 10 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 2,212 cuft Drainage area Curve number = 0.580 ac= 57 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc) $= 5.00 \, \text{min}$ Tc method = User Total precip. = 5.03 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



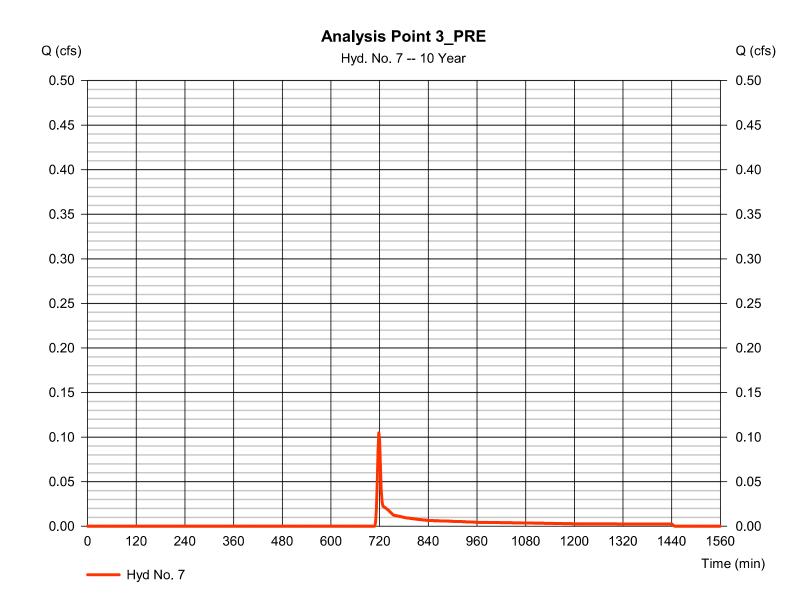
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Monday, 01 / 29 / 2024

Hyd. No. 7

Analysis Point 3_PRE

Hydrograph type = SCS Runoff Peak discharge = 0.105 cfsStorm frequency = 10 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 266 cuft Drainage area Curve number = 0.120 ac= 49 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc) $= 5.00 \, \text{min}$ Tc method = User Total precip. = 5.03 inDistribution = Type II Storm duration = 24 hrs = 484 Shape factor



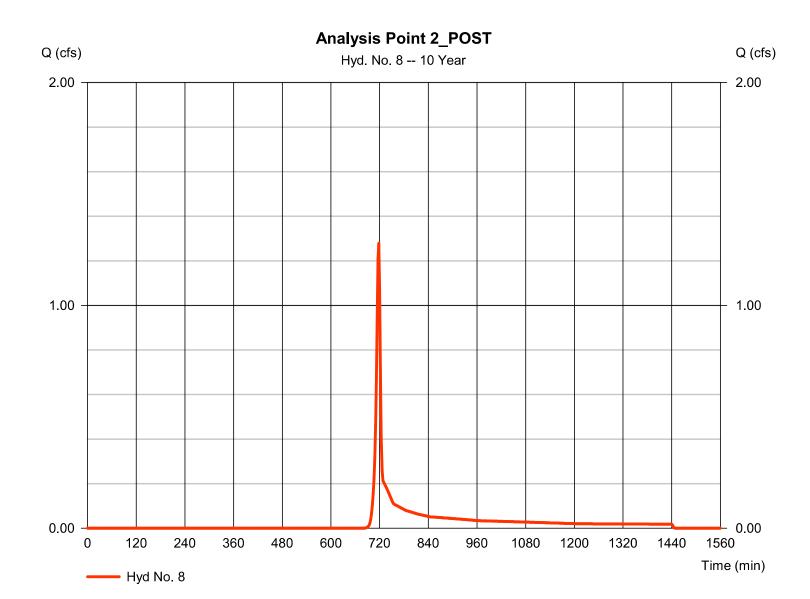
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Monday, 01 / 29 / 2024

Hyd. No. 8

Analysis Point 2_POST

Hydrograph type = SCS Runoff Peak discharge = 1.279 cfsStorm frequency = 10 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 2,603 cuftDrainage area Curve number = 0.580 ac= 60 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc) $= 5.00 \, \text{min}$ Tc method = User Total precip. = 5.03 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



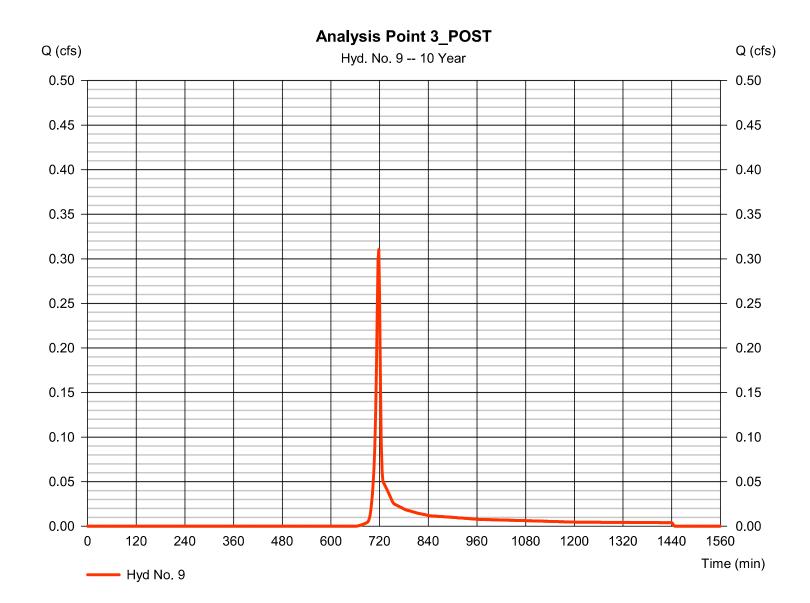
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Monday, 01 / 29 / 2024

Hyd. No. 9

Analysis Point 3_POST

Hydrograph type Peak discharge = SCS Runoff = 0.310 cfsStorm frequency = 10 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 624 cuft Curve number Drainage area = 0.120 ac= 63 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc) $= 5.00 \, \text{min}$ Tc method = User Total precip. = 5.03 inDistribution = Type II Storm duration = 24 hrs = 484 Shape factor



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Monday, 01 / 29 / 2024

Return Period	Intensity-Duration-Frequency Equation Coefficients (FHA)									
(Yrs)	В	D	E	(N/A)						
1	67.6239	13.2000	0.9048							
2	0.0000	0.0000	0.0000							
3	0.0000	0.0000	0.0000							
5	0.0000	0.0000	0.0000							
10	68.3177	11.9000	0.7902							
25	62.3903	11.0000	0.7384							
50	0.0000	0.0000	0.0000							
100	0.0000	0.0000	0.0000							

File name: Town of Rolesville.IDF

Intensity = B / (Tc + D)^E

Return	Intensity Values (in/hr)													
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60		
1	4.90	3.93	3.30	2.84	2.50	2.24	2.03	1.86	1.71	1.59	1.48	1.39		
2	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
5	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
10	7.32	5.96	5.07	4.43	3.95	3.57	3.27	3.01	2.80	2.62	2.47	2.33		
25	8.05	6.59	5.63	4.94	4.43	4.02	3.69	3.42	3.19	3.00	2.83	2.68		
50	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
100	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		

Tc = time in minutes. Values may exceed 60.

Precip. file name: S:\333\54832-Rolesville_Self_Stor\Calc\Stm\Town of Rolesville.pcp

		Rainfall Precipitation Table (in)											
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr					
SCS 24-hour	2.86	3.46	0.00	4.40	5.03	5.85	6.56	7.50					
SCS 6-Hr	2.05	2.44	0.00	0.00	3.54	4.10	4.55	5.44					
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00					
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00					
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00					
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00					
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00					
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00					