

# STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

ROY COOPER
GOVERNOR

J.R. "JOEY" HOPKINS
SECRETARY

May 22, 2024

**COUNTY:** Wake

**SUBJECT:** Application for Driveway Entrance onto SR 2053

Permit Number D051-092-23-00187

**Jones Dairy Storage Facility** 

Timmons Group - Garret Frank 5410 Trinity Road Raleigh, NC 27606

Dear Sir or Madam,

Attached for your file is a copy of the Driveway Permit which you requested.

This permit is approved with the understanding the applicant is responsible for the proper construction of the above drive in accordance with the permit application, approved plans and the attached Standard and Special Provisions.

#### FOR ACCESS ONLY

A PERFORMANCE AND INDEMNITY BOND IN THE VALUE OF \$350,000.00 IS REQUIRED AND SHALL BE POSTED WITH THE DISTRICT OFFICE PRIOR TO THE START OF WORK. THE BOND SHALL DISPLAY THE DRIVEWAY PERMIT NUMBER.

Should you have any questions, please contact the District Engineers Office at (919) 814-6115.

Sincerely,

Daniel T. Boulware, P.E.

DTB/mvs

Attachment

cc: Mr. B. H. Jones, P. E., Division Engineer

Town of Rolesville

Mailing Address: NC DEPARTMENT OF TRANSPORTATION DIVISION 5 – DISTRICT 1 1575 MAIL SERVICE CENTER RALEIGH, NC 27699-1575

Telephone: (919) 814-6115 Fax: (919) 715-5778 Customer Service: 1-877-368-4968

Website: www.ncdot.gov

#### **Driveway Permit Special Provision**

- 1. NCDOT WORK ZONE TRAFFIC CONTROL QUALIFICATIONS AND TRAINING PROGRAM:
  - A. Effective July 1, 2010, all flagging operations within NCDOT Right of Way require qualified and trained Work Zone Flaggers.
  - B. Effective July 1, 2011, qualified and trained Work Zone Traffic Control Supervisors will be required on Significant Projects.
  - C. Training for this certification is provided by NCDOT approved training sources and by private entities that have been pre-approved to train themselves. If you have questions, contact our web site at <a href="http://www.ncdot.org/doh/preconstruct/wztc/WZTCTrainingProgram/default.html">http://www.ncdot.org/doh/preconstruct/wztc/WZTCTrainingProgram/default.html</a>, or contact Kenneth Thornewell, PE with NCDOT Work Zone Traffic Control Unit at (919) 662-4338.
- 2. Before work begins, please forward the contact information of the general contractor to the Senior Assistant District Engineer, Montel Sparrow at mvsparrow@ncdot.gov. Include contact name, emergency phone number and email.
- 3. Current and future state projects take precedence over this driveway.
- **4.** Approval is for one full access driveway onto SR 2053 (Jones Dairy Road) located approximately 815 feet east of the intersection with SR 4403 (Winter Spring Drive).
- **5.** NCDOT reserves the right to further restrict this/these access/accesses at the expense of the encroaching party if/when accident history exists and/or operational issues occur.
  - NCDOT reserves the right to revise, restrict, suspend and/or void this driveway permit if the execution and/or operation of said permit is found to be a hazard to the traveling public.
- **6.** The roadway improvements, as shown on the plans, shall be constructed under an encroachment agreement E051-092-23-01269.
- 7. Roadway improvements shall be required as shown on the attached plans.
- 8. A \$350,000.00 Performance and Indemnity Bond shall be executed and posted with the District Office at 4009 District Drive, Raleigh, North Carolina 27607, prior to beginning any work in the Right of Way. When the project has been completed and upon written request by the Permitee to the District Office, an inspection and review will be performed by NCDOT, and if all work is found to be satisfactory, the bond will be released.
- **9.** All lots/outparcels shall be served internally, with no additional accesses onto SR 2503 (Jones Dairy Road). This shall be conveyed in any buy, sell or lease agreement.
- 10. Any future development or attachment will require review and approval by NCDOT. Additional roadway improvements and/or further restriction of existing access points may be required. This will be determined by the traffic conditions at the time of the future development.
- 11. Approval of the driveway permit does not constitute review and approval of streets for NCDOT maintenance. If addition of streets to the state system is desired, plans for review and a petition for addition shall be submitted to the District Engineer's office. For further information, contact the District Engineer's Office, at (919) 814-6115.
- 12. This driveway agreement only covers work within NCDOT Right-of-Way and Control Access Right-of-Way as shown on the attached plans received in the District Office.

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- 13. Notify the Town of Rolesville prior to beginning work.
- 14. Any personnel or equipment working within five feet of a travel lane shall require a full lane closure. No lane of traffic shall be closed or restricted between the hours of 6:00 AM to 9:00 AM and 4:00 PM to 7:00 PM Monday thru Friday, during any time of inclement weather, or upon District Engineers' directive. Traffic shall be maintained at all times. Any violation of these hours will result in termination of the Driveway Permit and liquidated damages in the amount of \$2,000.00 per hour or any portion thereof will be assessed by the District Engineers Office.
- 15. Sight distance shall be free and clear of any debris, foliage and/or earth material for a minimum AASHTO sight distance shown in the plans. Vegetation removal and/or grading may be necessary to achieve the required sight distance. If the sight distance requirement is not achieved NCDOT reserves the right to deny/close/restrict this/these access(es)
- 16. Pavement pre-markings shall be approved by the District Office before thermoplastic is placed.
- 17. All (cast-in-place and/or pre-cast) splice boxes, handholes, manholes, drainage structures and other appurtenances within NCDOT Right of Way shall be of a NCDOT approved design for traffic bearing, HS-20 loading, and shall be flush mounted. Manholes, handholes and vaults shall not be placed in the ditch-line, side slopes of the ditch or in the pavement.
  - All frames, grates, rings, covers, etc. are to be manufactured in accordance with the requirements of Section 106-1B "Domestic Steel". Foreign castings are not approved for use within NCDOT Right of Way.
- **18.** The Permittee shall contact the District Office at (919) 814-6115 for inspection of forms or grade line prior to placing concrete for curb and gutter. Please provide 48 hours notification for inspections. All storm drainage grates and frames shall be on site at the time of the inspection.
- 19. All bare and disturbed areas must have a sufficient stand of vegetation. Address any erosion issues that may arise during time of construction. Monitor these areas as needed to assure this requirement is satisfied.
- 20. The roadway shall be kept free of silt, mud and debris at all times.

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#### **Driveway Permit Standard Provision**

- 1. An executed copy of this driveway access permit, and any associated encroachment permits, will be present at the construction site at all times during construction. NCDOT reserves the right to stop all work unless evidence of approval can be shown.
- 2. NCDOT reserves the right to further restrict this access at the expense of the encroaching party if/when accident history exists and/or operational issues occur.
- 3. NCDOT reserves the right to revise, restrict, suspend and/or void this driveway permit if the execution and/or operation of said permit is found to be a hazard to the traveling public.
- 4. This driveway permit only covers work within NCDOT Right-of-Way. The encroacher is responsible for verifying all right of way. NCDOT does not guarantee the right of way on this road. If the right of way was not obtained by the fee simple method, it is the responsibility of the encroacher to obtain permission from the underlying property owner/owners.
  - Encroacher shall be responsible for obtaining all necessary permanent and/or temporary construction, drainage, utility and/or sight distance easements. All Right of Way and easements necessary for construction and maintenance shall be dedicated to NCDOT with proof of dedication furnished to the District Engineer prior to beginning work.
- 5. Please note that approval of the driveway permit does not constitute review or approval of utilities or sidewalk by NCDOT. Plans and a completed encroachment agreement shall be submitted to the District Engineer's office for review and approval.
- **6.** The encroacher is responsible for any claim for damages brought by any property owner by reason of the installation.
- 7. Notify the District Engineer's Office at (919) 814-6115 or at 4009 District Drive, Raleigh, NC 27607, prior to beginning and after completion of work.
- **8.** The Encroacher shall notify the public, including all adjacent property owners and businesses, a minimum of 2 weeks prior to beginning work.
- **9.** Driveway radii and widths shall be as shown on the attached approved plans.
- 10. Any and all changes noted in red on the plans shall be incorporated into and made part of the approved permit.
- 11. The encroaching party shall comply with all applicable local, state and federal environmental regulations, and shall obtain all necessary state and federal environmental permits, including but not limited to, those related to sediment control, storm water, wetland, streams, endangered species, and historical sites.
- 12. All materials and construction shall be in accordance with NCDOT standards and specifications, including but not limited to, the latest version of NCDOT Standard Specifications for Roads and Structures, the NCDOT Roadway Standards Drawings, and NCDOT's Utilities Accommodation Manual.

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13. The encroacher shall provide traffic control devices, lane closures, road closures, positive protection and/or any other warning or positive protection devices necessary for the safety of road users during construction and any subsequent maintenance. This shall be performed in conformance with the latest NCDOT Roadway Standard Drawings and Standard Specifications for Roads and Structures and Amendments or Supplements and MUTCG thereto. When there is no guidance provided in the Roadway Standard Drawings or Specifications, comply with the Manual on Uniform Traffic Control Devices for Streets and Highways and Amendments or Supplements thereto. No work shall be performed in the Right of Way unless this requirement is satisfied. NCDOT reserves the right to require a written traffic control plan for encroachment operations.

Sidewalk closures shall be installed as necessary. Pedestrian traffic shall be detoured around these closures and shall be signed appropriately and in accordance with The American with Disabilities Act Accessibility Guidelines.

- 14. No parking or material storage shall be allowed along the shoulders of any NCDOT roadways.
- 15. Two-way traffic shall be maintained at all times.
- **16.** No lane closures shall be permitted between the hours of 6:00 AM to 9:00 AM and 4:00 PM to 7:00 PM, Monday through Friday unless otherwise specified in the Special Provisions of this encroachment agreement.
- 17. At the end of each working day, equipment shall be parked outside of the clear recovery zone in order to avoid any obstruction to the travelling public. This clear recovery zone is measure from the edge of the nearest travel lane.
- 18. Work shall not be performed on both sides of the road simultaneously within the same area.
- 19. Ingress and egress shall be maintained to all businesses and dwellings at all times.
- 20. The paving of this roadway shall be in accordance with the latest version of NCDOT's Standard Specifications, Sections 610, 1012 and 1020. The Contractor shall follow all procedures of the attached Quality Management System (QMS) for asphalt pavement Maintenance Version. The Contractor must adhere to all testing requirements and quality control requirements specified. The Contractor shall contact the NCDOT Division 5 QA Supervisor at (910) 562995-6198 prior to producing plant mix and make the Supervisor aware that the mix is being produced for a future NCDOT road. Only NCDOT approved mix designs will be acceptable. A quality control plan shall be submitted to the District Engineer's Office prior to asphalt production. Use form QMS-MV1 for the Quality Control Plan submittal. Failing mixes and/or densities are subject to penalties including monetary payments or removal and replacement.
- 21. Roadway certification reports sealed by a Professional Engineer shall be submitted to the North Carolina Department of Transportation at 4009 District Drive, Raleigh, North Carolina, indicating the following:
  - \* Pavement thickness by type
  - \* Pavement density, core and/or test locations
  - \* Base thickness
  - \* Base density
  - \* Subgrade density

Test frequency and method shall be in conformance with the NCDOT "Materials and Tests" Manual. Test must be performed by a Certified Technician including name and Certification number on report.

- **22.** Any existing driveways, pavement, sidewalk, curb and gutter or drainage structures that are damaged during construction shall be repaired to their original condition.
- 23. When surface area in excess of one acre will be disturbed, the Encroacher shall submit a Sediment and Erosion Control Plan which has been approved by the appropriate regulatory agency or authority prior to beginning any work on the Right of Way. Failure to provide this information shall be grounds for suspension of operations.

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- 24. All erosion control devices and measures shall be constructed, installed, maintained, and removed by the Encroacher in accordance with all applicable Federal, State, and Local laws, regulations, ordinances, and policies. All earth areas shall be regraded and seeded in accordance with NCDOT Standards Specifications for Roads and Structures 1995. Seeding rates per acre shall be the following:
  - \* Year Round Mixture: 50# Pensacola Bahia Grass; 50# KY 31 Tall Fescue; 5# Centipede; 500# 10-20-20 Fertilizer; 4000# Limestone;
  - \* 2:1 Slopes Standard Mix: Use Year Round Mixture (Delete Centipede); Add 25# Service Lespedeza;
  - \* If Using Crown Vetch on 2:1 Slope (September-May): Use Year Round Mixture; Add Crown Vetch 15 lbs./Ac.; (Delete Centipede and Bahia)
- 25. The applicant is responsible for identifying project impacts to waters of the United States (wetlands, intermittent streams, perennial streams and ponds) located within the NCDOT right-of-way. The discharge of dredged or fill material into waters of the United States requires authorization from the United States Army Corps of Engineers (USACE) and certification from the North Carolina Division of Water Quality (NCDWQ). The applicant is required to obtain pertinent permits or certification from these regulatory agencies if construction of the project impacts waters of the United States within the NCDOT right-of-way. Additional information can be obtained by contacting the USACE or NCDWQ.
- 26. The applicant is responsible for avoiding impacts to federally protected species during project construction. Bald eagle, Michaux's sumac, smooth coneflower, dwarf wedge mussel, harperella, red-cockaded woodpecker and tar spinymussel are federally protected species that have been identified within NCDOT right-of-way in Durham, Person, Granville, Wake, Franklin, Vance, and Warren counties. Additional information can be obtained by contacting the North Carolina Natural Heritage Program or the United States Fish and Wildlife Services.
- 27. The applicant is responsible for complying with the Neuse and Tar-Pamlico Riparian Buffer Rule as regulated by the NCDWQ. The Rule regulates activity within a 50-foot buffer along perennial streams, intermittent streams and ponds. Additional information can be obtained by contacting the NCDWQ.
- 28. Existing drainage patterns shall be maintained at all times throughout the proposed construction. The encroacher shall keep the roadway clean of dirt and debris at all times throughout the duration of the project.
- **29.** All proposed landscaping and plantings located within the NCDOT right of way shall be approved by the Division Roadside Environmental Engineer at (919) 317-4700.
  - In the event these plants require relocation or removal for highway construction, reconstruction, or maintenance of safety, such removal or relocation will be done immediately by the permittee upon notification by the NCDOT entirely at the expense of the permittee.
- **30.** The Division Traffic Engineer, shall be notified at (919) 536-4000 prior to any excavation within 500 feet of a signalized intersection or if there are existing NCDOT signs in or near the proposed work zone. All traffic signal or detection cables must be located prior to excavation. All signal work and traffic signs shall be coordinated with the Division Traffic Engineer. Costs to relocate, replace, or repair NCDOT signs, signals, or associated equipment shall be the responsibility of the Encroacher.
- **31.** All temporary and final pavement markings, reflective pavement markings, raised pavement markers, non-cast iron snowplowable pavement markers and signage are the responsibility of the Encroacher. All final pavement markings shall be thermoplastic. Any pavement markings/markers that are damaged or obliterated shall be restored at no expense to NCDOT.
- 32. All Traffic signs moved shall be reinstalled as soon as possible to meet NCDOT specifications.
- **33.** Strict compliance with the Policies and Procedures in the NCDOT's Utilities Accommodation Manual shall be required.

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- **34.** It shall be the responsibility of the Encroacher to determine the location of other utilities within the encroachment area. The Encroacher shall be responsible for notifying other utility owners and providing protection and safeguards to prevent damage or interruption to existing facilities and to maintain accessibility to existing utilities.
- **35.** All earth areas disturbed shall be regraded and reseeded in accordance with Division of Highways Standards and Specifications.
- **36.** The Encroacher shall remove all trees, stumps and vegetative material from the right of way and dispose of in a licensed landfill or disposal site.
- 37. Excavated material shall not be placed on the roadway at any time.
- **38.** Trenching, bore pits and/or other excavations shall not be left open or unsafe overnight. The Contractor shall comply with all OSHA requirements and provide a competent person on site to supervise excavation at all times.
- 39. All excavations inside the theoretical 1:1 slope from the existing edge of pavement to the bottom of the nearest excavation wall should be made in accordance with the following conditions. Traffic should be moved to a travel lane outside the limits of a theoretical one-to-one slope from the bottom of the nearest trench wall to the pavement surface. Active excavation shoring, such as sheet piling, shall be installed. The design of the shoring shall include the effects of traffic loads. The shoring system shall be designed and sealed by an engineer registered in North Carolina. Trench boxes shall not be accepted as shoring. The trench backfill material should meet the Statewide Borrow Criteria.
- **40.** Excavated areas adjacent to pavement having more than a 2 inch drop shall be made safe with a 6:1 or flatter slope and shall be designated by appropriate delineation during periods of construction inactivity, including, but not limited to, night and weekend hours.
- 41. Backfill material is to be placed at a maximum of 6 inch loose layers and each layer thoroughly compacted. All embankment backfill shall be compacted to 95% density and all subgrade to 100% density in accordance with AASHTO T-99 as modified by NCDOT. They shall be signed by a Professional Engineer and sent to the District Engineers Office at 4009 District Drive, Raleigh, NC 27607.
- **42.** No commercial advertising shall be allowed within NCDOT Right of Way.
- **43.** Guardrail shall be installed where warranted and in accordance with the guidelines shown in the 2018 Highway Design Branch Roadway Standard Drawings.
  - Guardrail removed or damaged during construction shall be replaced or repaired to their original condition.
- **44.** Poles shall be located/relocated at or as near as possible to the right-of-way line, shall be set outside the Clear Recovery Area as outlined by AASHTO and outside sight distance triangles.
  - Poles located within guardrail sections shall be installed a minimum of 5 feet behind any guardrail. When applicable, poles shall be placed behind sidewalk.
  - Any associated guy wires to ground anchors and stub poles shall not be placed between a pole and the travel way and should be located outside the clear recovery area.
  - Minimum vertical clearance shall be 18' for aerial crossings over NCDOT roadways and 15'-6" for installations parallel to the roadway.
- **45.** Fire Hydrants shall be of the break-away type. Hydrants shall be placed a maximum of one foot inside the right of way in ditch sections or a minimum of 6 feet behind the curb in curb and gutter sections.

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- **46.** Retaining walls or other vertical structures shall not be permitted inside NCDOT right of way.
- 47. Current and future state projects take precedence over this encroachment.

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APPLICATION IDENTIFICATION	N.C. DEPARTMENT OF TRANSPORTATION
Driveway Date of Permit No. Application	STREET AND DRIVEWAY ACCESS
	PERMIT APPLICATION
1011-0-11-11-11-11-11-11-11-11-11-11-11-	- TENINY ALTEROATION
Development Name: JONES DAIRY STORAGE FACILITY	
Route/Road: SR-2053 / JONES DARIY ROAD	PERIY
Event Distance	
Exact Distance ☐ Miles N S E W 815 ☑ Feet ☐ ☐ ☑ ☐	
	. SR-4403 Toward SR-4478
	#0004000000 2
	ducational Facilities  TND  Emergency Services  Other
Property:   ✓ is  ☐ is not with  AGREEMEN	Oily Zoning Filed.
I, the undersigned property owner, request access and permission	
of-way at the above location.	Tto construct driveway(s) or street(s) on public right-
• I agree to construct and maintain driveway(s) or street entrance(s	) in absolute conformance with the current "Policy on
Street and Driveway Access to North Carolina Highways" as adop	oted by the North Carolina Department of
Transportation.  I agree that no signs or objects will be placed on or over the public	a right of way other than these approved by NODOT
I agree that the driveway(s) or street(s) will be constructed as sho	wn on the attached plans
<ul> <li>I agree that that driveway(s) or street(s) as used in this agreemen</li> </ul>	t include any approach tapers, storage lanes or
speed change lanes as deemed necessary.	
I agree that if any future improvements to the roadway become no leasted as public right of providing the standard or public right.	ecessary, the portion of driveway(s) or street(s)
located on public right-of-way will be considered the property of the will not be entitled to reimbursement or have any claim for present	t expenditures for driveway or street construction, and I
I agree that this permit becomes void if construction of driveway(s)	construction.
specified by the "Policy on Street and Driveway Access to North C	Carolina Highways".
<ul> <li>I agree to pay a \$50 construction inspection fee. Make checks pa</li> </ul>	yable to NCDOT. This fee will be reimbursed if
application is denied.	•
<ul> <li>I agree to construct and maintain the driveway(s) or street(s) in a the public travel.</li> </ul>	sate manner so as not to interfere with or endanger
I agree to provide during and following construction proper signs,	signal lights, flaggers and other warning devices for
the protection of traffic in conformance with the current "Manual or	n Uniform Traffic Control Devices for Streets and
Highways" and Amendments or Supplements thereto. Information	n as to the above rules and regulations may be
obtained from the District Engineer.  I agree to indemnify and save harmless the North Carolina Depart	tmont of Transportation from all damages and alsing
for damage that may arise by reason of this construction.	then of transportation from all damages and claims
<ul> <li>I agree that the North Carolina Department of Transportation will a</li> </ul>	assume no responsibility for any damages that may
be caused to such facilities, within the highway right-of-way limits,	in carrying out its construction.
<ul> <li>I agree to provide a Performance and Indemnity Bond in the amount construction proposed on the State Highway system.</li> </ul>	unt specified by the Division of Highways for any
The granting of this permit is subject to the regulatory powers of the state Highway system.	De NC Department of Transportation as provided by
law and as set forth in the N.C. Policy on Driveways and shall not	be construed as a contract access point.
<ul> <li>I agree that the entire cost of constructing and maintaining an app</li> </ul>	roved private street or driveway access connection
and conditions of this permit will be borne by the property owner,	the applicant, and their grantees, successors, and
<ul> <li>assignees.</li> <li>I AGREE TO NOTIFY THE DISTRICT ENGINEER WHEN THE P</li> </ul>	BODOSED WORK BECING AND WINES IT IS
COMPLETED.	MOROSED WORK DEGINS AND WHEN II IS
2004-07 NOTE: Submit Four Copies of Application to Local District Eng	ineer, N.C. Department of Transportation TEB 65-04rev.
61-03419	,

SIGNATURES	OF APPLICA	NT
PROPERTY OWNER (APPLICANT) Rivercrest Realty Investors, LLC  8816 SIX FORKS ROAD, SUITE 201 RALEIGH, NC 27615 Phone No. (919) 846-40	NAME SIGNATURE ADDRESS	BEIL SIX FORMS Pd.
AUTHORIZED AGENT  COMPANY SIGNATURE ADDRESS Phone No.  APPRO	NAME SIGNATURE ADDRESS	WITNESS
APPLICATION RECEIVED BY DISTRICT ENGINEER  SIGNATURE		DATE
APPLICATION APPROVED BY LOCAL GOVERNMENTAL AUTHORITY (when Meredith A. Gruber Planningsignature	n required)  ig Director  TITLE	
APPLICATION APPROVED BY NCDOT  SIGNATURE	TITLE	DATE
INSPECTION BY NCDOT  SIGNATURE	TITLE	DATE
COMMENTS:		

## **SUMMARY OF PROBABLE CONSTRUCTION COSTS**

Road Improvements (Non-Utility Related; 2-Party Road Widening)

Project Name: Jones Dairy Storage

Rolesville, NC

25-Jan-24

OFF-SITE IMPROVEMENTS	Quantity	Unit		Unit Cost	Total Cost
General Conditions					
General Conditions, Bonding & Layout	10	%	\$	2,641	\$ 26,408
Erosion Control					
Erosion Control	1	LS	\$	7,500	\$ 7,500
Traffic Control					
Flaggers and Barricades	15	Days	\$	1,500	\$ 22,500
Demolition					
Mill Tapers on Each End	2	EA	\$	1,500.00	\$ 3,000
Grading					
Earthwork (Cut & Fill)	500	CY	\$	10.00	\$ 5,000
Fine Grading	20000	SF	\$	1.00	\$ 20,000
Final Shoulder Backfill	1200	LF	\$	1.00	\$ 1,200
Site Improvements					
Asphalt Pvmt 1.5" Overlay	1760	SY	\$	25.00	\$ 44,000
Asphalt Pvmt NCDOT Full Depth Widening	870	SY	\$	128.00	\$ 111,360
Asphalt Pvmt Asphalt Side path	290	SY	\$	45.00	\$ 13,050
Curb & Gutter - 30"	280	LF	\$	24.00	\$ 6,720
Conc Driveway Aprons	100	SY	\$	85.00	\$ 8,500
Conc. ADA Ramps	2	EA	\$	2,500.00	\$ 5,000
15-in RCP Storm Sewer	50	LF	\$	75.00	\$ 3,750
15-in Conc. FES	4	EA	\$	500.00	\$ 2,000
Thermo Pavement Markings	1	EA	\$	2,500.00	\$ 2,500
Mailbox Relocations	1	LS	\$	1,000.00	\$ 1,000
Landscaping	· .				
Seeding & Cleanup	20000	SF	\$	0.25	\$ 5,000
Allowances 21-21	2				
Unsuitable Soil Allowance SEAL	20	CY	\$	100	\$ 2,000
1 / 955780	<b>/</b>				
Unsuitable Soil Allowance SEAL 055780	A	Subtotal	:		\$ 290,488
SNO	2 Miller 10%	Co	ontrac	tors OH&P	\$ 29,049
O William	5%	M	isc. C	ontingency	\$ 14,524
	ROAD IMPRO	VEMENT	COS	ST TOTAL:	\$ 334,061

Timmons Group's Opinion of Probable Construction Cost (OPCC) is based on best judgment, experience and being qualified professionals generally familiar with the construction industry. Because Timmons Group has no control over the cost of labor, materials, equipment, services furnished by others, or over competitive bidding or market conditions, Timmons Group cannot guarantee that actual construction costs will not vary from the OPCC presented.

Driveway 1: SR 2053 Speed Lim	nit 45				
Road Name Tones Dairy Rd			10	ER.	
Existing Cross Section:				4403	
2 Lanes	SA	X			
(No / Striped / Concrete) Median	3.5 Feet	20	53		
Feet			K		
Total Paved Width	27 Feet		T- ·		Table
Proposed Cross Section:		SD =1.47*V*t	Design	Time	SD
Lanes	Feet	t=(f)+ lanes*.5 Left Turn (6.5)	Speed (V)	Gap (t)	-
(No / Striped / Concrete / Planted ) Median		Right Turn	1	6.5	
	Feet	Measured from	19' hohine		
Total Paved Width	Feet	ivieasured iron	i to nemino	EUI	
Ditch (Ves.) No) Depth: 1.5 Crosslines (Yes /					ndition:
Ditch (Ves.) No) Depth: 1.5 Crosslines (Yes / (Notes:  Driveway 2: SR Speed Lim	(G) Size:				ndition:
Ditch (Ves.) No) Depth: 1.5 Crosslines (Yes / 6 Notes:  Driveway 2: SR Speed Lim Road Name	(G) Size:				ndition:
Ditch (Fes. No.) Depth: 1.5 Crosslines (Yes / 6  Notes: Speed Lim Road Name Existing Cross Section:	(i) Size:				ndition:_
Driveway 2: SR Speed Lim Road Name Existing Cross Section:  Lanes	Size:				ndition:
Ditch (Ses No.) Depth: 1.5 Crosslines (Yes / 6  Notes: Speed Lim Road Name Existing Cross Section:	Size:				ndition:
Lanes (No / Striped / Concrete) Median	Size:				ndition:
Driveway 2: SR Speed Lim Road Name Existing Cross Section: Lanes (No / Striped / Concrete) Median  Total Paved Width	Size:				ndition:
Driveway 2: SR Speed Lim Road Name Lanes (No / Striped / Concrete) Median  Total Paved Width  Proposed Cross Section:	Feet Feet Feet Feet Feet	Location:	Pave	ment Co	
Driveway 2: SR Speed Lim Road Name Existing Cross Section: Lanes (No / Striped / Concrete) Median  Total Paved Width  Proposed Cross Section: Lanes Lanes	Feet Feet Feet Feet Feet	Location:SD =1.47*V*t	Pave	ment Co	ndition:
Driveway 2: SR Speed Lim Road Name Lanes (No / Striped / Concrete) Median  Total Paved Width  Proposed Cross Section:	Feet Feet Feet Feet Feet Feet Feet	SD =1.47*V*t t=(f)+ lanes*.5	Pave	ment Co	
Driveway 2: SR Speed Lim Road Name Existing Cross Section: Lanes (No / Striped / Concrete) Median  Total Paved Width  Proposed Cross Section: Lanes (No / Striped / Concrete) Median	Feet Feet Feet Feet Feet Feet Feet Feet	SD =1.47*V*t t=(f)+ lanes*.5 teft Turn (6.5)	Pave	Time Gap (t)	
Driveway 2: SR Speed Lim Road Name Existing Cross Section: Lanes (No / Striped / Concrete) Median  Total Paved Width  Proposed Cross Section: Lanes Lanes	Feet Feet Feet Feet Feet Feet Feet	SD =1.47*V*t t=(f)+ lanes*.5 Left Turn (6.5) Right Turn (5.5)	Design Speed (V)	Time Gap (t)	
Driveway 2: SR Speed Lim Road Name Existing Cross Section: Lanes (No / Striped / Concrete) Median  Total Paved Width  Proposed Cross Section: Lanes (No / Striped / Concrete) Median	Feet Feet Feet Feet Feet Feet Feet Feet	SD =1.47*V*t t=(f)+ lanes*.5 teft Turn (6.5)	Design Speed (V)	Time Gap (t)	
Driveway 2: SR Speed Lim Road Name Lanes (No / Striped / Concrete) Median  Total Paved Width  Proposed Cross Section: Lanes (No / Striped / Concrete) Median  Total Paved Width  Total Paved Width  Total Paved Width	Feet Feet Feet Feet Feet Feet Feet Feet	SD =1.47*V*t t=(f)+ lanes*.5 teft Turn (6.5) Right Turn (5.5) Measured from	Design Speed (V)	Time Gap (t)	
Driveway 2: SR Speed Lim Road Name Existing Cross Section: Lanes (No / Striped / Concrete) Median  Total Paved Width Proposed Cross Section: Lanes (No / Striped / Concrete) Median  Total Paved Width Proposed Cross Section: Lanes (No / Striped / Concrete) Median  Total Paved Width  Proposed Access: Right In Right	Feet Feet Feet Feet Feet Feet Feet Feet	SD =1.47*V*t t=(f)+ lanes*.5 Left Turn (6.5) Right Turn (5.5) Measured from	Design Speed (V)	Time Gap (t) 6.5 d EOT	
Driveway 2: SR Speed Lim Road Name Existing Cross Section:     Lanes     (No / Striped / Concrete) Median  Total Paved Width  Proposed Cross Section:     Lanes     (No / Striped / Concrete) Median  Total Paved Width  Proposed Cross Section:     Lanes     (No / Striped / Concrete) Median  Total Paved Width  Proposed Access: Right In Right  Sight Distance: Looking Leading Leadin	Feet Feet Feet Feet Feet Feet Feet Feet	SD =1.47*V*t t=(f)+ lanes*.5 Left Turn (6.5) Right Turn (5.5) Measured from the first tend to be a considered from the first tend tend to be a considered from the first tend tend tend tend tend tend tend ten	Design Speed (V) m 18' behin ut ht (Left Tur	Time Gap (t) 6.5 d EOT	
Driveway 2: SR Speed Lim Road Name Existing Cross Section: Lanes (No / Striped / Concrete) Median  Total Paved Width Proposed Cross Section: Lanes (No / Striped / Concrete) Median  Total Paved Width Proposed Cross Section: Lanes (No / Striped / Concrete) Median  Total Paved Width  Proposed Access: Right In Right	Feet Feet Feet Feet Feet Feet Feet Feet	SD =1.47*V*t t=(f)+ lanes*.5 Left Turn (6.5) Right Turn (5.5) Measured from the cooking Right Cookin	Design Speed (V) m 18' behin ut ht (Left Tur	Time Gap (t) 6.5 d EOT	SD

Driveway 3: SR Speed Lim	nit			THE		
Road Name					-	
Existing Cross Section:						
Lanes	Feet					
(No / Striped / Concrete) Median	Feet					
	Feet					
Total Paved Width	Feet					
Proposed Cross Section:						
Lanes	Feet		SD =1.47*V*t	Design	Time	SD
(No / Striped / Concrete / Planted ) Median	Feet		t=(f)+ lanes*.5	Speed (V)		- manual -
	Feet		Left Turn (6.5)			
Total Paved Width	Feet		Right Turn (5.5)		6.5	
			Measured from			-
District	D1-1-1-D-1					
Proposed Access: Right In	Right Out	Le	ft in Lef	t Out		
ight Distance: Looking Le	ft (Right Turn	)	Looking Right (L	eft Turn)		
Required Sight Distance: Looking Le	ft (Right Turn	)	Looking Right (I	eft Turn)		
					No. 20 Canada	
Ditch (Yes / No) Depth: Crosslines (Yes / N	vo) Size:	Loca	tion:	_ Paveme	ent Con	dition:
lotes:						
Driveway 4: SR Speed Lim Road Name	it					
Existing Cross Section:						
Lanes	Feet					
(No / Striped / Concrete) Median	Feet					
	Feet					
Total Paved Width	Feet					
Proposed Cross Section:						
Lanes	Feet		SD =1.47*V*t	Design	Time	SD
(No / Striped / Concrete) Median	Feet		t=(f)+ lanes*.5	Speed (V)		
	Feet		Left Turn (6.5)			
Total Paved Width	Feet		Right Turn (5.5)		6.5	
			Measured from	n 18' behi	nd EOT	
						-
Proposed Access: Right In	Right Out	Le	ft In Lef	t Out		
ight Distance: Looking Le	ft (Right Turn	)	Looking Right (L	eft Turn)		
Required Sight Distance: Looking Le						
Ditch (Yes / No) Depth: Crosslines (Yes / N	lo) Size:	_ Loca	tion:	_ Paveme	nt Con	dition:
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ame: Vidul Jayaranan Si	gnature:_	Last	-	Da	te: <u> </u>  2	101/20
ame: Si	gnature:		*	Dat	te:	

93850 **TOWNE BANK TIMMONS GROUP** 1001 Boulders Pkwy, Suite 300 North Chesterfield, Virginia 23225 (804) 200-6500 Fax (804) 560-1016 FED LD# 54-1301413 October 23, 2023 PAY TIMMONS GROUP Fifty and 00/100 Dollars TO THE \$50.00 ORDER North Carolina Department of Transportation (NCDOT) OF NCDOT Division 5, District 1 Driveway Permit Review 4009 District Drive Raleigh, NC 27607

"093850" "051408949" 0281001456"

## VERIFICATION OF COMPLIANCE WITH **ENVIRONMENTAL REGULATIONS**

(Check Appropriate Box)

<b>~</b>	Permits from the N.C. Department of Environmental Quality and the U.S. Army Corp of Engineers are not required for this project. However, all applicable federal and state regulations have been followed.					
	The required permits from the N.C. Department of Environmental Quality and the U.S. Army Corp of Engineers have been obtained for this project. Copies of the permits are attached.					
V	All applicable NPI be met for this pr	DES Stormwater Permit ro oject.	equirements have b	een or will		
V	₫ ® .c. com	ompliance with all applic ws and regulations.	able sedimentation	and		
Pro	ject Name:	Jones Dairy Storage Facility				
Tov	vnship:	Rolesville	County:	Wake		
Pro	ject Engineer:	Garrett Frank	Phone Number	919-866-4503		
Pro	ject Contact:	Garrett Frank				
App	olicant's Name:	Rivercrest Realty Associates,	LLC STREET	CSERIO SANTA		
Dat	e Submitted:	10/27/2023		-1-2342 SEAL		
For	m VCER-1			GINEE RAP TO WAR		

Form VCER-1 June 2016

#### MA-22-09 CONDITIONS OF APPROVAL:

- DEVELOPMENT OF THE POPERTY SHALL BE IN SUBSTANTIAL CONFORMANCE WITH THE ACCOURANTNE ENHIGHT O CONCERT PLAN. I CONTROL SHOWN FOR COMMITTED LESIENTS INCLUDING, BUT ON THE POPERTY SHALL BE IN CONTROL SHOWN FOR CONTROL MASSURES SHOWN ON EXHIBIT OF LAKE CONCEPTUAL AND PROVIDED FOR ILLUSTRATION AND CONTEXT ONLY. IN ALL CONCEPTUAL SHALL BE CHEENING AS SUBSECURED IS SHALES OF A POPERTY IN ALL CONTROL SHELL SHALL BE CHEENING AS SUBSECURED IS SHALES OF A POPERTY IN ALL CONTROL SHALL SHALL BE CHEENING AS SUBSECURED IN SHALES SHALL SHALL BE CHEENING AS SUBSECURED IN SHALL SHALL BE CHEENING AS THE SHALL SH
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  ON EXTERIOR FAÇADES FACING A PUBLIC RIGHT-OF-JWAY OR EXTERIOR FAÇADES FACING PINS.
  1444706822. 1850604353, 1850606343, 1850606333, 1850607323, AND 1850608313. THE FOLLOWING MATERIALS ARE PROHIBITED: CINDERBLOCK, CONCRETE, METAL SIDING, AND
- VINYL SIDING, AND BRICK, EIFS BRICK, STONE, OR CONCRETE MASONRY COMPONENTS SHALL BE A MINIMUM OF

## SITE PLAN REVIEW DRAWINGS: **FOR**

## JONES DAIRY STORAGE FACILITY

1200 JONES DAIRY RD ROLESVILLE, NC 27587

SITE PLAN#: SDP-23-03 REZONING CASE #: MA-22-09 TEXT AMENDMENT CASE #: TA-22-02

#### APPROVED TA-22-02

Sheet List Table

COVER OVERALL SITE PLAN & KEY MA

EXISTING CONDITIONS & DEMOLITION PLAN

SITE PLAN

GRADING & DRAINAGE PLAN

SITE LITELITY PLAN

OFF-SITE UTILITY PLAN OFF-SITE UTILITY PLAN & PROFILE

FROSION & SEDIMENATION CONTROL PLAN- PH I

EROSION & SEDIMENATION CONTROL PLAN - PH II

SITE DETAILS

C0.0

C2.0

C3.0

C4 0

C4.1

C4.2

C5.0

C5.1

C6.0

C6.1

C6.2

C6.3 C6.4

C6.5

C6.6

C6.7

5.1.6.0.5.b. - 'INDUSTRIAL, LIGHT' PRINCÍPAL USE - SELF-SERVICE STORAGE SHALL BE CONTAÍNED WITHIN A FULLY ENCLOSED BUILDING AND CONTAÍNED IN A SINGLE BUILDING, ACCESS INTERNALL' EXCEPT IN THE GENERAL INDUSTRICT NOT LOCATED ON NAM 1ST OR 8 MAIN ST. 5.1.61.6.1.4. "VAREHOUSING" PRINCÍPAL USE - EXAMPLES. INCLUDES WAREHOUSES AND MINWARCHOUSES.

#### APPROVED ALTERNATE PARKING PLAN CONDITIONS

1 PARKING SPACE PER 100 SELF-STORAGE UNITS. PARKING PERMITTED BETWEEN A PROPOSED BUILDING AND THE STREET FRONTAGE.

#### ATTENTION CONTRACTORS

Failure to call for Impection, Install a Domistream Plug, has Permitted Plans on the Jobale, or any other Violation of City of Baleigh Standards will result to a Fine and Possible Evolution to lature work in the City of Raleigh.

PLAN - DEMO, SITE AND SIGNAGE & STRIPING PLANS

ROAD WIDENING PLAN - GRADING, DRAINAGE & FROSION CONTROL PLAN

JONES DAIRY ROAD - CROSS SECTIONS STA. 10+00-

JONES DAIRY ROAD - CROSS SECTIONS STA, 11+00

JONES DAIRY ROAD - CROSS SECTIONS STA. 12+60-

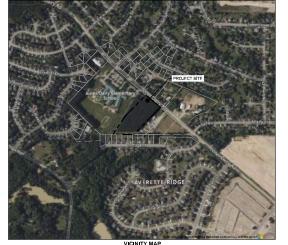
JONES DAIRY ROAD - CROSS SECTIONS STA, 13+00 -JONES DAIRY ROAD - CROSS SECTIONS STA. 14+00

IONES DAIRY ROAD - CROSS SECTIONS STA. 16+00 IONES DAIRY ROAD DRIVEWAY STEM PROFILE

Sheet List Table

#### SITE DATA TABLE

PROJECT NAME:	JONES DAIRY STORAGE FACILITY
SITE PLAN NUMBER	SDP-23-03
PROPERTY OWNER/ DEVELOPER:	ROLESVILLE (LOCKBOX) LLC 8816 SIX FORKS ROAD, SUITE 201 RALEIGH, NC 27615
REAL ESTATE ID:	0162697
PIN:	1850608722
DEED ACRES:	5.50 ACRES
PROPERTY ADDRESS:	1200 JONES DAIRY RD, ROLESVILLE NC 27587
PROPERTY ZONING:	GI-CZ
CURRENT USE:	VACANT
PROPOSED USE:	SELF STORAGE (COMMERCIAL)
DISTURBED AREA:	5.77 ACRES
CURRENT IMPERVIOUS:	0 ACRES
PROPOSED IMPERVIOUS:	2.92 ACRES
TREE SAVE SUMMARY:	2.02 NONLO
REQUIRED TREE PRESERVATION:	10% SAVE = 0.10*4.76 (TREE AREA ON-SITE) = 0.476
PROVIDED TREE PRESERVATION:	ACRES
	0.607 ACRES = 13%
PARKING SUMMARY:	A PRACE PER ASS STORAGE UNITE - SASARS -
REQUIRED VEHICULAR SPACES:	1 SPACE PER 100 STORAGE UNITS = 540/100 = 6 SPACES
PROVIDED VEHICULAR SPACES:	7 SPACES
ADA PARKING SUMMARY:	
REQUIRED VEHICULAR SPACES:	1 SPACE
PROVIDED VEHICULAR SPACES:	1 SPACE
BUILDING HEIGHT:	MAXIMUM BUILDING HEIGHT: WITHOUT SPRINKLERS: 35-FT WITH SPRINKLERS: 60-FT PROVIDED BUILDING HEIGHTS: 16 FT
BUILDING SQUARE FOOTAGE(S):	CLIMATE CONTROLLED BUILDING: 62,610 SQFT STORAGE BUILDING(S): 1800 SQFT (2), 3800 SQFT (2), 6600 SQFT
NUMBER OF STORAGE UNITS:	TOTAL NUMBER OF UNITS 540 BALLONG A SAS & UNITS
BUILDING/STRUCTURE SETBACKS:	
FRONT:	30-FT
SIDE:	15-FT
REAR:	36-FT
LANDSCAPE BUFFERS:	
ADJACENT PROPERTY:	50' TYPE "4" BUFFER (SOUTH & WEST OF PARCEL)
ADJACENT PROPERTY:	SEE CONDITION 4 (EAST OF PARCEL)
WATERSHED:	NEUSE RIVER
RIVER BASIN:	SANFORD CREEK (NEUSE)
SURFACE WATER CLASSIFICATION:	NONE



#### PROJECT TEAM

#### LAND OWNER

CONTACT: ROLESVILLE (LOCKBOX) LLC 8816 SIX FORKS ROAD, SUITE 201 RALEIGH NC 27615

#### DEVELOPER

RIVERCREST REALTY INVESTORS CONTACT: BRIAN HOLDER PHONE: (919) 846-4046 8816 SIX FORKS ROAD, SUITE 201 RALEIGH, NC 27615

#### CIVIL ENGINEER

TIMMONS GROUP CONTACT: GARRETT FRANK, PE, PLA PHONE: (919) 866-4503 5410 TRINITY ROAD, SUITE 102 RALEIGH, NC 27607

## Public Sewer Collection / Extension System

#### CITY OF RALEIGH - PLANS AUTHORIZED FOR CONSTRUCTION

ELECTRONIC APPROVAL THIS APPROVAL DE SIENDISSUED ELECTRONIC CAPPROVAL THIS APPROVAL DE SIENDISSUED ELECTRONICALLY THA APPROVAL IS VALID ONLY UPON THE SIGNATURE OF A CITY OF MELGIER REVIEW OF THE SIENDISSUE OF THIS APPROVAL VISIT APPROVAD DAMA, ANY VORK AUTHORIZED BY THIS APPROVAL VISIT ELECTRONIC APPROVAL ANY MOY BE CITY ON CESSION. ANY MODIFICATION TO THIS APPROVAL ONCE SISUED WILL INVALIDATE THIS APPROVAL.

CITY OF RALEIGH DEVELOPMENT APPROVAL RALEIGH WATER REVIEW OFFICER



08/01/

04/05/2023 L. BARNES

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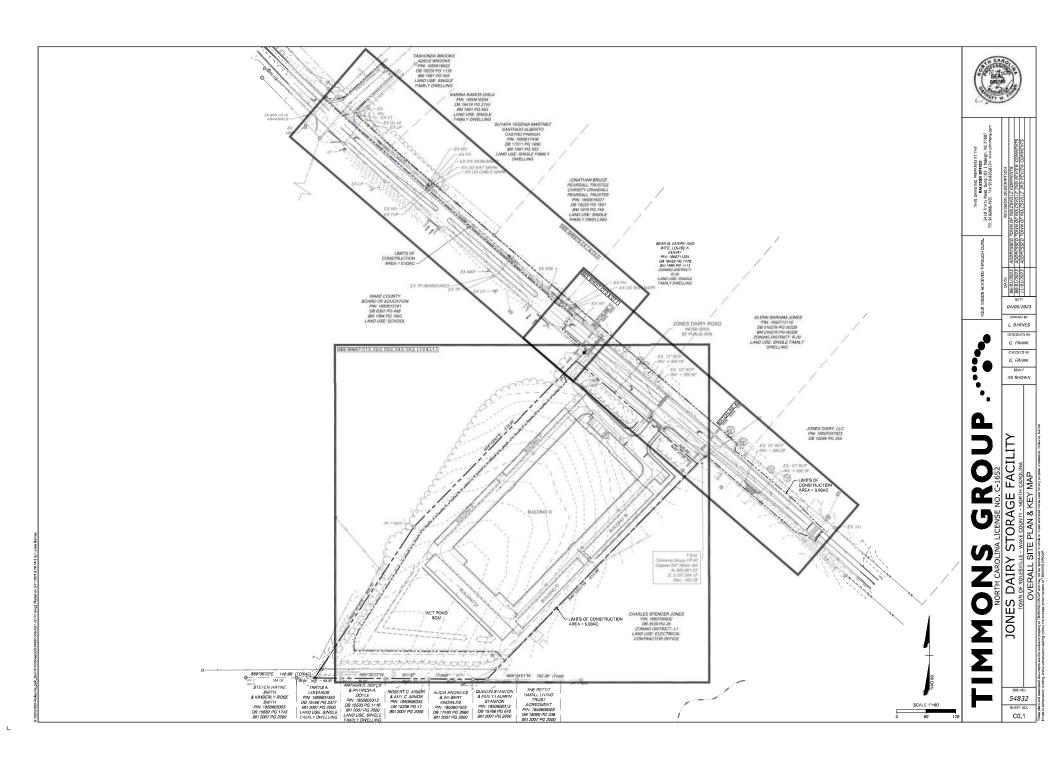
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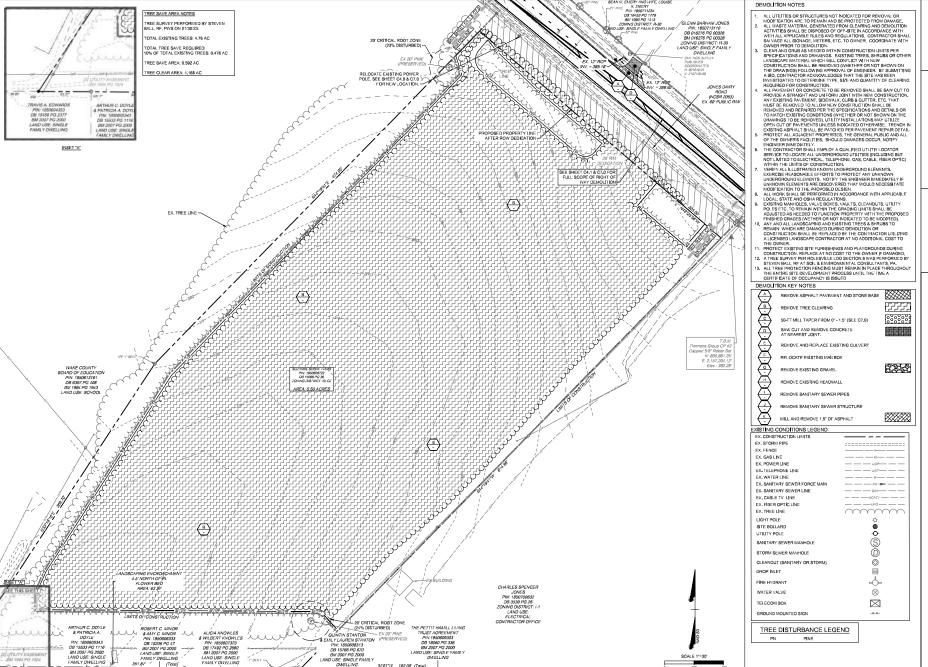
FACILITY STORAGE I

DAIRY JONES L

EROSION CONTROL, STORMWATE AND FLOODPLAIN MANAGEMENT APPROVED
EROSION CONTROL 
5-STORMWATER MGMT. S-FLOOD STUDY S-

54832 C0.0





34'01"W 192.08' (Total)



ADDRESSED TOWN OF ROLESVILLE COMMENTS
ADDRESSED TOWN OF ROLESVILLE ZNO REVIEW COMMENTS
ADDRESSED TOWN OF ROLESVILLE ZNO REVIEW COMMENTS DATE 06/01/2023 08/01/2023 11/01/2023

> 04/05/2023 L. BARNES

DESIGNED OF G. FRANK CHECKED BY G. FRANK

SCALE AS SHOWN

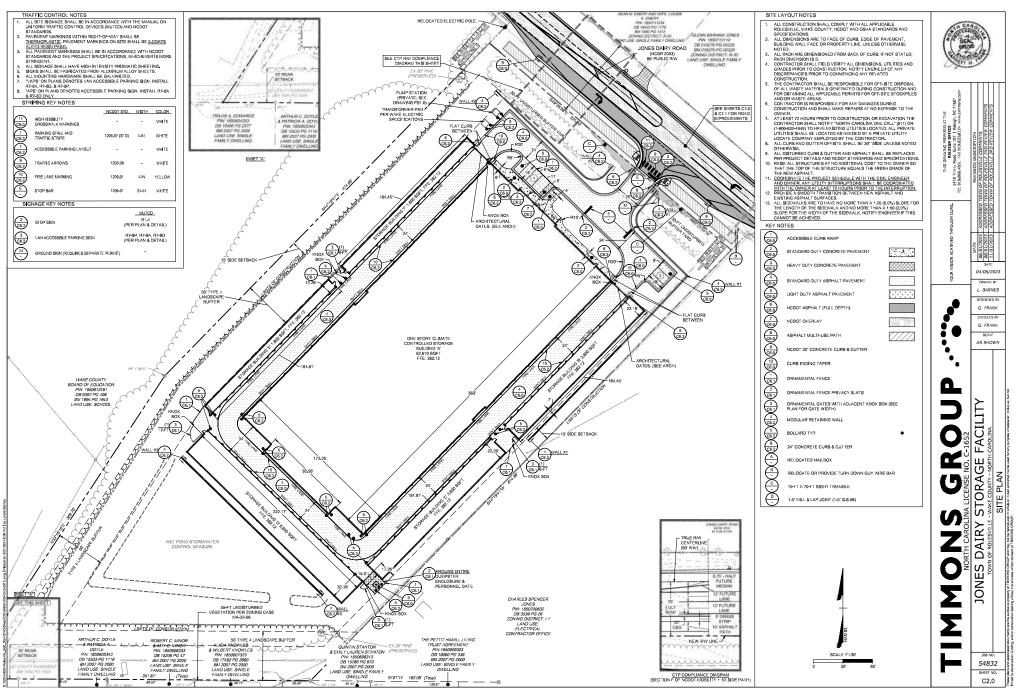
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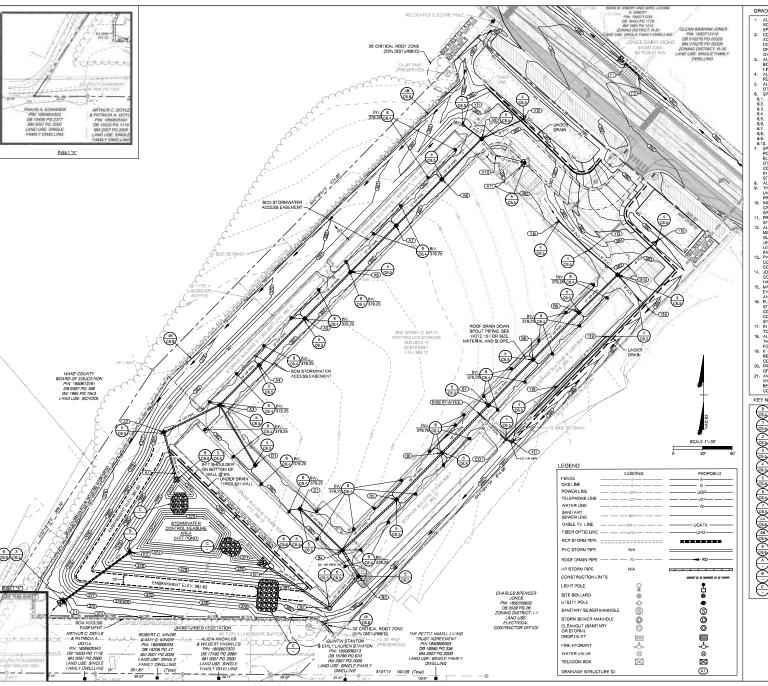
E FACILITY N PLAN

AJRY STORAGE FACT ROLESTILE - WANTE COUNTY - MOETH CAROLINA 3 CONDITIONS & DEMOLITION PI DAIRY EXISTING (

JONES [

54832 SHEET NO. C1.0





#### GRADING & DRAINAGE NOTES

- ALL CONSTRUCTION SHALL COMPLY WITH ALL APPLICABLE ROLESVILLVE, WAKE COUNTY, NCDOT AND OSHA STANDARDS AND SPECIFICATIONS.
- COORDINATE THE PROJECT SCHEDULE WITH THE OWNER, AND COORDINATE THE PROJECT SCHEDULE WITH THE OWNER, AND ADJACEN USERS OF THE PROPERTY. MAINTAIN TRAFFIC FLOW AND DO NOT INTERRUPT UTILITIES AROUND THE SITE. DO NO DISTURB OPERATIONS OF ADJACENT SITES AND FACILITIES ANDIOT THEIR OWNERS THE ADJACHULES STORD FACILITIES AND THEIR OWNERS THE ADJACHULES STORD THE STRUCTURES, VALVE
- ALLESIA NA VALLES AND THORSE AND STED AS NEEDED TO MATCH
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  SPOT GRADE ASSETUTATIONS
  5.1 TO TOP OF CURB
  6.2 THE CONVERSE OF AND THE ADJUST OF THE MATCH
  S. THE TOP OF WALL
  6.3 THE TOP OF WALL
  6.4 SANK SIDP-WALK
  6.5 THE TOP OF TOP OF THE TOP OF TOP OF THE TOP OF TH ES, CLEANOUTS, ETC. SHALL BE ADJUSTED AS NEEDED TO MATCH

- 3.8. T.L. FLOW LINE
  3. GEO SCHOOL
  3.0. GEO
  3. GEO
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- ALL RUDE UNLESS INDICATED OTHERWISE. ALL ROOF DRAINS FROM BUILDING TS. (C. AND TO, SHALL BE 8" PVC (SCH 40) BT. NEWS MIN. SLOPE UNLESS INDICATED OTHERWISE. USE DOUTLE IRON WHEN SLOPE UNLESS INDICATED OTHERWISE. USE DUTLE IRON WHEN COVER IS LESS THAN 24-IN. MATCH PIPE CROWNS WITH CONNECTION TO DROP
- INLET.

  13. PVC ROOF DRAIN PIPING UNDER PAVEMENT SHALL HAVE 24-IN MINIMUM
- COVER, IF ROOF DRAIN PIPING UNDER PAVEMENT HAS LESS THAN 24-IN COVER, ROOF DRAIN PIPING SHALL BE 8" OIP (IN LIEU OF PVC). JOINT FILL AND CAULK EACH CONCRETE EXPANSION JOINT AND WHERE CONCRETE PAVEMENT ABUTS OTHER PAVEMENTS, SIDEWALKS, OR HARD SUBFACEST.
- HARD SUIFACES.

  I. MAINTAIN ALL PROSION CONTROL DEVICES AFTER EACH RAINFALL EVENT IN ACCORDANCE WITH NODEO LAND QUALITY REQUIREMENTS AND AS BIRECTED BY THE NODEO, AND CHILD WITH REQUIREMENTS AND AS BIRECTED BY THE NODEO, AND CHILD WITH ALL SEQUENCY COUNTY OF STANDLINES FOR ADMINISTRA

- CONSTRUCTION TO MAINTAIN PROPER FUNCTIONING OF THE DRAINAGE
  SYSTEM.

  17. IN DISTURBED PARS, AMEND THE TOP 8 INCHES OF LAWN AREAS WITH
  18. IN STRUMBED PARS, AMEND THE TOP 8 INCHES OF LAWN AREAS WITH
  18. ALL SIDEWANS ARE TO HAVE NO MORE THAN A 120 GMB, 30.0PE FOR
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#### KEY NOTES

DOWNSPOUT BOOT CONNECTOR STORM SEWER CLEANOUT C8.5 STORM DRAINAGE TRENCHES

FLARED END SECTION

YARD INLET

CONCRETE DROP INLET

STORM SEWER MANHOLE

STORM SEWER MANHOLE COVER SILT FENCE (COMBINATION FENCE; SEE TREE FENCE DETAIL; SEE EROSION CONTROL PLAN)

TREE PROTECTION FENCE (SEE EROSION CONTROL PLAN) RIP RAP OUTLET PROTECTION (SEE EROSION CONTROL PLAN)

PROVIDE SMOOTH PAVING TRANSITION

CONCRETE RISER STRUCTURE (SEE SHEET C3.3)

CLASS B RIP RAP, SEE SCM DETAIL SHEETS.

DATE 06/01/2023 08/01/2023 11/01/2023

04/05/2023 L. BARNES

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SCALE AS SHOWN

FACILITY PLAN

STORAGE F

- WAKE COUNTY - NORTH C

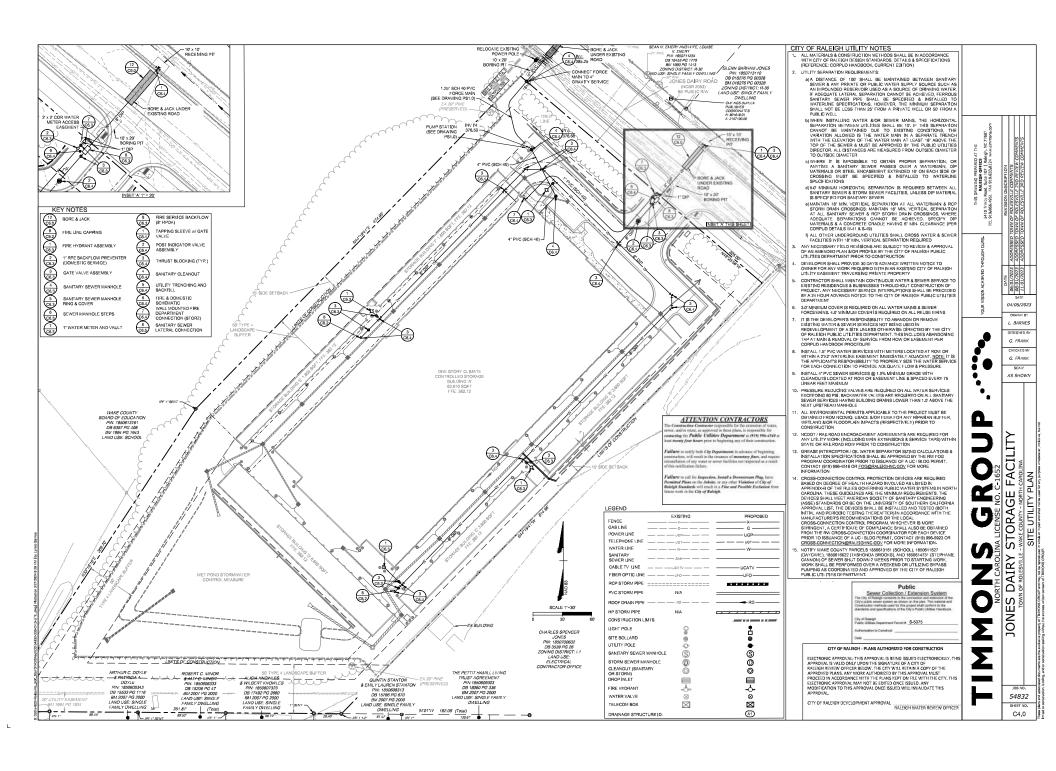
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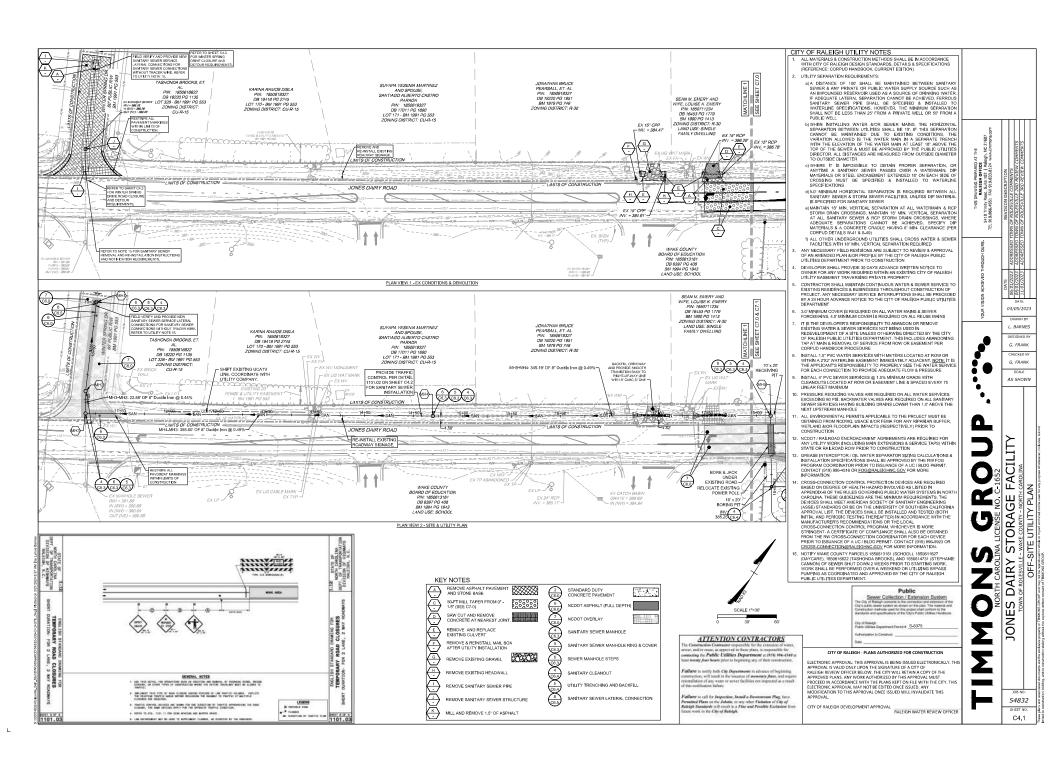
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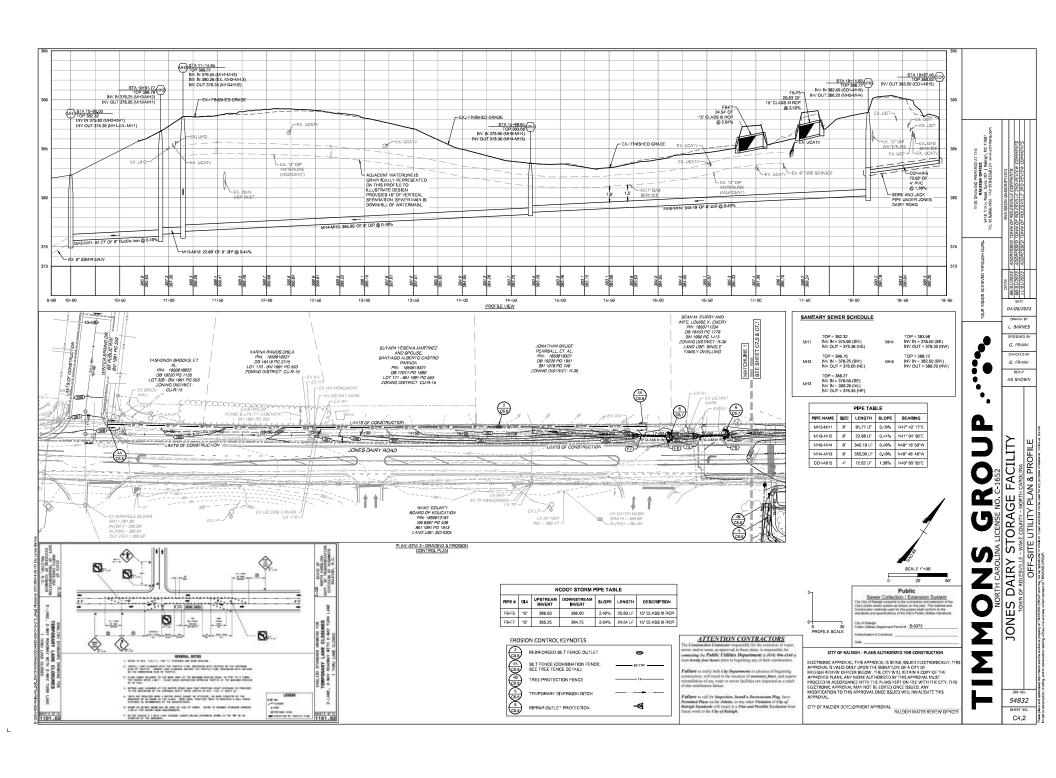
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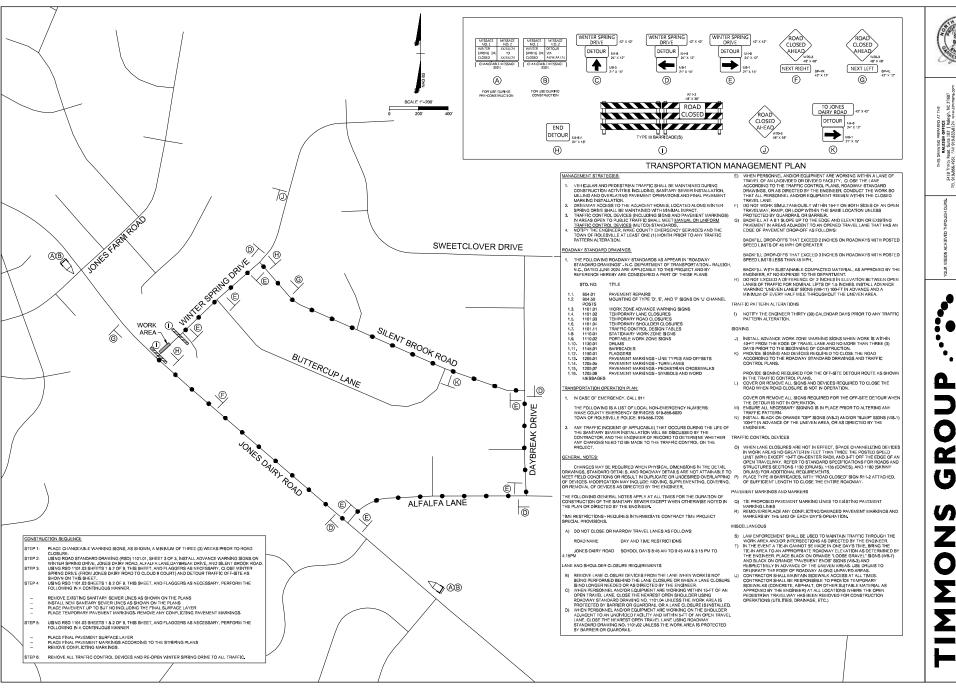
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5410 Trinity Ros 919-866-4951

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04/05/2023 L. BARNES

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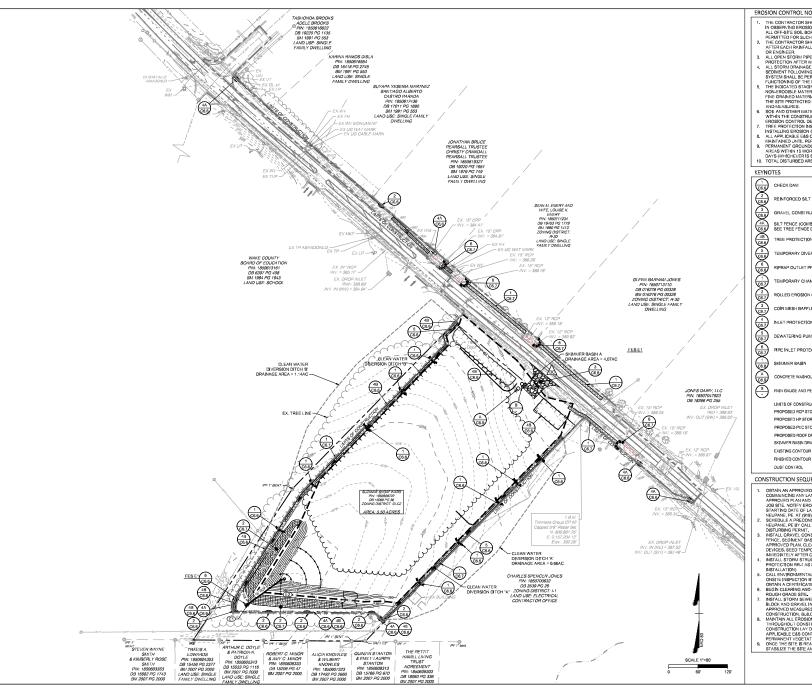
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STORAGE F.

- WAKE COUNTY - NORTH CA
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#### FROSION CONTROL NOTES

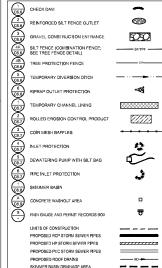
- WITHIN THE CONSTRUCTION LIMITS PROTECTED BY SEQUENT AND RESPONDED CONTINUE DEVICES AND MEASURES.

  RESPONDED CONTINUE DEVICES.

  NETALLING EROSION CONTROL DEVICES.

  ALL APPLICABLE SES CONTROL DEVICES.

  APPLICA



#### CONSTRUCTION SEQUENCE - PH I

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- DISTURBING PERVIT.

  IN STALL GRANE CONSTRUCTION PAD, TEMPORARY DISENSIONS, SILT FENCE, EDIDMENT MASINS OR OF HER MEASURES AS SHOWN ON THE FENCE, EDIDMENT MASINS OR OF HER MEASURES AS SHOWN ON THE DEVICES, SEED THAT WAS AN OF HER MEASURES. AS SHOWN ON THE SECRET AS SHOWN ON THE PROPERTY OF SHOWN ON THE SECRET AS SHOWN ON

- BEGIN CLEARING AND GRUBBING, MAINTAIN DEVICES AS NEEDED.
- BEGIN CLEARING AND GRUSSING, MINITAIN DEVICES AS NEEDED, NOUGH GRAZE SAMER, IES HOWN, AND PROTECT INLETS WITH BLOCK AND GRAVEL NIET CONTROLS, SEDIMENT TRAPS OR OTHER APPROVED MEASURES AS SHOWN ON THE PLAN. BEGIN CONSTRUCTION, BULLDING, ETC.
  MANTAIN ALL EROSION & SEDIMENT CONTROL MEASURES
- MAN IAN ALL EROSION & SEMMENT CONTROL INC. IN ASSURES TO THE ASSURES AND AN ARTHUR AND AN ARTHUR AND AN ARTHUR AND ARTHUR



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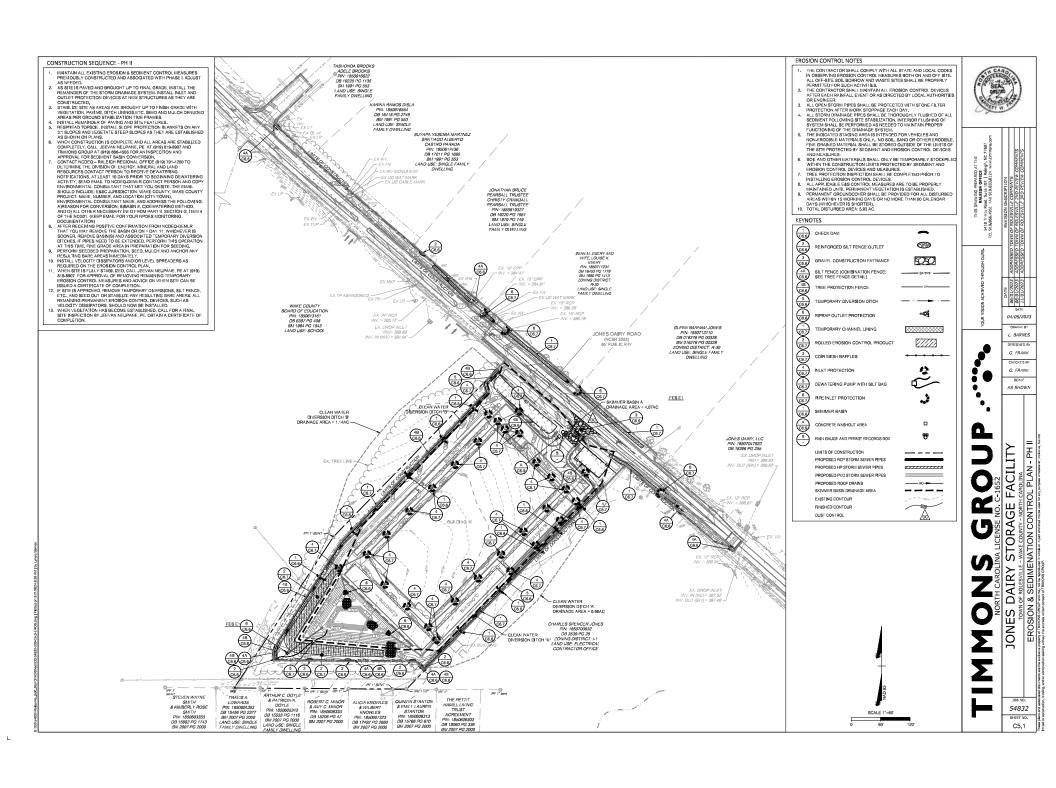
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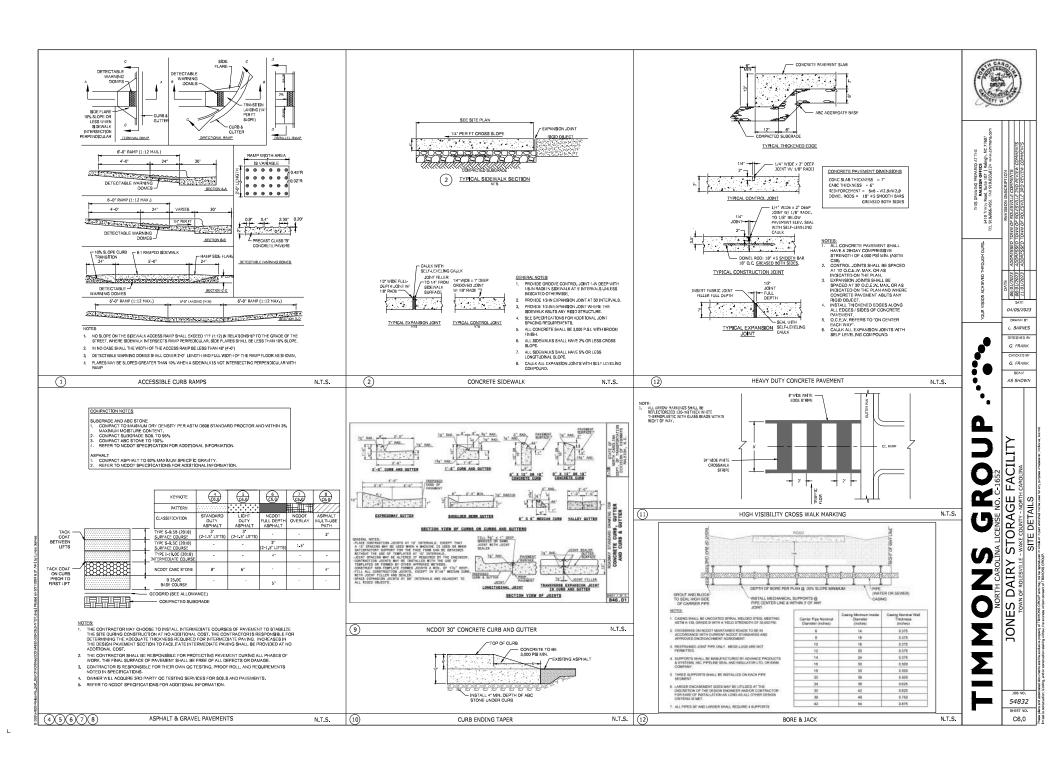
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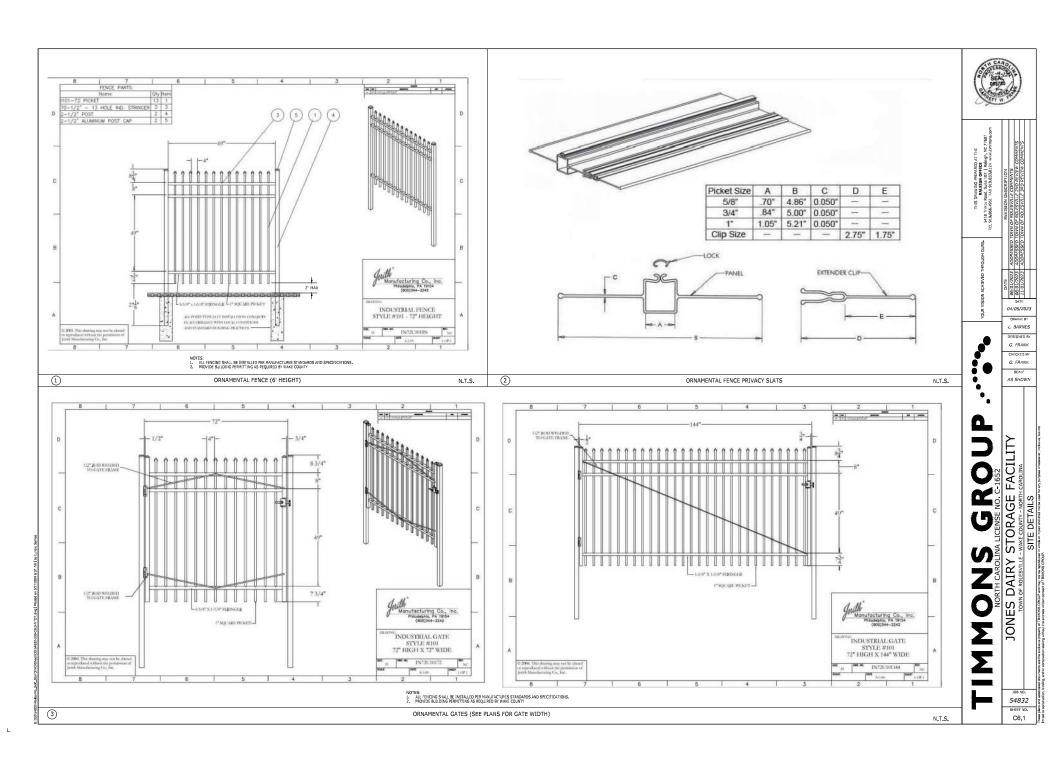
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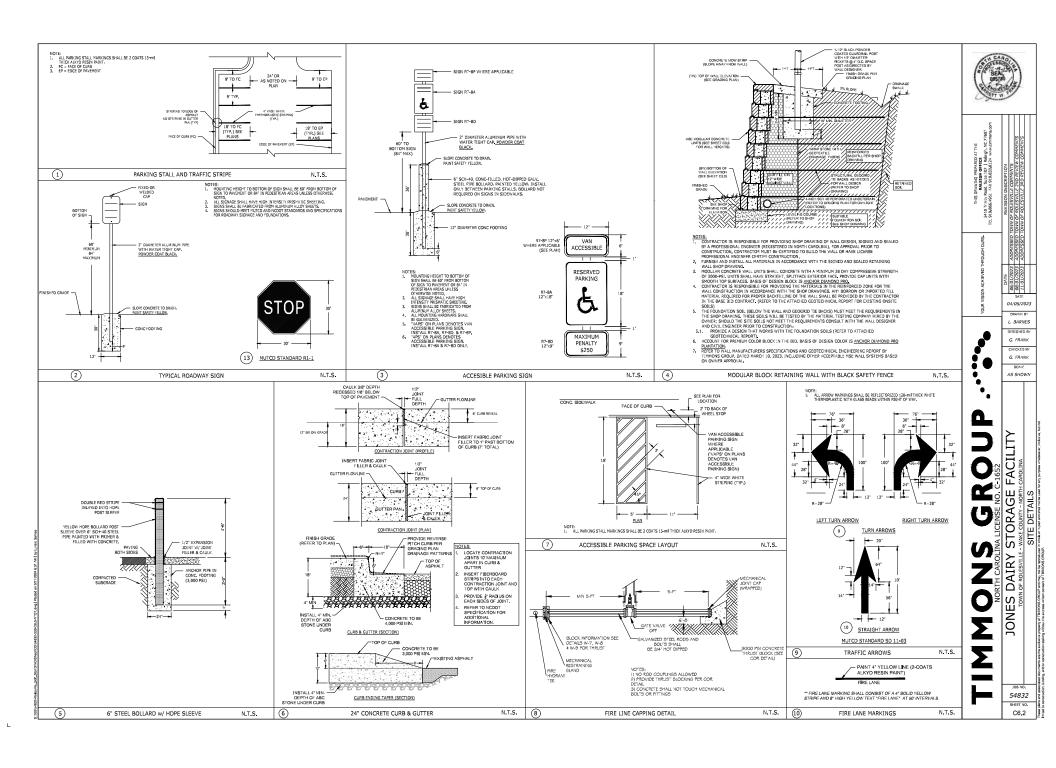
JONES DAIRY STORAGE FACILITY
TOWN OF ROLESTILE - WAKE COURTY- MORTH CAROLINA
EROSION & SEDIMENATION CONTROL PLAN - PH 1
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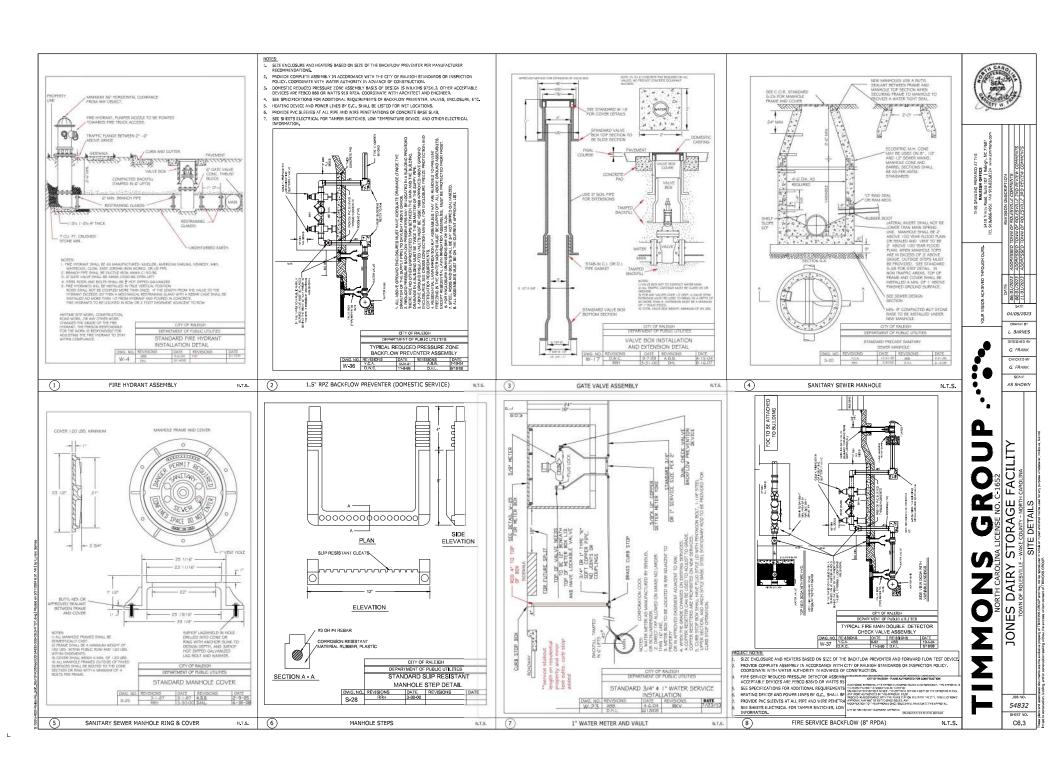
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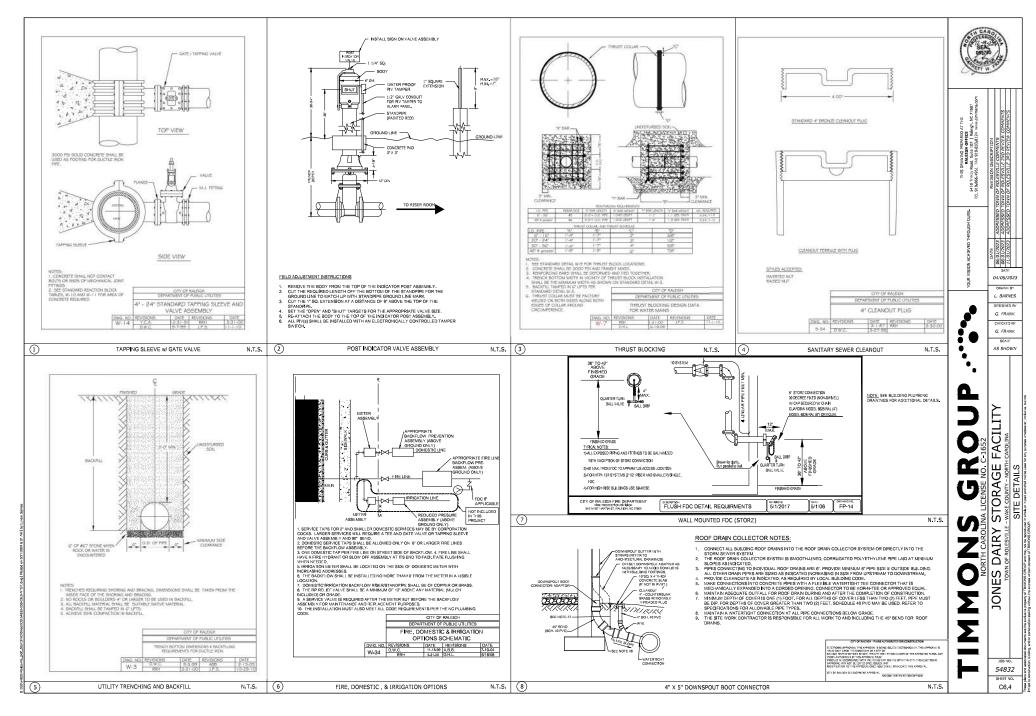




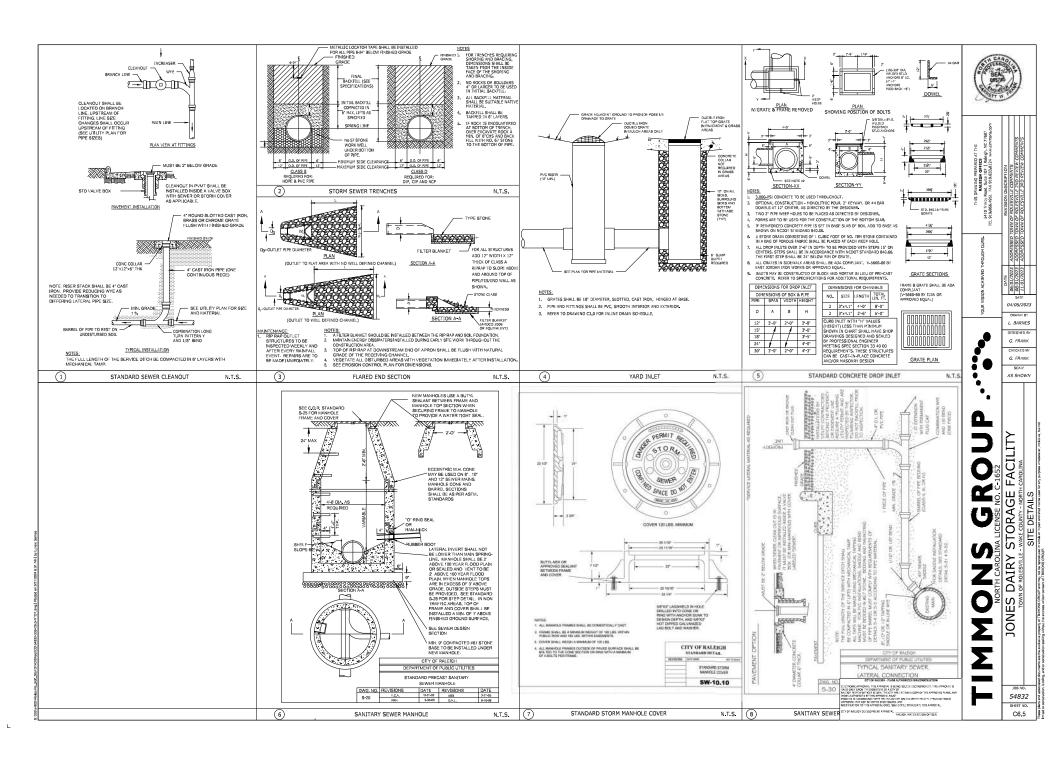


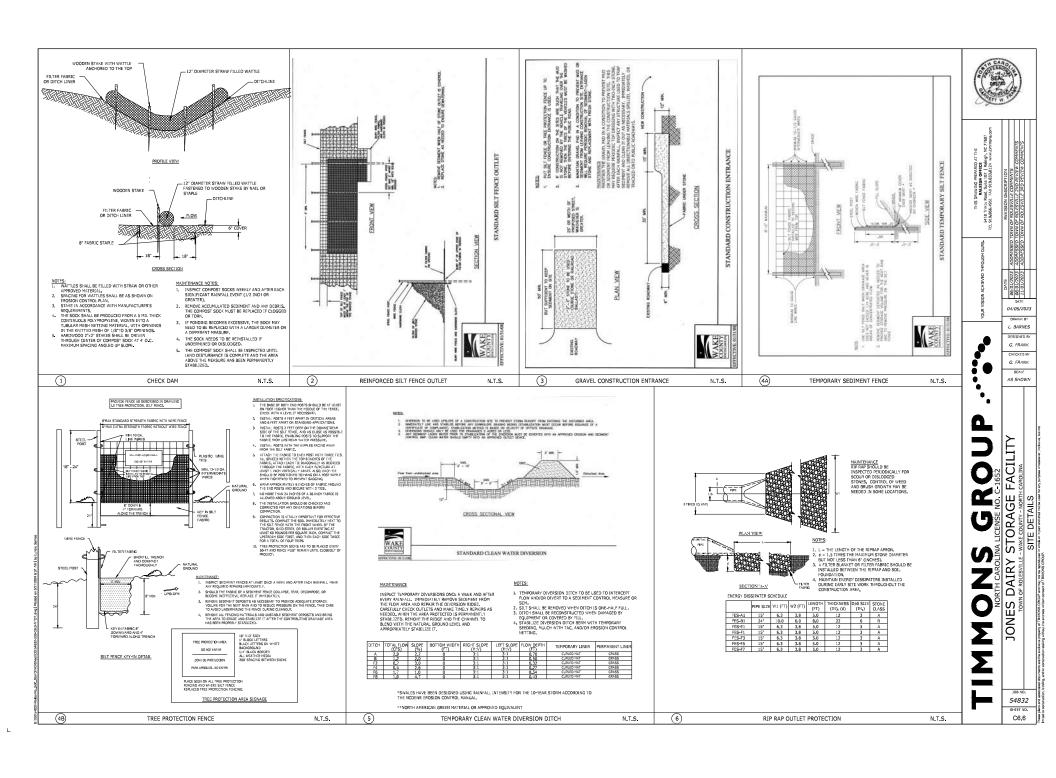


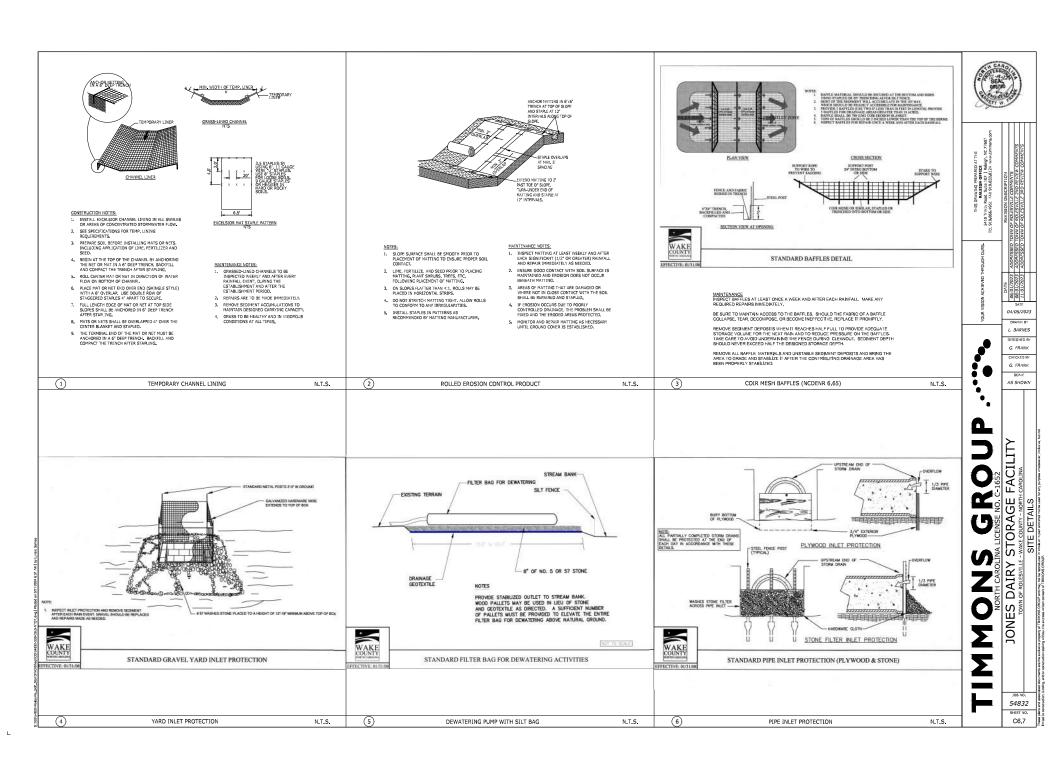


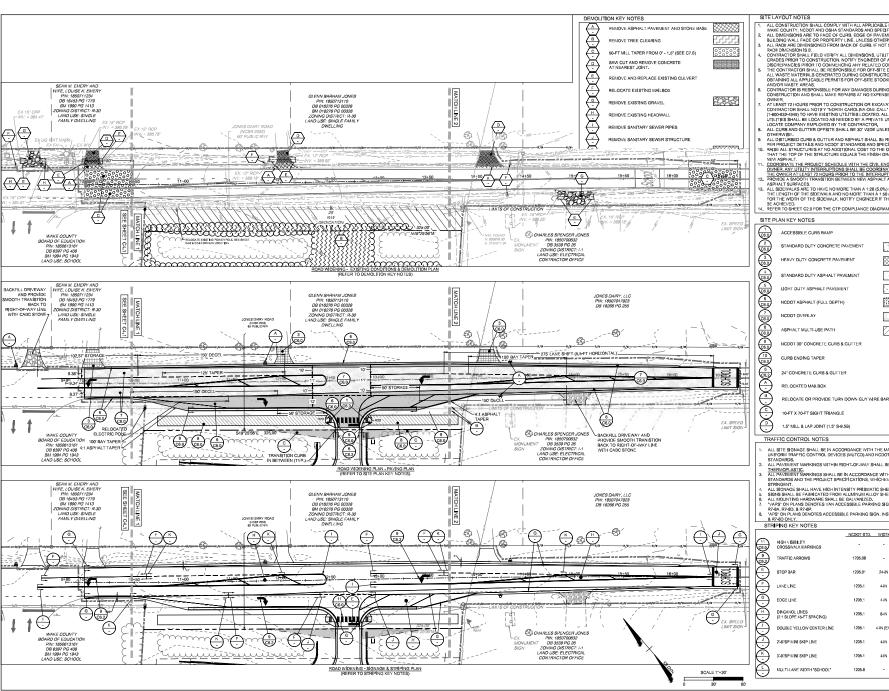


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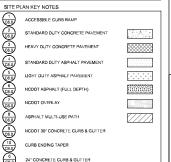


#### SITE LAYOUT NOTES

- ALL CONSTRUCTION SHALL COMPLY WITH ALL APPLICABLE ROLESVILLE, WAKE COUNTY, NCDOT AND OSHA STANDARDS AND SPECIFICATIONS.

- WARE COUNTY, NODOTAIN DISHA STANDARDS AND SPECIFICATIONS.
  ALL DIMENSIONS ARE TO PACE OF CIPIE, BODGE OF PRACTICATIONS.
  BULDING WALL FACE OF PROPERTY LINE; DILLESS OTHERWISE NOTED.
  BULDING WALL FACE OF PROPERTY LINE; DILLESS OTHERWISE NOTED.
  BULDING WALL FACE OF PROPERTY ALL DIMENSIONS, UTILITIES AND
  CONTRACTOR SHALL FEEL VERIFY ALL DIMENSIONS, UTILITIES AND
  DISCORPINACION CONSTRUCTION, DOTTE PROBLECT OF STATED,
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  DISCORPINACION CONTRACTOR SHALL SELECTION OF STATED
  ALL WASTE MARKING SCENARIO BURNING CONSTRUCTION AND FOR
  OSTANDAND ALL PRACTICATION OF STATED OFFENDRES OFFENDRES
  AND CONTRACTOR SHALL SELECTION OF STATED OFFENDRES OFFENDRES
  AND CONTRACTOR SHALL SHAL
- CONTRACTOR IS RESPONSIBLE FOR ANY DAMAGES DURING CONSTRUCTION AND SHALL MAKE REPAIRS AT NO EXPENSE TO THE
- OWNER.
  AT LEAST 72 HOURS PRIOR TO CONSTRUCTION OF EXCAVATION THE
  COVIRACTOR SHALL NOTIFY "NORTH CARROLING ONE CALL" (811) OR
  (\*480-633-409) TO HAVE EXISTING UTILITIES LOCATE D. ALL PRIVATE
  LITLITES SHALL BE LOCATED AS NEEDED BY A PRIVATE LITLITY
  LOCATE COMPANY CHEN CYCLED BY HE CONTRACTOR.
  ALL CLIRE AND GUTTER OFFSITE SHALL BE 30" WIDE UNLESS NOTED
  OTHERWISE.
- OTHERWISE. ALL DISTURBED CURB & GUTTER AND ASPHALT SHALL BE REPLACED PER PROJECT DETAILS AND NODOT STANDARDS AND SPECIFICATIONS
- PER PROJECT DETAILS AND NOODT STANDARDS AND SPECIFICATIONS. RAISE ALL STRUCTURES AT NO ADDITIONAL COST TO THE OWNER SO THAT THE TOP OF THE STRUCTURE EQUALS THE FINISH, GRADE OF THE NEW ASPHALT. COORDINATE THE PROJECT SCHEDULE WITH THE CIVIL ENGINEER AND OWNER. ANY UTILITY INTERRUPTIONS SHALL BE COOPENIATED WITH THE OWNER AT LEAST 72 HOURS PRIOR TO THE INTERRUPTION. PROVIDE A SHOOTH TRANSITION BETWEEN THE WAS THAT AND EXISTING PROVIDE A SHOOTH TRANSITION BETWEEN THE WAS THAT LAND EXISTING THE OWNER AT LEAST 72 HOURS PRIOR TO THE INTERRUPTION.

- PROVIDE A SMOOTH TRANSHIPS SET THE ASPHALT SUPPACES.
  ALL SIDEWALKS ARE TO HAVE NO MORE THAN A 1:20 (5.0%) SLOPE FOR THE LENGTH OF THE SIDEWALK AND NO MORE THAN A 1:50 (2.0%) SLOPE FOR THE WIDTH OF THE SIDEWALK NOTIFY ENGINEER IF THIS CANNOT OF ADMICTS.
- BE ACHIEVED. REFER TO SHEET C2.0 FOR THE CTP COMPLIANCE DIAGRAM.



#### TRAFFIC CONTROL NOTES

10-FT X 70-FT SIGHT TRIANGLE

- ALL SITE SIGNAGE SHALL BE IN ACCORDANCE WITH THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (MUTCD) AND NODOT STANDARDS.
- PALE FAVEMENT MARKINGS WITHIN RIGHT-OF-WAY SHALL BE THERWOPLASTIC. ALL PAVEMENT MARKINGS SHALL BE IN ACCORDANCE WITH NODOT STANDARDS AND THE PROJECT SPECIFICATIONS, WHICHEVER IS MOR STRINGENT.
- STRINGENT. ALL SIGNAGE SHALL HAVE HIGH INTENSITY PRISMATIC SHEETING.
- ALL SIGNACE SHALL HAVE HICH INTENSITY PRISMATIG SHEETING. SIGNS SHALL BE FABRICATED FROM ALLIMINUM ALLOY S-EETS. ALL MOUNTING HARDWARE SHALL BE GALVANIZED. "VARS" ON FLAND SEMOTIES VAN ACCESSIBLE PARKING SIGN. INSTALL R.P.B., R.P.B.), & R.P.B.P. "APS" ON PLANS DEMOTIES ACCESSIBLE PARKING SIGN. INSTALL R7-BA
- & R7-8D ONLY.

#### STRIPING KEY NOTES

	NCDOT STD.	WIDTH	COLOR
HIGH VISIBILITY CROSSWALK MARKINGS	-	-	WHITE
TRAFFIC ARROWS	1205.08	-	WHITE
STOP BAR	1205.01	24-IN	WHITE
LANE LINE	1205.1	4-IN	WHITE
EDGE LINE	1205.1	4-IN	WHITE
DIAGANOL LÍNES (2:1 SLOPE 45-FT SPACÍNG)	1205.1	8-IN	YELLOW
DOUBLE YELLOW CENTER LINE	1206.1	4-IN (EA)	YELLOW
2'-6'SP MINI SKIP LINE	1205.1	4-IN	WHITE
3'-9'/SP MINI SKIP LINE	1205.1	4-IN	WHITE
MULTI-LANE WIDTH "SCHOOL"	1205.8	-	WHITE



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PLANS

STRIPING FACILITY ু ∞ - NORTH CAROLIN

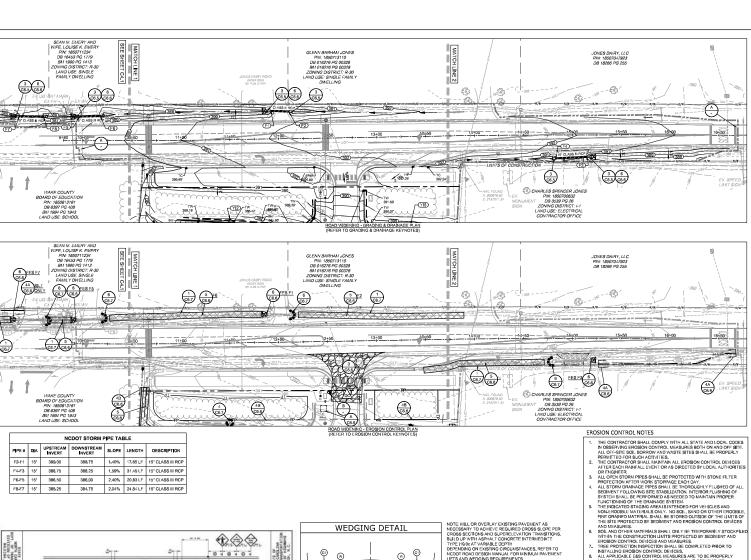
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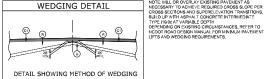
- WAKE COUNTY - NOR.

, SITE AND SIG DEMO, S DAIRY JONES

ROAD WIDENING PLAN -

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	PAVEMENT SCHEDULE				
	C1	PROP, APPROX, 1,5" ASPHALT CONCRETE SURFACE COURSE, TYPE \$9.5C.			
	w	PROP. VAR. DEPTH ASPHALT CONCRETE SURFACE COURSE, TYPE \$9.5C.			
	U	EXISTING PAVEMENT			
_					

	NCDOT STORM STRUCTURE TABLE				
	STRUCTURE #	TOP	STRUCTURE HEIGHT	DESCRIPTION	
	F1	390.44	N/A	15" FES	
	F2	390.19	N/A	15" FES	
	F3	389.69	N/A	15" FES	
4	F4	390,19	N/A	15" FES	
\$2 <sub>3</sub>	F5	387.44	N/A	15" FES	
7	F6	387.94	N/A	15" FES	
SCALE 1"=30"	F7	386.19	N/A	15" FES	
207 607	F8	386.69	N/A	15" FES	

ALL PRELOGIES CONTINUE MERSONES AND TO SERVICE TO BE PROVIDED TO SERVICE THE MAINTAINED UNTIL PERMANENT VEGETATION IS ESTRABLISHED.

PERMANENT GROUNDCOVER SHALL BE PROVIDED FOR ALL DISTURBED AREAS WITHIN 15 WORKING DAYS OR NO MORE THAN 90 CALENDAR DAYS (WHICHEVER IS SHORTER).

TOTAL DISTURBED AREA. 593 AC.

#### GRADING & DRAINAGE NOTES

- COORDINATE THE PROJECT SCHEDULE WITH THE OWNER, AND COORDINATE THE PROJECT SCHEDULE WITH THE OWNER, AND ADJACENT USERS OF THE PROPERTY, MAINTAN TRAFFICE FLOW AND DO OPERATIONS OF ADJACENT SITES AND FACILITIES AND OR THEIR OWNERS ONGOING OPERATIONS.

  ALL EXISTING VALUES, MANHOLES, STORM DRAIN STRUCTURES, VALVE BOXES, CLEANOUTS, ETC. SHALL BE ADJUSTED AN BEGED TO MATCH

- BOXES, CLEMOUTS, ETC. SHALL BE ADJUSTED AS NEEDED TO HATCH FINISHED CRADE, PRINCED CRADE, SINISHING HEAD FROM THE PRINCED TO HATCH FINISH CRADE AND THE CONNERS INDEPENDENT TESTING LAGORATORY. ALL SPOT LEXATIONS INDICATED ARE AT TOP OF CURB UNLESS NOTED OTHERWISE.

  SPOT CRADE ABSREVIATIONS.
- POT GRADE ABBREVIATIONS:
  TC. TOP OF CURB
  EP. EDGE OF PAVEMENT
  HIP HIGH POINT
  SVIK: SIDEWALK
  FFE: FINISHED FLOOR ELEVATION
  TW: TOP OF WALL
  BW: BOTTOM WALL
  FL. FLOW LINE
  GND. GROUND

- EX GND: EXISTING GROUND
- 4.10. LX OND EXISTING OROUND
  7. SPOT ELEANTONS ARE GIVEN AT THE MAJORITY OF THE MAJOR SPEAK POINTS BUT IT SHOULD NOT BE ASSUMED THAT ALL NECESSARY SPOT ELEVATIONS ARE SHOWN. DUE TO SPACE LIMITATIONS. THERE MAY BE CONSIDERATED AND SHOULD SHO

- SPECIFICATIONS.

  PROVIDE HALF-SENCH CONCRETE INLET SHAPING FOR ALL CONCRETE STORM SEWER STRUCTURES.

  ALL ROOF DRAINS FROM BUIDLING A' SHALL BE 8" PVC (SCH 40) @ 1.04%.
- MILL ROOF DRAINS FROM BUILDING A STACLE DE PVC (SCH 30) BY 1004 MIN. SLOPE UNLESS INDICATED OTHERWISE. ALL ROOF DRAINS FROM BUILDING 'S', C', AND 'D', SHALL BE 6' PVC (SCH 40) <u>6</u>1.04% MIN. SLOPE UNLESS INDICATED OTHERWISE. USE DUTLE IRON WHEN COVER IS LESS THAN 24-IN, MATCH PIPE CROWNS WITH CONNECTION TO DROP MILET.
- INLET.
  PVC ROOF DRAIN PIPING UNDER PAVEMENT SHALL HAVE 24-IN MINIMUM
  COVER, IF ROOF DRAIN PIPING UNDER PAVEMENT HAS LESS THAN 24-IN
  COVER, ROOF DRAIN PIPING SHALL BE 8'D IF UN LEU OF PVC).
  JOINT FILL AND CALLK EACH CONCRETE EXPANSION JOINT AND VHERE
  CONCRETE PAVEMENT ABUTS OTHER PAVEMENTS, SIDEWALKS, OF

- JOHN THE AND CALLY EAGH CONCRETE CPANISON, JOHN THEO WHEELERS, BECKNESSES, WELL AND CONCRETE PRAYED AND STORM THE WHEELENS, BECKNESSES, OR CONCRETE PRAYED AND STORM THE AND T

- CONSTRUCTION TO MAINTAIN PROPER PLUCTIONING OF THE DANIAGE
  SYSTAM.

  NO INSTRUCTION OF THE SAME TO BE NOTICES OF LAWN AREAS WITH

  NO INSTRUCTION OF THE SIDE OF THE NOTICES OF LAWN AREAS WITH

  ALL SIDEWALK ARE TO THAKE NO MORE THAN A 120 G.M.) SLOPE FOR
  THE LENGTH OF THE SIDEWALK AND NO MORE THAN A 130 G.M.) SLOPE FOR
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  THE CONTROL OF THE SIDEWALK AND NO MORE THAN A 130 G.M.) SLOPE

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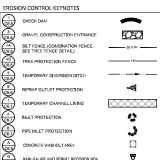
  OF ALL UTILITY SYSTEMS AND STORM SEWING TO MAIN AN ASSAULT SIMPLEY

  OF ALL UTILITY SYSTEMS AND STORM SEWING THE SHEW STORY SHALL

  HIS PARE DANIAGE DATING DESIGNATION OF CONSTRUCTION SHALL

  SHOW THAN A 130 G.M. OF THE STORY SHALL SHAL CONTRACTOR AT NO ADDITIONAL COST TO THE OWNER

RADING & DRAINAGE KEY NOTES				
3 C6.5	FLARED END SECTION			
(6.6)	RIP RAP OUTLET PROTECTION (SEE EROSION CONTROL PLAN)			
$\overline{\wedge}$	DEPONING ENGATE DAVING TRANSPICAL			



BAIN GALIGE AND PERMIT RECORDS BOX

LIMITS OF CONSTRUCTION

EXISTING CONTOUR FINISHED CONTOUR DUST CONTROL



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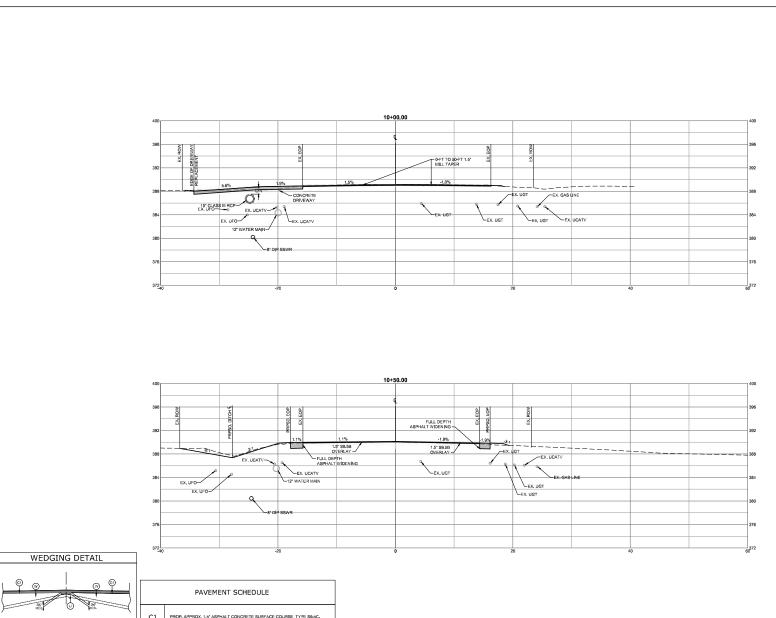
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WIDENING ROAD \

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THIS DRAWING PREPARED ATTHE RALEIGH OFFICE 5410 Trinty Road, Suite 102 | Paleigh, NC 27607 TEI 919,866,4951 FAX 919,833,8124 www.timmons.

DATE 06/01/2023 9 08/01/2023 11/01/2023

04/05/2023 L. BARNES

DESIGNED BY G. FRANK CHECKED BY G. FRANK

SCALE

AS SHOWN

JONES DAIRY STORAGE FACILITY
TOWN OF ROLES/ILE - WAKE COUNTY - NORTH CAROLINA
JONES DAIRY ROAD - CROSS SECTIONS STA. 10+00 - 10+50
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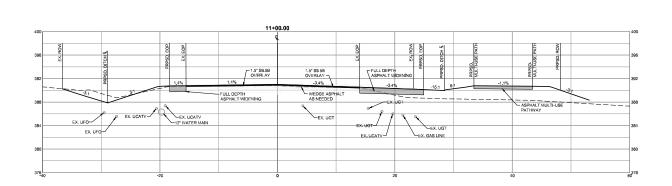
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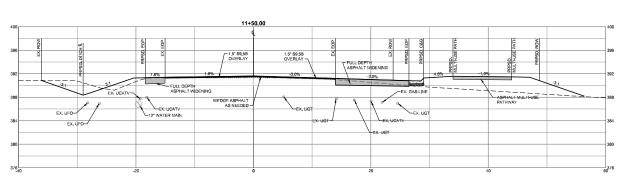
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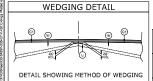
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NOTE: MILL OR OVERLAY EXISTING PAVEMENT AS NECESSARY TO ACHEVE REQUIRED CROSS SLOWE PER SULD UP WITH SAPILAT CONCEPT ENTERHEDIATE TYPE TISO AT VARIBALE DEPTH DEPENDING OR AUSTRICA CONCEPT ON DOD TO ADD DESIGN OF THE OWNER OF THE OWNER OF THE OWNER OWNE

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W	PROP. VAR. DEPTH ASPHALT CONCRETE SURFACE COURSE, TYPE \$9.5C.	
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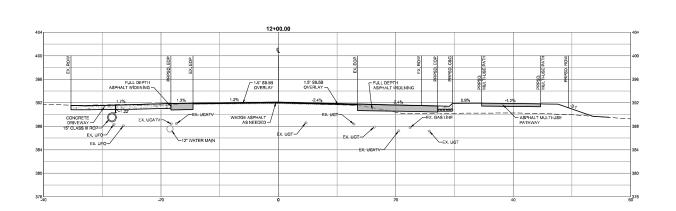
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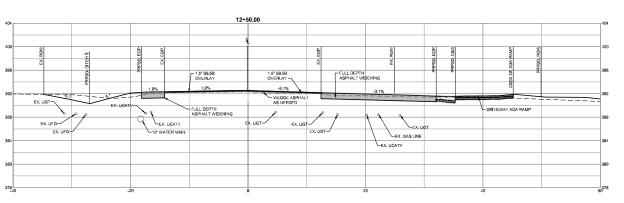
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JONES DAIRY STORAGE FACILITY
TOWN OF DOLES/ILLE - WARE COUNTY - HORTH CAROLINA
JONES DAIRY ROAD - CROSS SECTIONS STA. 11+600 - 11+500
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w	PROP. VAR. DEPTH ASPHALT CONCRETE SURFACE COURSE, TYPE \$9.5C.		
U	EXISTING PAVEMENT		



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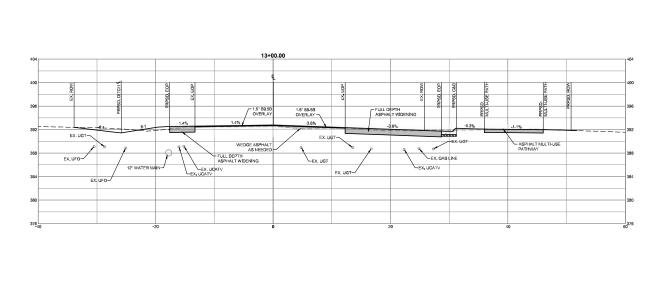
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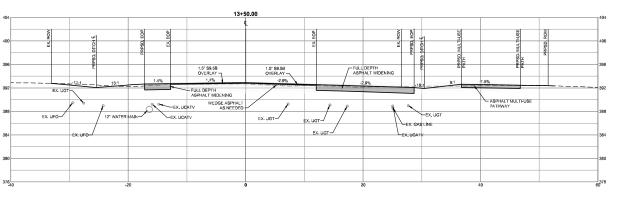
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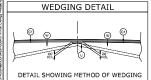
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JONES DAIRY STORAGE FACILITY
TOWN OF ROLESCILLE - WARE COUNTY - NORTH CAROLINA
JONES DAIRY ROAD - CROSS SECTIONS STA. 12+00 - 12+50
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W	PROP. VAR. DEPTH ASPHALT CONCRETE SURFACE COURSE, TYPE \$9.5C.		
U	EXISTING PAVEMENT		



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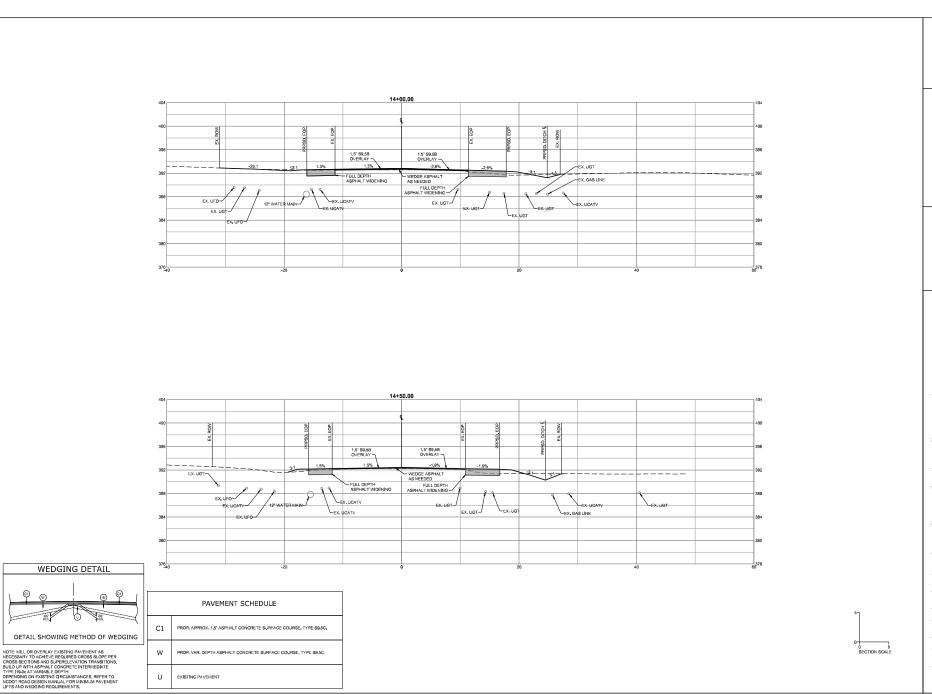
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JONES DAIRY STORAGE FACILITY
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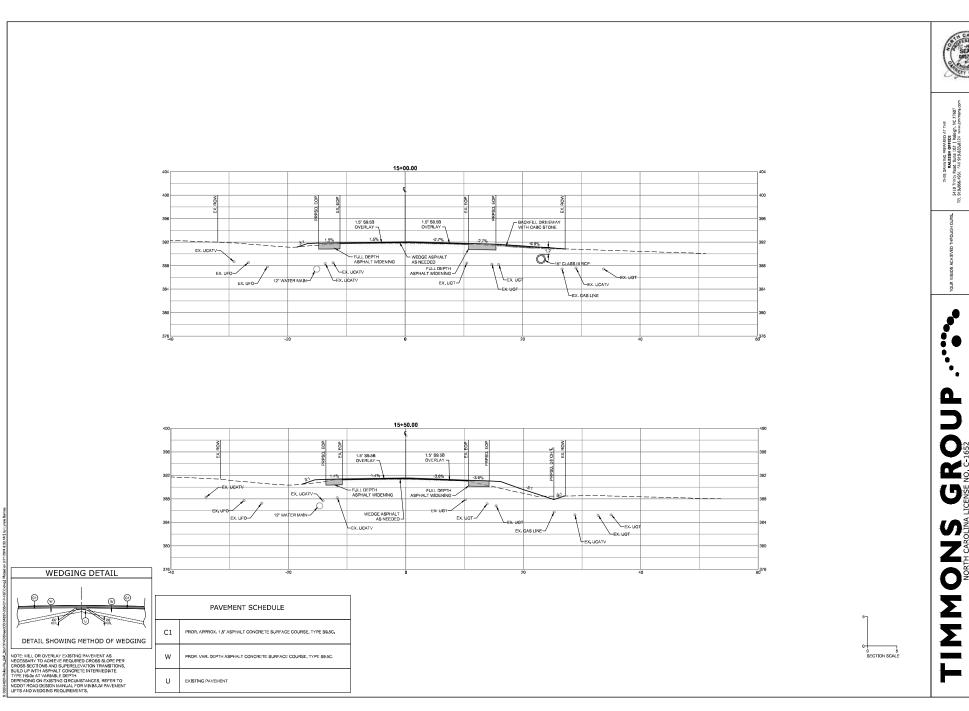
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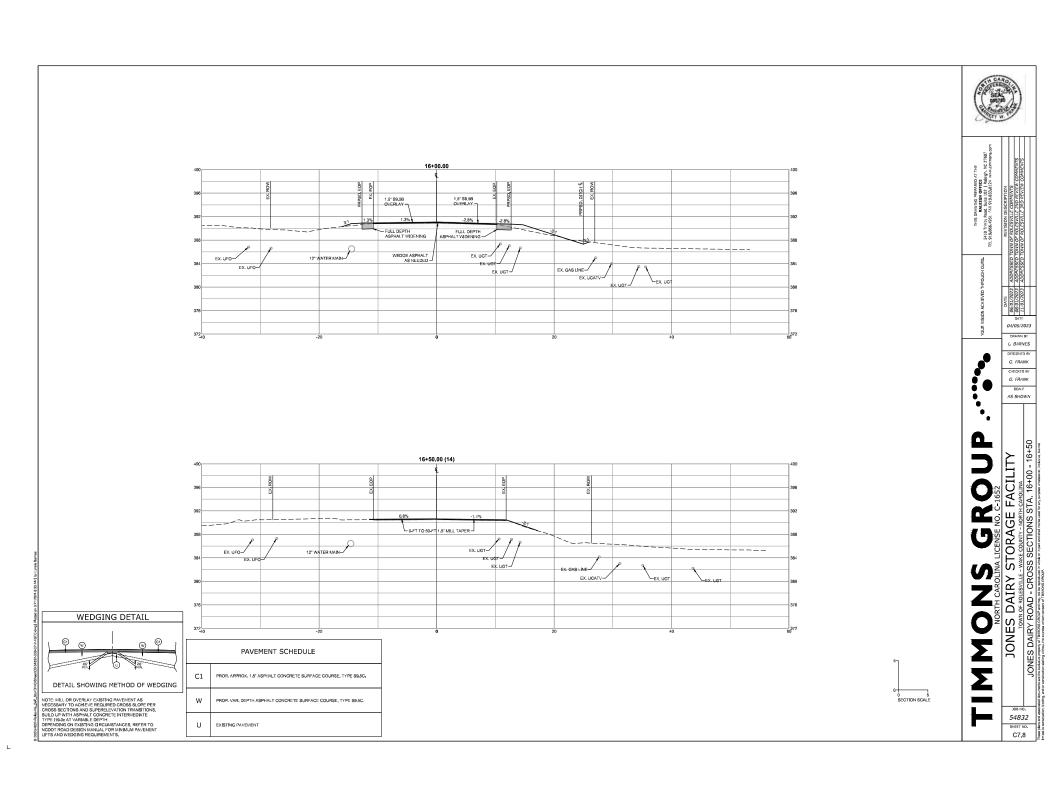
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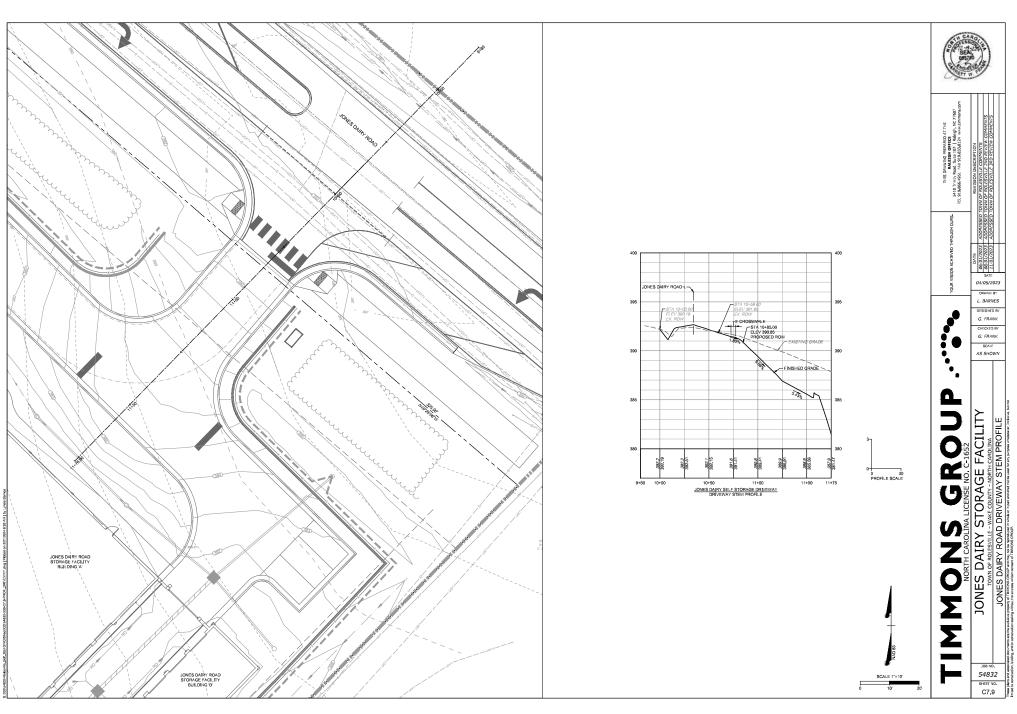
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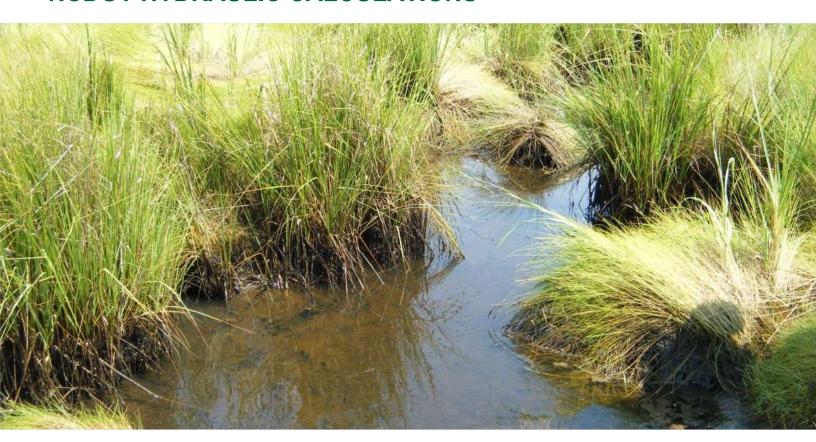
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TOWN OF ROLES/ILLE - WARK COUNTY - NORTH CAROLINA
JONES DAIRY ROAD - CROSS SECTIONS STA. 15+00 - 15+50
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# JONES DAIRY STORAGE FACILITY NCDOT HYDRAULIC CALCULATIONS



**JANUARY 25, 2023** 

#### PREPARED BY:

Garrett Frank, PLA, PE 5410 Trinity Road, Suite 102 Raleigh, North Carolina 27607 919.866.4503 phone 919.859.5663 fax Garrett.frank@timmons.com www.timmons.com





#### **RIP RAP OUTLET DESIGN**



OUTLET PROTECTION	DATE
DESIGN	07/31/2023
PROJECT NAME	PROJECT NO
Rolesville Self Storage Facility	54832
LOCATION	BY
Rolesville, NC	LMB



#### **RIPRAP FES F1**

Pipe Diameter: 15.00 in 389.00 Lower Invert: 388.75 Pipe Length: 17.45 ft

Pipe Slope: 0.014 Discharge:

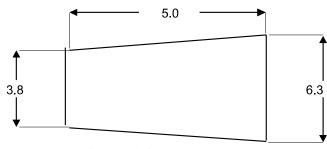
Pipe Material: RCP

Q10 = 0.47 cfs Qfull = 7.74 cfs Vfull = 6.31 fps Q10/Qfull = 0.06 V/Vfull = 0.52 V = 3.29 fps

0.47 cfs

Dissipator Dimensions \* Zone = 1
Stone Filling Class = A

Entry Width  $(3 \times D_0)$  = 3.8 ft Length  $(4 \times D_0)$  = 5.0 ft Width  $(La + D_0)$  = 6.3 ft Min. Thickness = 12 inches Stone Diameter  $(D_{50})$  = 3 inches



\*Discharge from Storm Sewers

\* All units are in feet

8/3/2023 Page 4 of 5

<sup>\*\*</sup> Dissipator pad designed for full flow of pipe

OUTLET PROTECTION	DATE
DESIGN	07/31/2023
PROJECT NAME	PROJECT NO
Rolesville Self Storage Facility	54832
LOCATION	ВҮ
Rolesville, NC	LMB



#### **RIPRAP FES F3**

15.00 Pipe Diameter: in Upper Invert: 389.00 Lower Invert: 388.50 Pipe Length: 31.45

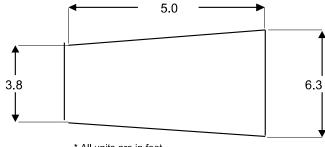
\*Discharge from Storm Sewers Pipe Slope: 0.016 Discharge: 0.32 cfs

Pipe Material: RCP

Q10 = 0.32 cfs  $Q_{10}/Q_{full} =$ 0.04 Qfull = V/Vfull = 0.42 8.16 cfs 2.81 fps Vfull = 6.65 fps V =

Dissipator Dimensions \* Zone = Stone Filling Class = Α

Entry Width (  $3 \times D_0$  ) = 3.8 ft Length (  $4 \times D_0$  ) = 5.0 ft Width (La +  $D_0$ ) = 6.3 ft Min. Thickness = 12 inches Stone Diameter (D<sub>50</sub>)= 3 inches



\* All units are in feet

8/3/2023 Page 5 of 5

<sup>\*\*</sup> Dissipator pad designed for full flow of pipe

OUTLET PROTECTION	DATE
DESIGN	01/19/2024
PROJECT NAME	PROJECT NO
Rolesville Self Storage Facility	54832
LOCATION	ВҮ
Rolesville, NC	LMB



0.15

0.72

6.51 fps

#### **RIPRAP FES F5**

 Pipe Diameter:
 15.00 in

 Upper Invert:
 387.00

 Lower Invert:
 386.40

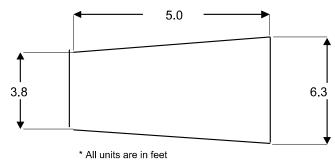
 Pipe Length:
 20.35 ft

Pipe Length: 20.35 ft \*Discharge from Storm Sewers
Pipe Slope: 0.029 triangle to the storm Sewers
Discharge: 1.70 cfs

Pipe Material: RCP

 $Q_{10} = 1.70 \text{ cfs}$   $Q_{10}/Q_{full} = Q_{full} = 11.11 \text{ cfs}$   $V/V_{full} = V_{full} = 9.05 \text{ fps}$  V = 0.00

Dissipator Dimensions \* Zone =  $\frac{1}{A}$ Stone Filling Class =  $\frac{1}{A}$ Entry Width  $(3 \times D_0)$  = 3.8 ft



\*\* Dissipator pad designed for full flow of pipe

1/19/2024 Page 6 of 7

OUTLET PROTECTION	DATE
DESIGN	01/19/2024
PROJECT NAME	PROJECT NO
Rolesville Self Storage Facility	54832
LOCATION	ву
Rolesville, NC	LMB



0.21

0.78

5.88 fps

#### **RIPRAP FES F7**

15.00 Pipe Diameter: in Upper Invert: 385.00 Lower Invert: 384.50 Pipe Length: 24.50

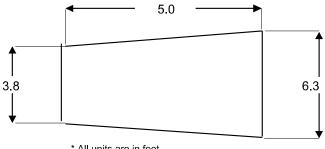
\*Discharge from Storm Sewers Pipe Slope: 0.020 Discharge: 1.90 cfs

Pipe Material: RCP

Q10 = 1.90 cfs Q10/Qfull = Qfull = V/Vfull = 9.24 cfs Vfull = 7.53 fps V =

Dissipator Dimensions \* Zone = Stone Filling Class = Α

Entry Width (  $3 \times D_0$  ) = 3.8 ft Length (  $4 \times D_0$  ) = 5.0 ft Width (La +  $D_0$ ) = 6.3 ft Min. Thickness = 12 inches Stone Diameter (D<sub>50</sub>)= 3 inches



\* All units are in feet

1/19/2024 Page 7 of 7

<sup>\*\*</sup> Dissipator pad designed for full flow of pipe

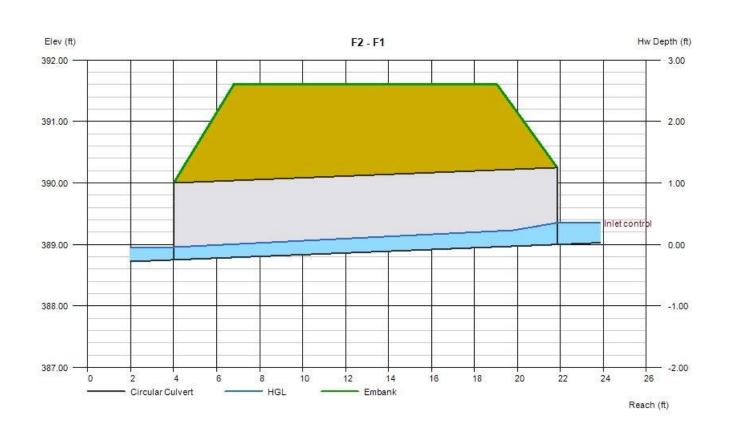
#### **CULVERT DESIGN**



Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

#### F2 - F1

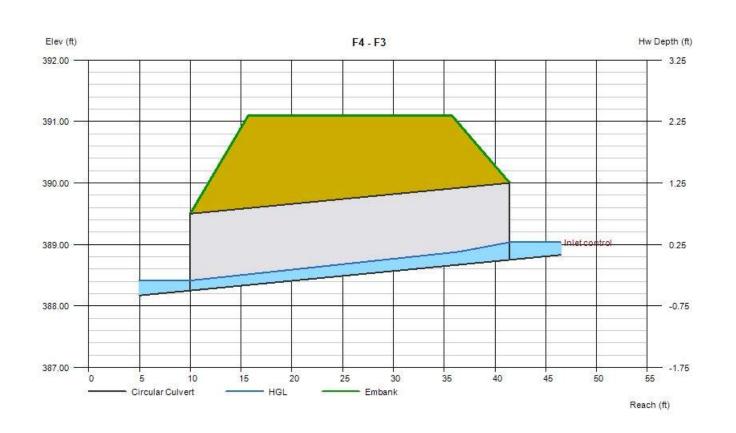
= 388.75	Calculations	
= 17.85	Qmin (cfs)	= 0.47
= 1.40	Qmax (cfs)	= 0.47
= 389.00	Tailwater Elev (ft)	= Normal
= 15.0	` '	
= Circular	Highlighted	
= 15.0		= 0.47
= 1	,	= 0.47
= 0.012	Qovertop (cfs)	= 0.00
= Circular Concrete	Veloc Dn (ft/s)	= 3.57
= Square edge w/headwall (C)	Veloc Up (ft/s)	= 2.45
= 0.0098, 2, 0.0398, 0.67, 0.5	HGL Dn (ft)	= 388.96
	HGL Up (ft)	= 389.27
	Hw Elev (ft)	= 389.35
= 391.60	Hw/D (ft)	= 0.28
= 12.25	Flow Regime	= Inlet Control
= 12.25	-	
	= 17.85 = 1.40 = 389.00 = 15.0 = Circular = 15.0 = 1 = 0.012 = Circular Concrete = Square edge w/headwall (C) = 0.0098, 2, 0.0398, 0.67, 0.5 = 391.60 = 12.25	= 17.85 Qmin (cfs) = 1.40 Qmax (cfs) = 389.00 Tailwater Elev (ft) = 15.0 = Circular Highlighted = 15.0 Qtotal (cfs) = 1 Qpipe (cfs) = 0.012 Qovertop (cfs) Circular Concrete Veloc Dn (ft/s) = Square edge w/headwall (C) Veloc Up (ft/s) = 0.0098, 2, 0.0398, 0.67, 0.5 HGL Dn (ft) HGL Up (ft) HW Elev (ft) HW/D (ft) = 391.60 Hw/D (ft) Flow Regime



Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

#### F4 - F3

= 388.25	Calculations	
= 31.45	Qmin (cfs)	= 0.32
= 1.59	Qmax (cfs)	= 0.32
= 388.75	Tailwater Elev (ft)	= Normal
= 15.0		
= Circular	Highlighted	
= 15.0	Qtotal (cfs)	= 0.32
= 1	Qpipe (cfs)	= 0.32
= 0.012	Qovertop (cfs)	= 0.00
<ul><li>Circular Concrete</li></ul>	Veloc Dn (ft/s)	= 3.38
<ul><li>Square edge w/headwall (C)</li></ul>	Veloc Up (ft/s)	= 2.21
= 0.0098, 2, 0.0398, 0.67, 0.5	HGL Dn (ft)	= 388.41
	HGL Up (ft)	= 388.97
	Hw Elev (ft)	= 389.04
= 391.10	Hw/D (ft)	= 0.23
= 20.00	Flow Regime	= Inlet Control
= 20.00	-	
	= 31.45 = 1.59 = 388.75 = 15.0 = Circular = 15.0 = 1 = 0.012 = Circular Concrete = Square edge w/headwall (C) = 0.0098, 2, 0.0398, 0.67, 0.5 = 391.10 = 20.00	= 31.45 Qmin (cfs) = 1.59 Qmax (cfs) = 388.75 Tailwater Elev (ft) = 15.0 = Circular Highlighted = 15.0 Qtotal (cfs) = 1 Qpipe (cfs) = 0.012 Qovertop (cfs) = Circular Concrete Veloc Dn (ft/s) = Square edge w/headwall (C) Veloc Up (ft/s) = 0.0098, 2, 0.0398, 0.67, 0.5 HGL Dn (ft) HGL Up (ft) HW Elev (ft) = 391.10 Hw/D (ft) = 20.00 Flow Regime



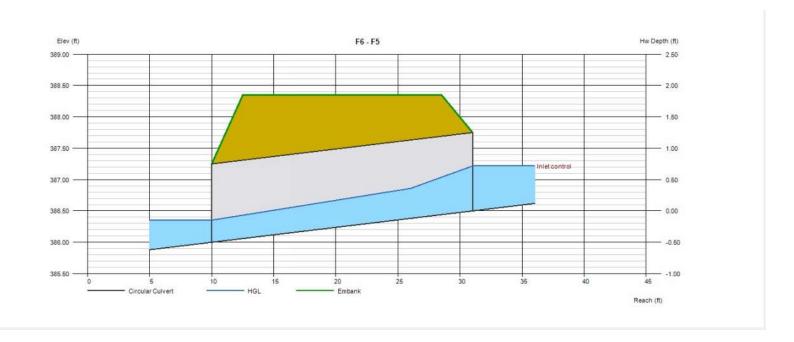
# **Culvert Report**

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Friday, Jan 26 2024

#### F6 - F5

Invert Elev Dn (ft)	= 386.00	Calculations	
Pipe Length (ft)	= 21.01	Qmin (cfs)	= 1.70
Slope (%)	= 2.38	Qmax (cfs)	= 1.70
Invert Elev Up (ft)	= 386.50	Tailwater Elev (ft)	= Normal
Rise (in)	= 15.0		
Shape	= Circular	Highlighted	
Span (in)	= 15.0	Qtotal (cfs)	= 1.70
No. Barrels	= 1	Qpipe (cfs)	= 1.70
n-Value	= 0.012	Qovertop (cfs)	= 0.00
Culvert Type	<ul><li>Circular Concrete</li></ul>	Veloc Dn (ft/s)	= 5.98
Culvert Entrance	<ul><li>Square edge w/headwall (C)</li></ul>	Veloc Up (ft/s)	= 3.55
Coeff. K,M,c,Y,k	= 0.0098, 2, 0.0398, 0.67, 0.5	HGL Dn (ft)	= 386.35
		HGL Up (ft)	= 387.02
Embankment		Hw Elev (ft)	= 387.22
Top Elevation (ft)	= 388.35	Hw/D (ft)	= 0.57
Top Width (ft)	= 16.00	Flow Regime	= Inlet Control
Crest Width (ft)	= 16.00		



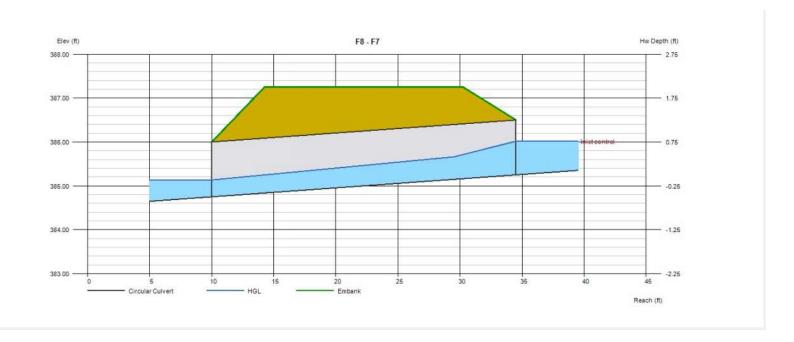
# **Culvert Report**

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Friday, Jan 26 2024

#### F8 - F7

Invert Elev Dn (ft)	= 384.75	Calculations	
Pipe Length (ft)	= 24.50	Qmin (cfs)	= 1.89
Slope (%)	= 2.04	Qmax (cfs)	= 1.89
Invert Elev Up (ft)	= 385.25	Tailwater Elev (ft)	= Normal
Rise (in)	= 15.0		
Shape	= Circular	Highlighted	
Span (in)	= 15.0	Qtotal (cfs)	= 1.89
No. Barrels	= 1	Qpipe (cfs)	= 1.89
n-Value	= 0.012	Qovertop (cfs)	= 0.00
Culvert Type	<ul><li>Circular Concrete</li></ul>	Veloc Dn (ft/s)	= 5.91
Culvert Entrance	= Square edge w/headwall (C)	Veloc Up (ft/s)	= 3.66
Coeff. K,M,c,Y,k	= 0.0098, 2, 0.0398, 0.67, 0.5	HGL Dn (ft)	= 385.13
		HGL Up (ft)	= 385.80
Embankment		Hw Elev (ft)	= 386.02
Top Elevation (ft)	= 387.25	Hw/D (ft)	= 0.61
Top Width (ft)	= 16.00	Flow Regime	= Inlet Control
Crest Width (ft)	= 16.00		



#### **CHANNEL DESIGN**









#### PERMANENT DIVERSION DITCH: F2

This worksheet is designed to determine channel lining based on flow and ditch geometry (base width, side slope, & channel slope). This program determines depth of flow, velocity and tractive force assuming n=0.03 for grass or rip rap (n can be varied if desired). (Vary depth until z actual equals z required; v & t will be correct; select appropriate lining based on tractive force)

#### **Watershed Data**

Area (A) = **0.17** (Acres)

Coef. (C) = 0.60 (Dimensionless)

Inte. (I) = 7.19 (in/hr) Flow (Q<sub>10</sub>) = 0.7 (cfs)

#### Known Quantities

Line channel with: 6 " rip-rap (Assume 6" even if using grass)

Manning n = 0.030 (Dimensionless) Ditch Length = 122 LF Slope = 0.030 (ft/ft) Highest Elevation = 392.6 Z Required = 0.08 Ratio Lowest Elevation = 389

Side Slope (M) = 3:1

#### Variable Quantities (Ditch Width & Depth)

Flow Depth (Y) = 0.32 ft 3.8 in
Bottom Width (B) = 0.00 ft 0.0 in
Freeboard = 0.00 ft 0.0 in

#### **Compute Tractive Force**

T=YxDxS

T =

Y = Weight of water (62.4 LB/CUFT)
0.59 LB/SF D=Depth of flow in channel (ft)

S=Slope of channel (ft/ft)

General Lining	LB/SF
Jute Net	0.45
Curled Mat	1.55
Class A (4"@9"THK)	2.00
Class B (8"@18"THK)	3.50
Class I (12"@22"THK)	5.00
Class II (18"@30"THK)	7.50

North American	
Green Lining	LB/SF
S75	1.55
S150	1.75
SC150	2.10
C125	2.25
P300	8.00

#### Determine If Z Actual Is Greater Than Z Required

New A = 0.31 SF

New P = 2.02 Wetted perimeter New R = 0.15 Hydraulic radius

Z Actual = 0.09 Must be greater than z required=> 0.08 Okay

New V = 2.43 fps

# Minimum Design Geometry Permanent Liner: Grass Temporary Liner: Curled Mat Top Width = 1.92 ft Min. Ditch Depth = 0.32 ft Bottom Width (B) = 0 ft Top Width = 1.92 ft Flow Depth (Y) = 0.32 ft

	Di	tch Geometry		
Min. Ditch Depth (ft)	Bottom Width (ft)	Flow Depth (ft)	Top Width (ft)	Side Slope
0.32	0.00	0.32	1.92	3:1

#### Ditch Liner

Permanent Liner:Grass,Temporary Liner:Curled Mat







#### PERMANENT DIVERSION DITCH: F4

This worksheet is designed to determine channel lining based on flow and ditch geometry (base width, side slope, & channel slope). This program determines depth of flow, velocity and tractive force assuming n=0.03 for grass or rip rap (n can be varied if desired). (Vary depth until z actual equals z required; v & t will be correct; select appropriate lining based on tractive force)

#### **Watershed Data**

Area (A) = 0.09 (Acres)

**0.60** (Dimensionless) Coef. (C) =

Inte. (I) = **7.19** (in/hr) Flow  $(Q_{10}) =$ **0.4** (cfs)

#### Known Quantities

Line channel with: 6 " rip-rap (Assume 6" even if using grass)

Manning n = 0.030 (Dimensionless) Ditch Length = 132 LF Slope = 0.026 (ft/ft) Highest Elevation = 392.2 Z Required = 0.05 Ratio Lowest Elevation = 388.75

Side Slope (M) = 3:1

#### Variable Quantities (Ditch Width & Depth)

Flow Depth (Y) = 3.2 in **0.27** ft Bottom Width (B) = 0.00 ft 0.0 in Freeboard = 0.00 ft 0.0 in

#### **Compute Tractive Force**

T=YxDxS

Y = Weight of water (62.4 LB/CUFT) T = 0.44 LB/SF D=Depth of flow in channel (ft) S=Slope of channel (ft/ft)

General Lining LB/SF Jute Net 0.45 Curled Mat 1.55 Class A (4"@9"THK) 2.00 Class B (8"@18"THK) 3.50 Class I (12"@22"THK) 5.00 Class II (18"@30"THK) 7.50

North American	
Green Lining	LB/SF
S75	1.55
S150	1.75
SC150	2.10
C125	2.25
P300	8.00

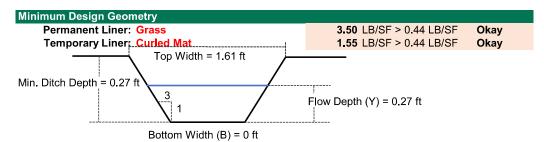
#### Determine If Z Actual Is Greater Than Z Required

New A = 0,22 SF

New P = 1.70 Wetted perimeter New R = 0.13 Hydraulic radius

Z Actual = **0.06** Must be greater than z required=> **0.05** Okay

New V =



	Di	tch Geometry		
Min. Ditch Depth (ft)	Bottom Width (ft)	Flow Depth (ft)	Top Width (ft)	Side Slope
0.27	0.00	0.27	1.61	3:1

#### Ditch Liner

Permanent Liner: Grass, Temporary Liner: Curled Mat







#### PERMANENT DIVERSION DITCH: F6

This worksheet is designed to determine channel lining based on flow and ditch geometry (base width, side slope, & channel slope). This program determines depth of flow, velocity and tractive force assuming n=0.03 for grass or rip rap (n can be varied if desired). (Vary depth until z actual equals z required; v & t will be correct; select appropriate lining based on tractive force)

#### **Watershed Data**

Area (A) = **0.39** (Acres)

Coef. (C) = 0.60 (Dimensionless)

Inte. (I) = 7.19 (in/hr) Flow ( $Q_{10}$ ) = 1.7 (cfs)

#### Known Quantities

Line channel with: 6 " rip-rap (Assume 6" even if using grass)

Manning n =0.030 (Dimensionless)Ditch Length =170 LFSlope =0.010 (ft/ft)Highest Elevation =388.75Z Required =0.34 RatioLowest Elevation =387

Side Slope (M) = 3:1

#### Variable Quantities (Ditch Width & Depth)

Flow Depth (Y) = 0.54 ft 6.5 in
Bottom Width (B) = 0.00 ft 0.0 in
Freeboard = 0.00 ft 0.0 in

#### **Compute Tractive Force**

T=YxDxS

XS Y = Weight of water (62.4 LB/CUFT)
T = 0.35 LB/SF D=Depth of flow in channel (ft)

S=Slope of channel (ft/tt)

S=Slope of channel (ft/ft)

General Lining	LB/SF
Jute Net	0.45
Curled Mat	1.55
Class A (4"@9"THK)	2.00
Class A (4"@9"THK) Class B (8"@18"THK) Class I (12"@22"THK) Class II (18"@30"THK)	3.50
Class I (12"@22"THK)	5.00
Class II (18"@30"THK)	7.50

North American	
Green Lining	LB/SF
S75	1.55
S150	1.75
SC150	2.10
C125	2.25
P300	8.00

#### Determine If Z Actual Is Greater Than Z Required

New A = 0.87 SF

New P = 3.41 Wetted perimeter New R = 0.26 Hydraulic radius

Z Actual = 0.35 Must be greater than z required=> 0.34 Okay

New V = 2.03 fps

# Minimum Design Geometry Permanent Liner: Grass Temporary Liner: Curled Mat. Top Width = 3.23 ft Min. Ditch Depth = 0.54 ft Bottom Width (B) = 0 ft

	Di	tch Geometry		
Min. Ditch Depth (ft)	Bottom Width (ft)	Flow Depth (ft)	Top Width (ft)	Side Slope
0.54	0.00	0.54	3.23	3:1

#### Ditch Liner

Permanent Liner:Grass,Temporary Liner:Curled Mat







#### PERMANENT DIVERSION DITCH: F8

This worksheet is designed to determine channel lining based on flow and ditch geometry (base width, side slope, & channel slope). This program determines depth of flow, velocity and tractive force assuming n=0.03 for grass or rip rap (n can be varied if desired). (Vary depth until z actual equals z required; v & t will be correct; select appropriate lining based on tractive force)

#### **Watershed Data**

Area (A) = **0.44** (Acres)

Coef. (C) = 0.60 (Dimensionless)

Inte. (I) = 7.19 (in/hr) Flow (Q<sub>10</sub>) = 1.9 (cfs)

#### Known Quantities

Line channel with: 6 " rip-rap (Assume 6" even if using grass)

Manning n =0.030 (Dimensionless)Ditch Length =30 LFSlope =0.047 (ft/ft)Highest Elevation =386.4Z Required =0.18 RatioLowest Elevation =385

Side Slope (M) = 3:1

#### Variable Quantities (Ditch Width & Depth)

Flow Depth (Y) = 0.43 ft 5.1 in
Bottom Width (B) = 0.00 ft 0.0 in
Freeboard = 0.00 ft 0.0 in

#### **Compute Tractive Force**

T=YxDxS

XS Y = Weight of water (62.4 LB/CUFT)
T = 1.25 LB/SF D=Depth of flow in channel (ft)
S=Slope of channel (ft/ft)

General Lining	LB/SF
Jute Net	0.45
Curled Mat	1.55
Class A (4"@9"THK) Class B (8"@18"THK)	2.00
Class B (8"@18"THK)	3.50
Class I (12"@22"THK)	5.00
Class II (18"@30"THK)	7.50

North American	
Green Lining	LB/SF
S75	1.55
S150	1.75
SC150	2.10
C125	2.25
P300	8.00

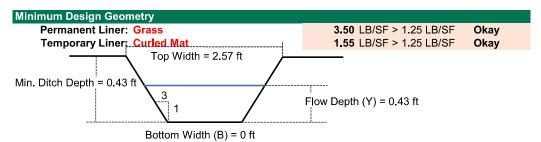
#### Determine If Z Actual Is Greater Than Z Required

New A = 0.55 SF

New P = 2.71 Wetted perimeter New R = 0.20 Hydraulic radius

Z Actual = 0.19 Must be greater than z required=> 0.18 Okay

New V = 3.71 fps



	Di	tch Geometry		
Min. Ditch Depth (ft)	Bottom Width (ft)	Flow Depth (ft)	Top Width (ft)	Side Slope
0.43	0.00	0.43	2.57	3:1

#### Ditch Liner

Permanent Liner:Grass,Temporary Liner:Curled Mat

#### **GUTTER SPREAD**



## **Channel Report**

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Monday, Jan 15 2024

#### **Gutter Spread Plan East**

Outlei		
Cross SI, Sx (ft/ft)	=	0.034
Cross SI, Sw (ft/ft)	=	0.020
Gutter Width (ft)	=	2.00
Invert Elev (ft)	=	391.72
Slope (%)	=	1.50
N-Value	=	0.013

#### Calculations

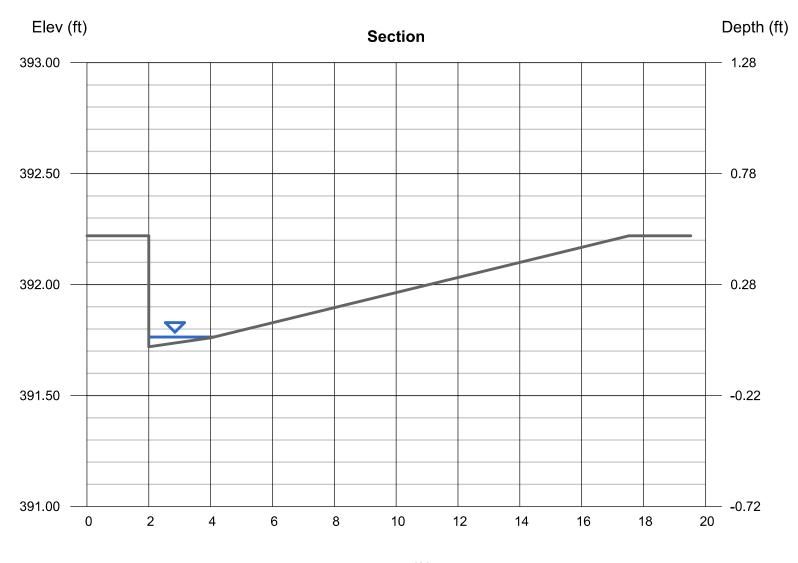
Gutter

Compute by: Known Q Known Q (cfs) = 0.06

# Highlighted Depth (ft) = 0.04 Q (cfs) = 0.060 Area (sqft) = 0.05 Velocity (ft/s) = 1.24

Velocity (ft/s) = 1.24Wetted Perim (ft) = 2.16Crit Depth, Yc (ft) = 0.06Spread Width (ft) = 2.12

EGL (ft) = 0.07



Reach (ft)

# **Channel Report**

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

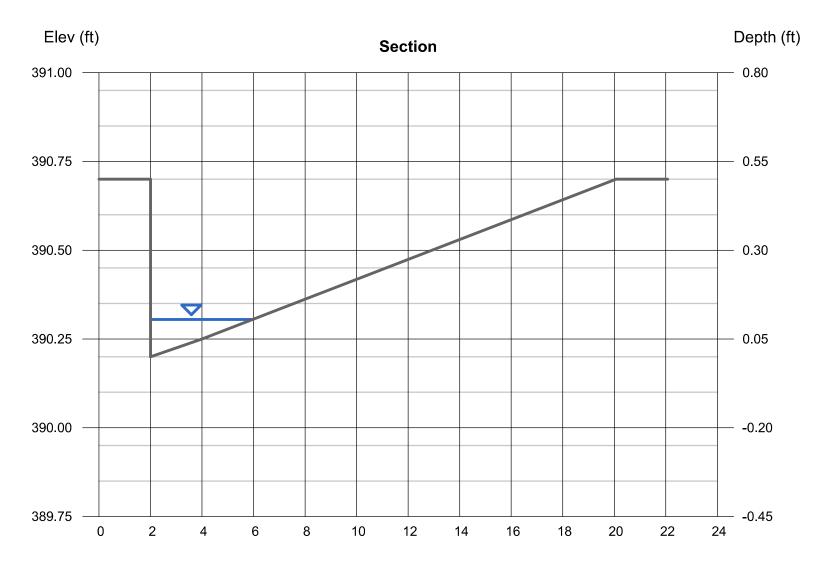
Monday, Jan 15 2024

#### **Gutter Spread Plan West**

Gutter		
Cross SI, Sx (ft/ft)	=	0.028
Cross SI, Sw (ft/ft)	=	0.025
Gutter Width (ft)	=	2.00
Invert Elev (ft)	=	390.20
Slope (%)	=	1.25
N-Value	=	0.013

**Calculations** 

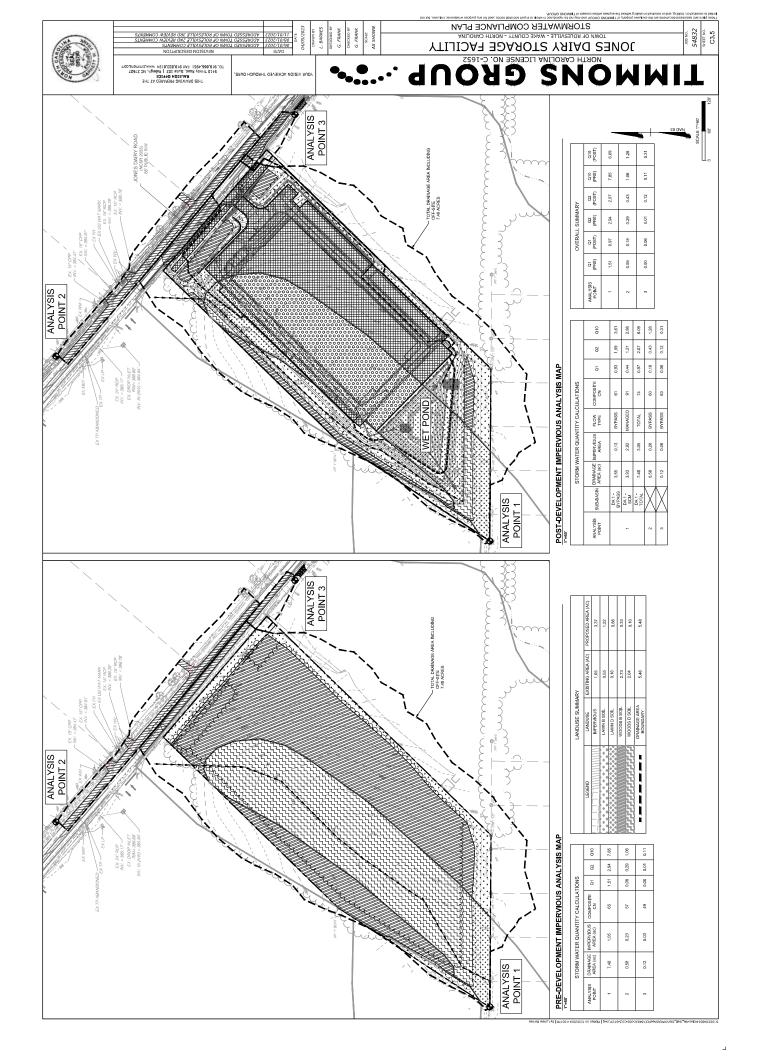
Compute by: Known Q Known Q (cfs) = 0.46 Highlighted Depth (ft) = 0.11Q (cfs) = 0.460Area (sqft) = 0.21Velocity (ft/s) = 2.15Wetted Perim (ft) = 4.07Crit Depth, Yc (ft) = 0.13Spread Width (ft) = 3.96EGL (ft) = 0.18



Reach (ft)

PRE/POST STORMWATER MAP





PRE/POST DRAINAGE AREA CALCULATIONS



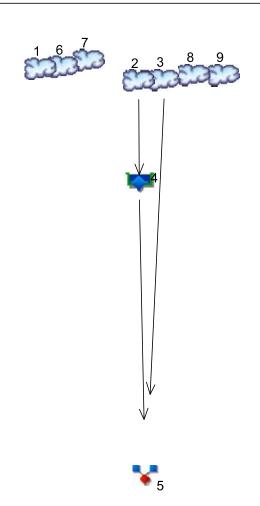
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### **Watershed Model Schematic**



#### **Legend**

<u>Hyd.</u>	<u>Origin</u>	<u>Description</u>
1	SCS Runoff	Analysis Point 1_PRE
2	SCS Runoff	Analysis Point 1_POST SCM
3	SCS Runoff	Analysis Point 1_POST BYPASS
4	Reservoir	Wetpond
5	Combine	POST SITE
6	SCS Runoff	Analysis Point 2_PRE
7	SCS Runoff	Analysis Point 3_PRE
8	SCS Runoff	Analysis Point 2_POST
9	SCS Runoff	Analysis Point 3_POST

Project: Rolesville Stormwater Compliance\_NCDOT.gpw

Monday, 01 / 29 / 2024

# Hydrograph Return Period Recap

		Inflow hyd(s)				Hydrograph					
No.			1-yr	2-yr	3-yr	5 <b>-</b> yr	10-yr	25-yr	50-yr	100-yr	Description
1	SCS Runoff		2.063	4.009			10.71			23.44	Analysis Point 1_PRE
2	SCS Runoff		10.85	13.85			21.66			33.79	Analysis Point 1_POST SCM
3	SCS Runoff		0.704	1.658			5.251			12.38	Analysis Point 1_POST BYPASS
4	Reservoir	2	0.437	1.213			2.863			10.53	Wetpond
5	Combine	3, 4	0.745	2.255			7.737			20.55	POST SITE
3	SCS Runoff		0.089	0.289			1.062			2.617	Analysis Point 2_PRE
7	SCS Runoff		0.001	0.005			0.105			0.370	Analysis Point 3_PRE
8	SCS Runoff		0.178	0.432			1.279			2.923	Analysis Point 2_POST
9	SCS Runoff		0.061	0.120			0.310			0.667	Analysis Point 3_POST

Proj. file: Rolesville Stormwater Compliance\_NCDOT.gpw

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# **Hydrograph Summary Report**

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

2         SCS Runoff         10.85         2         720         28,557           Analysis Point 1_POST SCM           3         SCS Runoff         0.704         2         728         4,036           Analysis Point 1_POST BYPASS           4         Reservoir         0.437         2         836         18,230         2         376.98         19,520         Wetpond           5         Combine         0.745         2         728         22,267         3,4          POST SITE           6         SCS Runoff         0.089         2         720         405           Analysis Point 2_PRE           7         SCS Runoff         0.001         2         902         22           Analysis Point 2_PRE           8         SCS Runoff         0.178         2         720         561           Analysis Point 2_POST	Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
3         SCS Runoff         0.704         2         728         4,036            Analysis Point 1_POST BYPASS           4         Reservoir         0.437         2         836         18,230         2         376.98         19,520         Wetpond           5         Combine         0.745         2         728         22,267         3,4          POST SITE           6         SCS Runoff         0.089         2         720         405           Analysis Point 2_PRE           7         SCS Runoff         0.001         2         902         22           Analysis Point 3_PRE           8         SCS Runoff         0.178         2         720         561           Analysis Point 2_POST	1	SCS Runoff	2.063	2	734	12,044				Analysis Point 1_PRE
4       Reservoir       0.437       2       836       18,230       2       376.98       19,520       Wetpond         5       Combine       0.745       2       728       22,267       3,4        POST SITE         6       SCS Runoff       0.089       2       720       405         Analysis Point 2_PRE         7       SCS Runoff       0.001       2       902       22         Analysis Point 3_PRE         8       SCS Runoff       0.178       2       720       561         Analysis Point 2_POST	2	SCS Runoff	10.85	2	720	28,557				Analysis Point 1_POST SCM
5         Combine         0.745         2         728         22,267         3, 4          POST SITE           6         SCS Runoff         0.089         2         720         405           Analysis Point 2_PRE           7         SCS Runoff         0.001         2         902         22           Analysis Point 3_PRE           8         SCS Runoff         0.178         2         720         561           Analysis Point 2_POST	3	SCS Runoff	0.704	2	728	4,036				Analysis Point 1_POST BYPASS
6         SCS Runoff         0.089         2         720         405           Analysis Point 2_PRE           7         SCS Runoff         0.001         2         902         22           Analysis Point 3_PRE           8         SCS Runoff         0.178         2         720         561           Analysis Point 2_POST	4	Reservoir	0.437	2	836	18,230	2	376.98	19,520	Wetpond
7 SCS Runoff 0.001 2 902 22 Analysis Point 3_PRE 8 SCS Runoff 0.178 2 720 561 Analysis Point 2_POST	5	Combine	0.745	2	728	22,267	3, 4			POST SITE
8 SCS Runoff 0.178 2 720 561 Analysis Point 2_POST	6	SCS Runoff	0.089	2	720	405				Analysis Point 2_PRE
	7	SCS Runoff	0.001	2	902	22				Analysis Point 3_PRE
9 SCS Runoff 0.061 2 718 153 Analysis Point 3_POST	8	SCS Runoff	0.178	2	720	561				Analysis Point 2_POST
	9	SCS Runoff	0.061	2	718	153				Analysis Point 3_POST

### **Hydrograph Report**

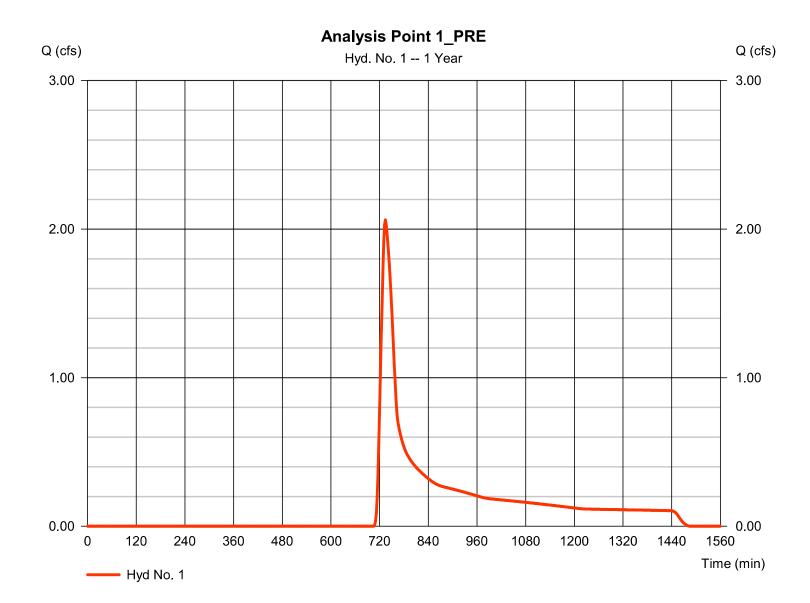
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

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#### Hyd. No. 1

Analysis Point 1\_PRE

Hydrograph type Peak discharge = 2.063 cfs= SCS Runoff Storm frequency Time to peak = 734 min = 1 yrsTime interval = 2 min Hyd. volume = 12,044 cuft = 7.480 acCurve number Drainage area = 65 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc) = 28.10 min Tc method = TR55 Total precip. = 2.86 inDistribution = Type II Storm duration = 24 hrs = 484 Shape factor



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**Hyd. No. 1**Analysis Point 1\_PRE

<u>Description</u>	A		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)  Travel Time (min)	= 0.400 = 100.0 = 3.46 = 2.93 = 17.73	+	0.011 0.0 0.00 0.00	+	0.011 0.0 0.00 0.00	=	17.73
, ,	- 17.70	•	0.00	•	0.00	_	17.70
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 1315.00 = 1.71 = Unpaved =2.11	i	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 10.39	+	0.00	+	0.00	=	10.39
Channel Flow							
X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015		0.00 0.00 0.00 0.015		
					0.00		
Flow length (ft)	0.0({0})		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							28.10 min

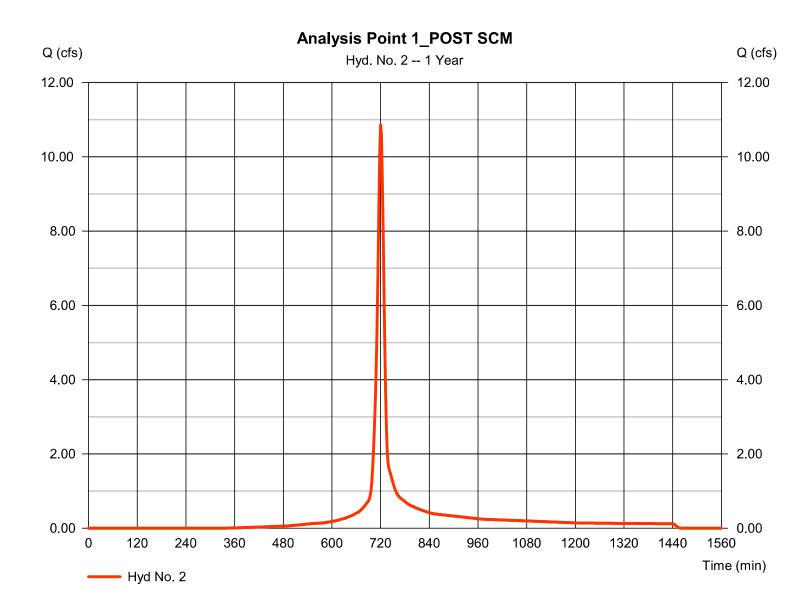
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#### Hyd. No. 2

Analysis Point 1\_POST SCM

Hydrograph type Peak discharge = SCS Runoff = 10.85 cfsStorm frequency Time to peak = 720 min = 1 yrsTime interval = 2 min Hyd. volume = 28,557 cuft Drainage area Curve number = 3.930 ac= 91 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method Time of conc. (Tc) = 13.20 min = User Total precip. = 2.86 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Q (cfs)

### **Hydrograph Report**

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

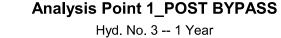
Monday, 01 / 29 / 2024

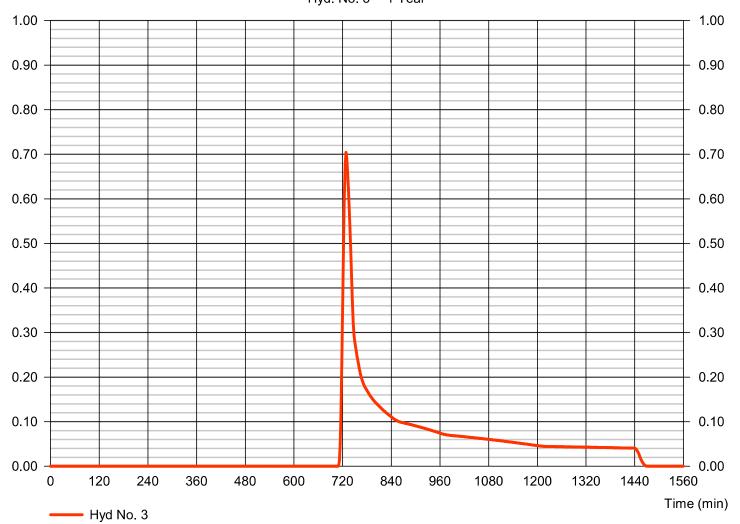
#### Hyd. No. 3

Q (cfs)

Analysis Point 1\_POST BYPASS

Hydrograph type Peak discharge = SCS Runoff = 0.704 cfsStorm frequency Time to peak = 728 min = 1 yrsTime interval = 2 min Hyd. volume = 4,036 cuft Drainage area Curve number = 3.550 ac= 61 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method Time of conc. (Tc)  $= 20.00 \, \text{min}$ = User Total precip. Distribution = Type II = 2.86 inStorm duration = 24 hrs Shape factor = 484





Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

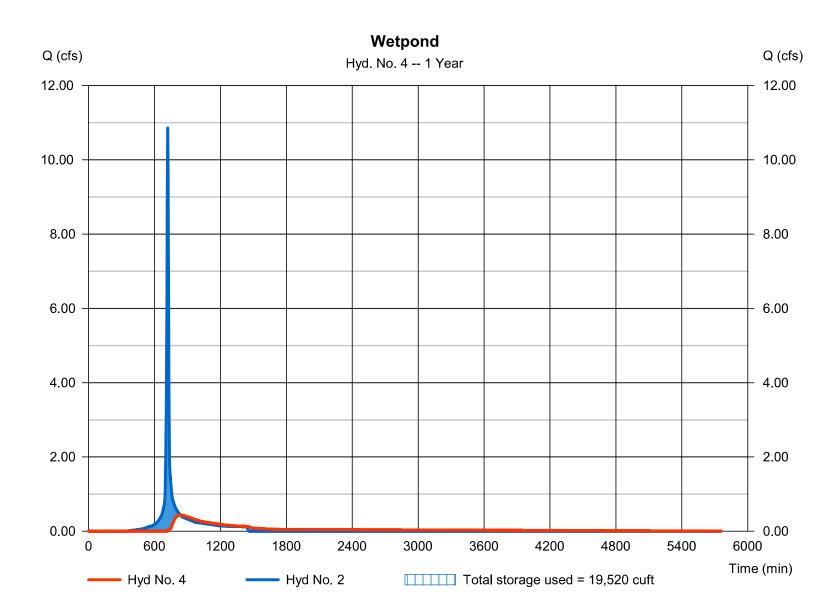
Monday, 01 / 29 / 2024

#### Hyd. No. 4

Wetpond

Hydrograph type Peak discharge = Reservoir = 0.437 cfsStorm frequency Time to peak = 836 min = 1 yrsHyd. volume Time interval = 2 min = 18,230 cuftInflow hyd. No. = 2 - Analysis Point 1\_POST SCMax. Elevation = 376.98 ftMax. Storage = Wet Pond Reservoir name = 19,520 cuft

Storage Indication method used.



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= 0.745 cfs

= 728 min

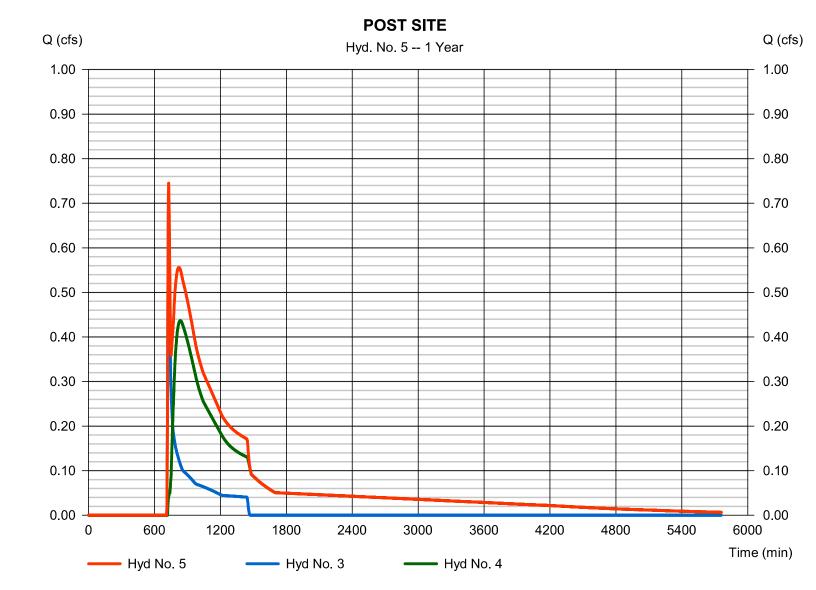
= 3.550 ac

= 22,267 cuft

### Hyd. No. 5

**POST SITE** 

Hydrograph type= CombinePeak dischargeStorm frequency= 1 yrsTime to peakTime interval= 2 minHyd. volumeInflow hyds.= 3, 4Contrib. drain. area



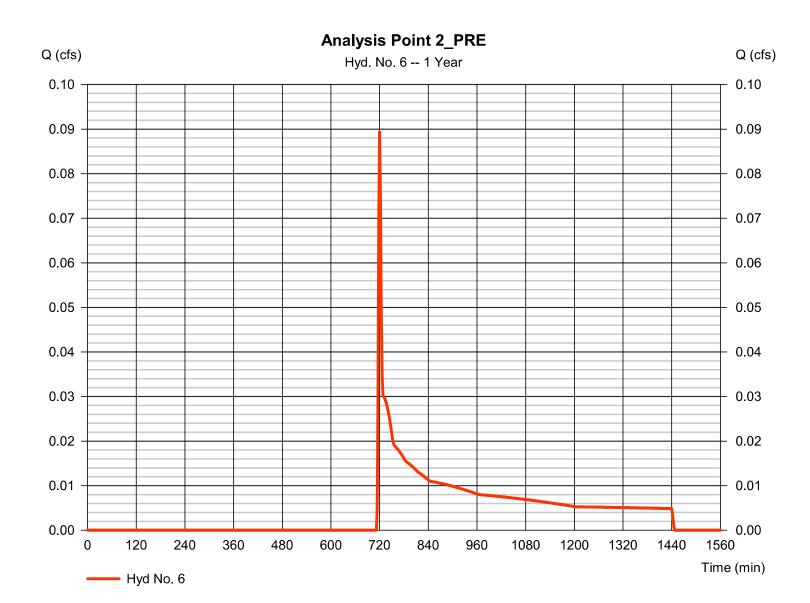
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#### Hyd. No. 6

Analysis Point 2\_PRE

Hydrograph type Peak discharge = SCS Runoff = 0.089 cfsStorm frequency Time to peak = 720 min = 1 yrsTime interval = 2 min Hyd. volume = 405 cuft Drainage area Curve number = 0.580 ac= 57 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc)  $= 5.00 \, \text{min}$ Tc method = User Total precip. Distribution = Type II = 2.86 inStorm duration = 24 hrs = 484 Shape factor



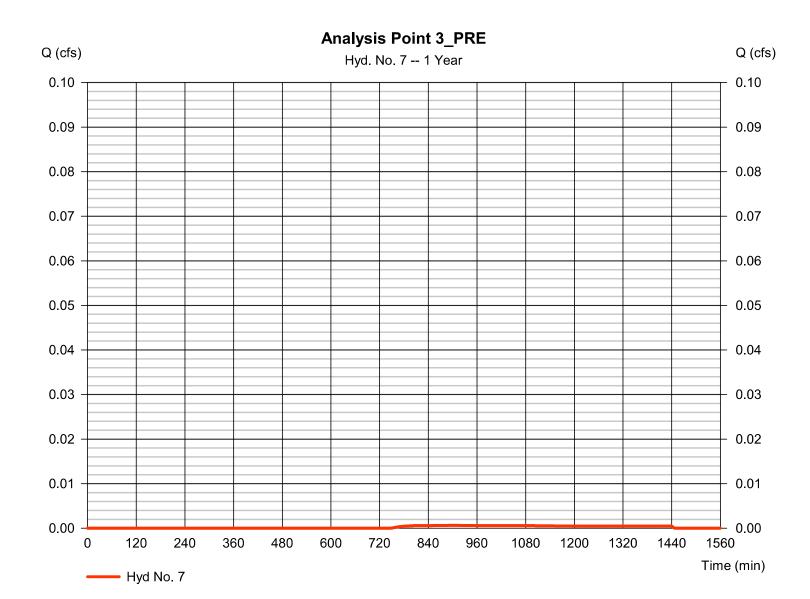
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#### Hyd. No. 7

Analysis Point 3\_PRE

Hydrograph type Peak discharge = SCS Runoff = 0.001 cfsStorm frequency Time to peak = 902 min = 1 yrsTime interval = 2 min Hyd. volume = 22 cuft Drainage area Curve number = 0.120 ac= 49 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method Time of conc. (Tc)  $= 5.00 \, \text{min}$ = User Total precip. Distribution = Type II = 2.86 inStorm duration = 24 hrs Shape factor = 484



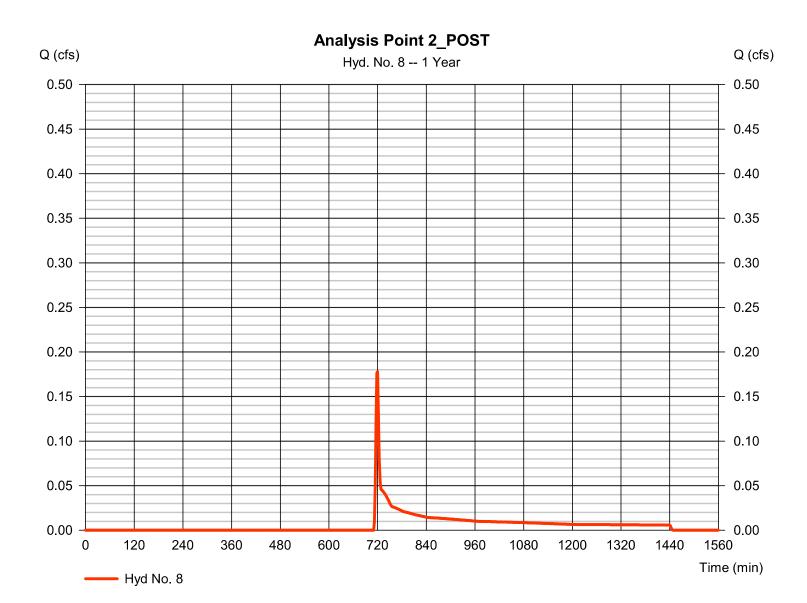
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#### Hyd. No. 8

Analysis Point 2\_POST

Hydrograph type Peak discharge = SCS Runoff = 0.178 cfsStorm frequency Time to peak = 720 min = 1 yrsTime interval = 2 min Hyd. volume = 561 cuft Drainage area Curve number = 0.580 ac= 60 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc)  $= 5.00 \, \text{min}$ Tc method = User Total precip. Distribution = Type II = 2.86 inStorm duration = 24 hrs = 484 Shape factor



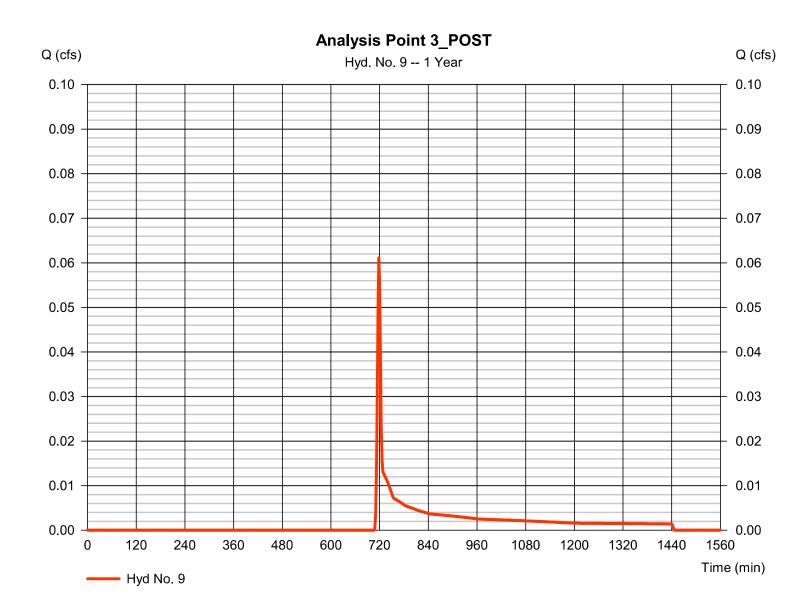
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#### Hyd. No. 9

Analysis Point 3\_POST

Hydrograph type Peak discharge = SCS Runoff = 0.061 cfsStorm frequency Time to peak = 718 min = 1 yrsTime interval = 2 min Hyd. volume = 153 cuft Drainage area Curve number = 0.120 ac= 63 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc)  $= 5.00 \, \text{min}$ Tc method = User Total precip. Distribution = Type II = 2.86 inStorm duration = 24 hrs = 484 Shape factor



# **Hydrograph Summary Report**

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

lyd. Io.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	4.009	2	732	19,850				Analysis Point 1_PRE
2	SCS Runoff	13.85	2	720	36,827				Analysis Point 1_POST SCM
3	SCS Runoff	1.658	2	728	7,150				Analysis Point 1_POST BYPASS
4	Reservoir	1.213	2	758	26,493	2	377.24	22,234	Wetpond
5	Combine	2.255	2	732	33,643	3, 4			POST SITE
6	SCS Runoff	0.289	2	718	791				Analysis Point 2_PRE
7	SCS Runoff	0.005	2	722	66				Analysis Point 3_PRE
8	SCS Runoff	0.432	2	718	1,015				Analysis Point 2_POST
9	SCS Runoff	0.120	2	718	261				Analysis Point 3_POST
	esville Storm	-10	<u> </u>	NODOT				Monday, 0	

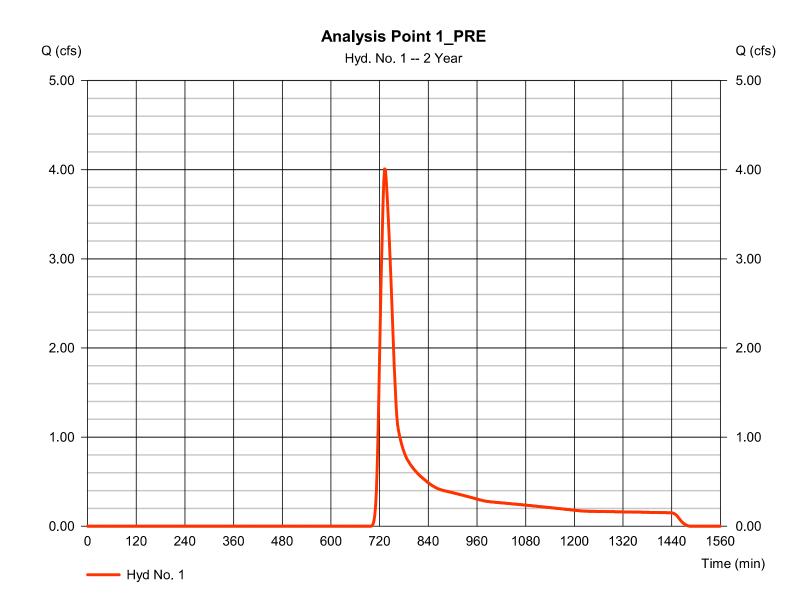
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

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#### Hyd. No. 1

Analysis Point 1\_PRE

Hydrograph type = SCS Runoff Peak discharge = 4.009 cfsStorm frequency Time to peak = 732 min = 2 yrsTime interval = 2 min Hyd. volume = 19,850 cuftCurve number Drainage area = 7.480 ac= 65 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc) = 28.10 min Tc method = TR55 Total precip. Distribution = Type II = 3.46 inStorm duration = 24 hrs = 484 Shape factor



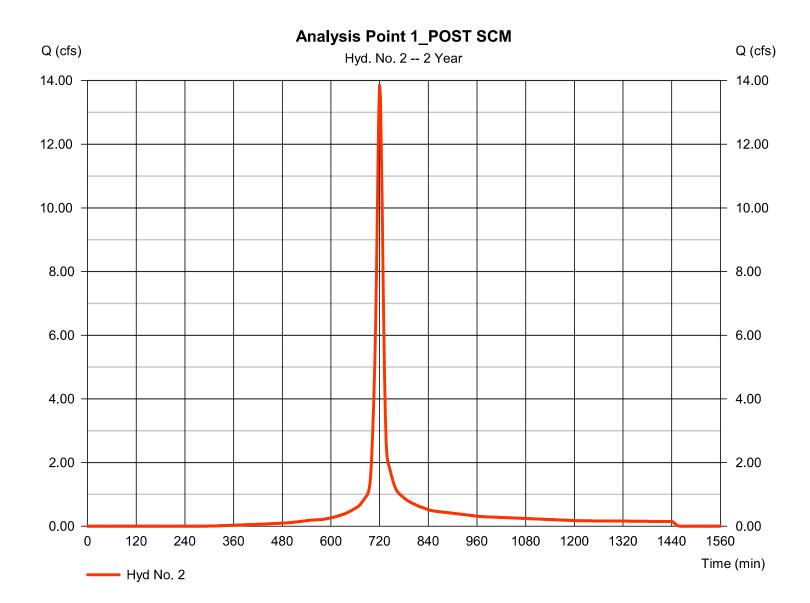
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#### Hyd. No. 2

Analysis Point 1\_POST SCM

Peak discharge Hydrograph type = SCS Runoff = 13.85 cfsStorm frequency Time to peak = 720 min = 2 yrsTime interval = 2 min Hyd. volume = 36,827 cuft Curve number Drainage area = 3.930 ac= 91 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc) Tc method = User  $= 13.20 \, \text{min}$ Total precip. = 3.46 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



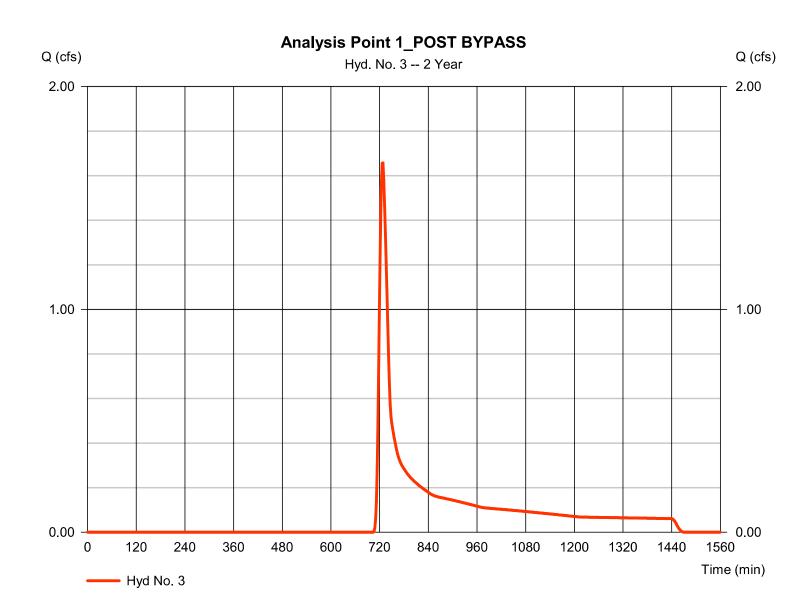
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#### Hyd. No. 3

Analysis Point 1\_POST BYPASS

Hydrograph type Peak discharge = SCS Runoff = 1.658 cfsStorm frequency Time to peak = 728 min = 2 yrsTime interval = 2 min Hyd. volume = 7,150 cuftDrainage area = 3.550 acCurve number = 61 = 0 ftBasin Slope = 0.0 % Hydraulic length Tc method Time of conc. (Tc)  $= 20.00 \, \text{min}$ = User Total precip. = 3.46 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



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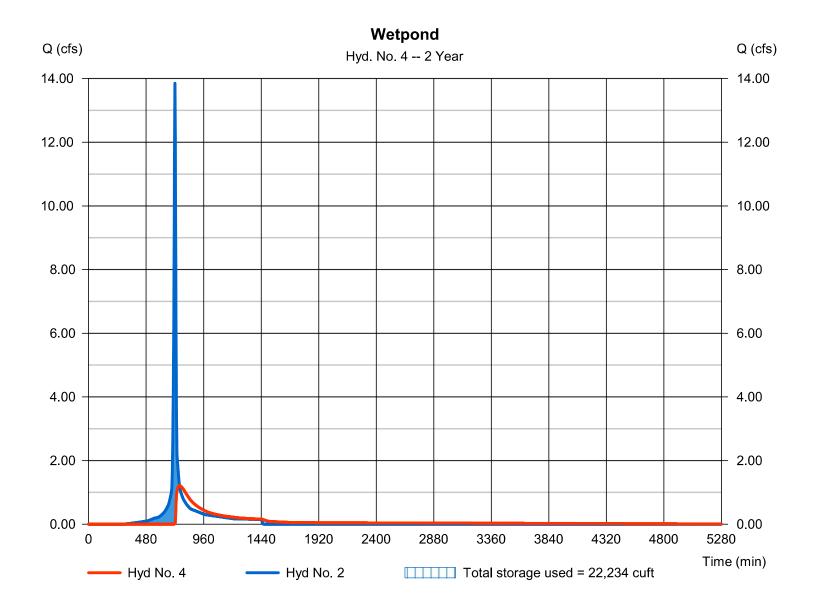
Monday, 01 / 29 / 2024

#### Hyd. No. 4

Wetpond

Hydrograph type = Reservoir Peak discharge = 1.213 cfsStorm frequency = 2 yrsTime to peak = 758 min Time interval = 2 min Hyd. volume = 26,493 cuft = 2 - Analysis Point 1\_POST SCMax. Elevation = 377.24 ftInflow hyd. No. = Wet Pond Reservoir name Max. Storage = 22,234 cuft

Storage Indication method used.



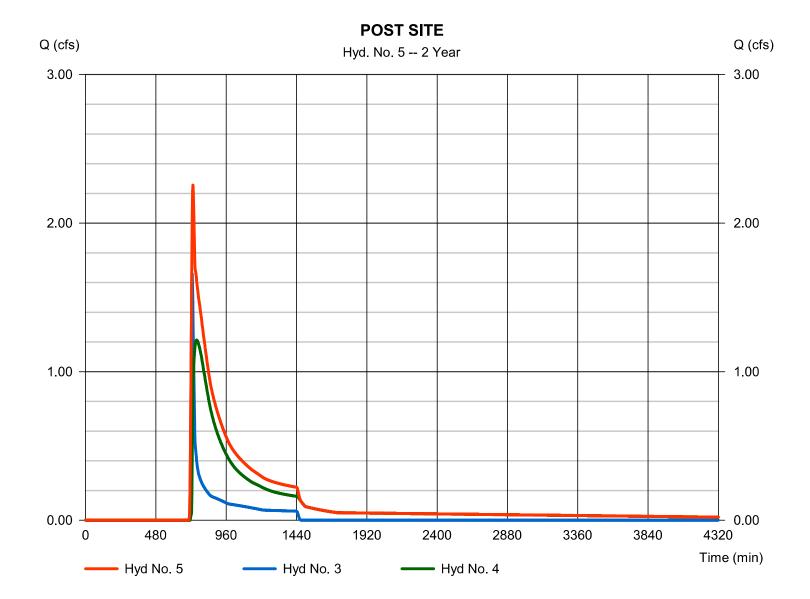
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#### Hyd. No. 5

**POST SITE** 

Hydrograph type = Combine Peak discharge = 2.255 cfsStorm frequency = 2 yrs Time to peak = 732 min Time interval = 2 min Hyd. volume = 33,643 cuft Inflow hyds. = 3, 4 Contrib. drain. area = 3.550 ac



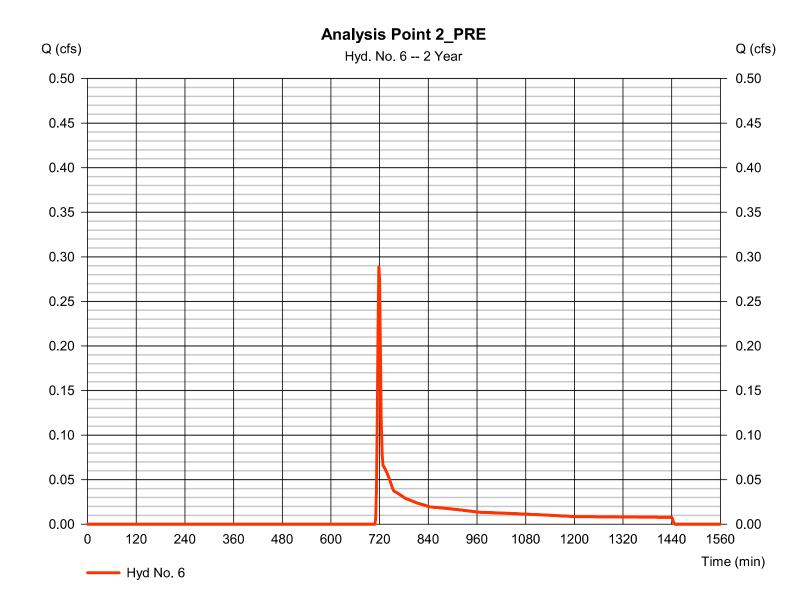
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#### Hyd. No. 6

Analysis Point 2\_PRE

Hydrograph type Peak discharge = SCS Runoff = 0.289 cfsStorm frequency Time to peak = 718 min = 2 yrsTime interval = 2 min Hyd. volume = 791 cuft Curve number Drainage area = 0.580 ac= 57 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc)  $= 5.00 \, \text{min}$ Tc method = User Total precip. Distribution = Type II = 3.46 inStorm duration = 24 hrs = 484 Shape factor



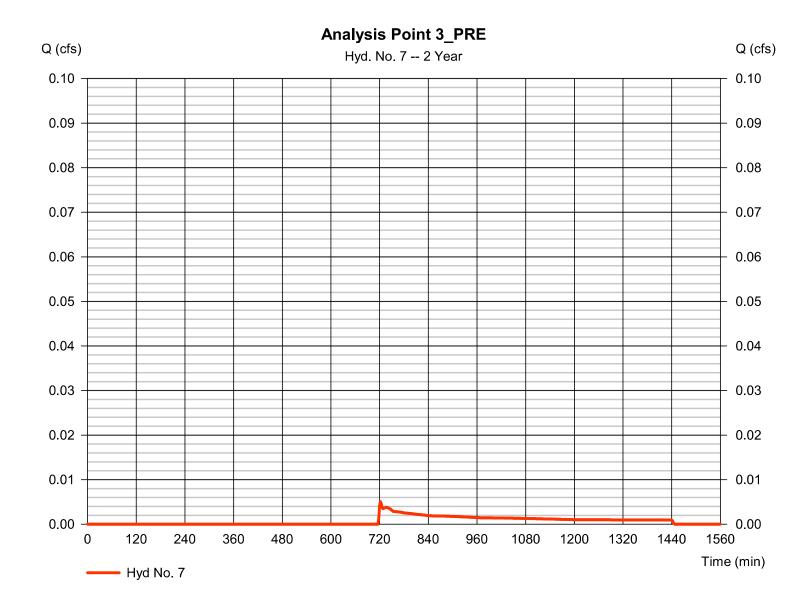
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#### Hyd. No. 7

Analysis Point 3\_PRE

Hydrograph type Peak discharge = SCS Runoff = 0.005 cfsStorm frequency Time to peak = 722 min = 2 yrsTime interval = 2 min Hyd. volume = 66 cuft Drainage area Curve number = 0.120 ac= 49 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc)  $= 5.00 \, \text{min}$ Tc method = User Total precip. Distribution = Type II = 3.46 inStorm duration = 24 hrs Shape factor = 484



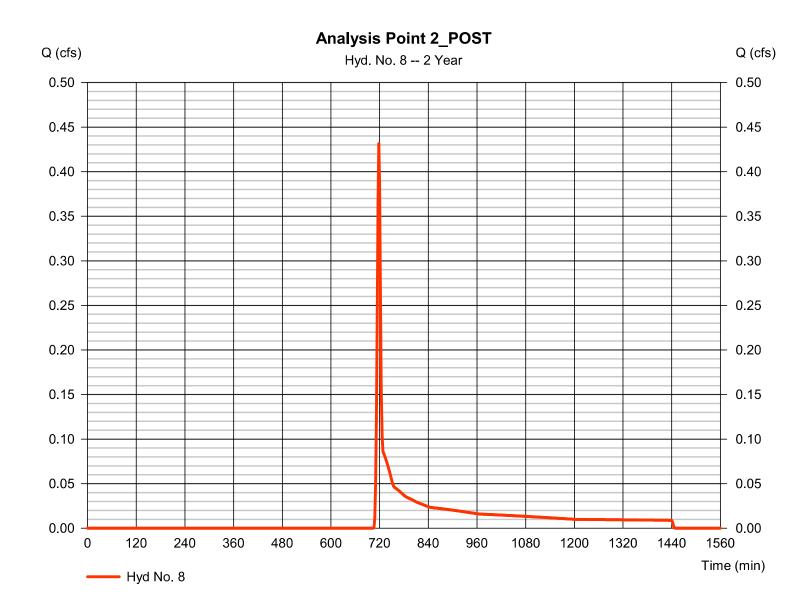
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#### Hyd. No. 8

Analysis Point 2\_POST

Hydrograph type Peak discharge = SCS Runoff = 0.432 cfsStorm frequency Time to peak = 718 min = 2 yrsTime interval = 2 min Hyd. volume = 1,015 cuft Drainage area Curve number = 0.580 ac= 60 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc)  $= 5.00 \, \text{min}$ Tc method = User Total precip. Distribution = Type II = 3.46 inStorm duration = 24 hrs = 484 Shape factor



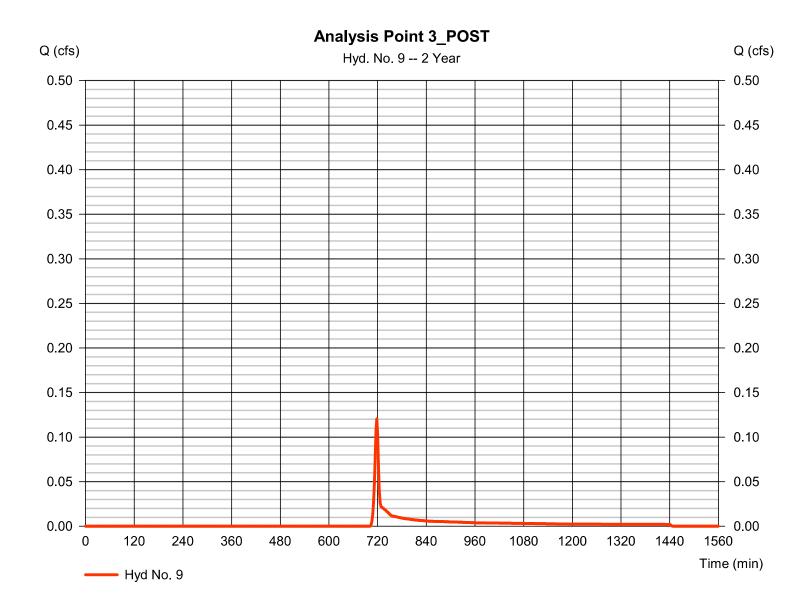
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#### Hyd. No. 9

Analysis Point 3\_POST

Hydrograph type Peak discharge = SCS Runoff = 0.120 cfsStorm frequency Time to peak = 718 min = 2 yrsTime interval = 2 min Hyd. volume = 261 cuft Drainage area Curve number = 0.120 ac= 63 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc)  $= 5.00 \, \text{min}$ Tc method = User Total precip. Distribution = Type II = 3.46 inStorm duration = 24 hrs = 484 Shape factor



# **Hydrograph Summary Report**

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

lyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	10.71	2	732	45,440				Analysis Point 1_PRE
2	SCS Runoff	21.66	2	720	59,012				Analysis Point 1_POST SCM
3	SCS Runoff	5.251	2	726	17,876				Analysis Point 1_POST BYPASS
4	Reservoir	2.863	2	744	48,663	2	378.33	34,009	Wetpond
5	Combine	7.737	2	726	66,539	3, 4			POST SITE
6	SCS Runoff	1.062	2	718	2,212				Analysis Point 2_PRE
7	SCS Runoff	0.105	2	718	266				Analysis Point 3_PRE
8	SCS Runoff	1.279	2	718	2,603				Analysis Point 2_POST
9	SCS Runoff	0.310	2	718	624				Analysis Point 3_POST

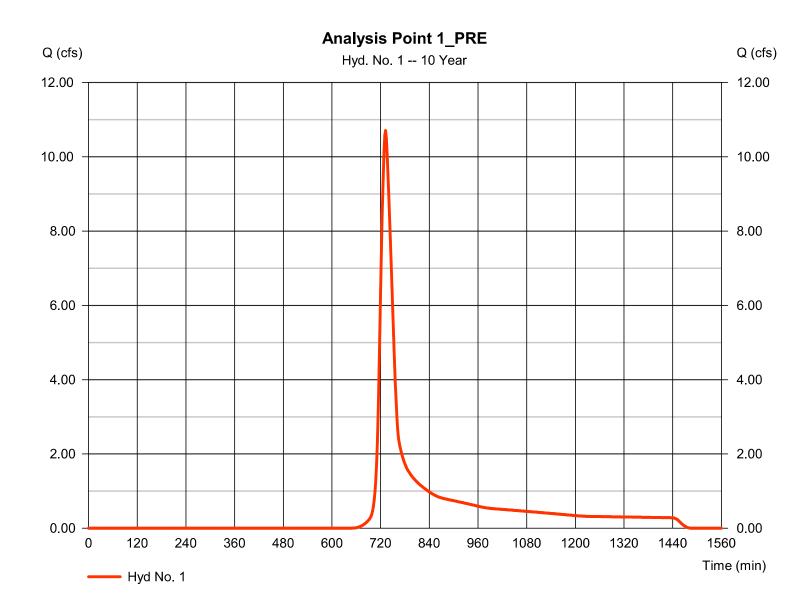
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

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#### Hyd. No. 1

Analysis Point 1\_PRE

Hydrograph type = SCS Runoff Peak discharge = 10.71 cfsStorm frequency = 10 yrsTime to peak = 732 min Time interval = 2 min Hyd. volume = 45,440 cuft Drainage area = 7.480 acCurve number = 65 = 0.0 % Basin Slope Hydraulic length = 0 ftTime of conc. (Tc) = 28.10 min Tc method = TR55 Total precip. = 5.03 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



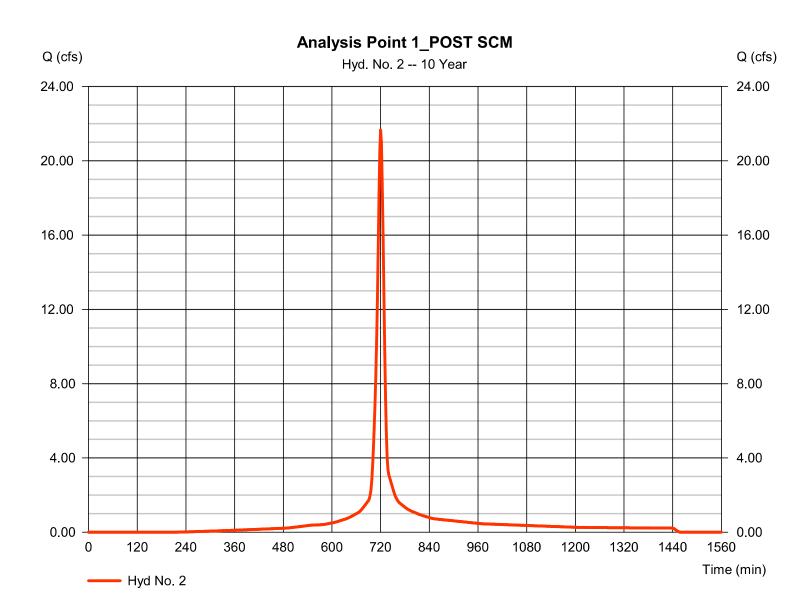
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#### Hyd. No. 2

Analysis Point 1 POST SCM

Peak discharge Hydrograph type = SCS Runoff = 21.66 cfsStorm frequency = 10 yrsTime to peak = 720 min Time interval = 2 min Hyd. volume = 59,012 cuftCurve number = 91 Drainage area = 3.930 acBasin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc) Tc method = User  $= 13.20 \, \text{min}$ Total precip. = 5.03 inDistribution = Type II Storm duration = 24 hrs = 484 Shape factor



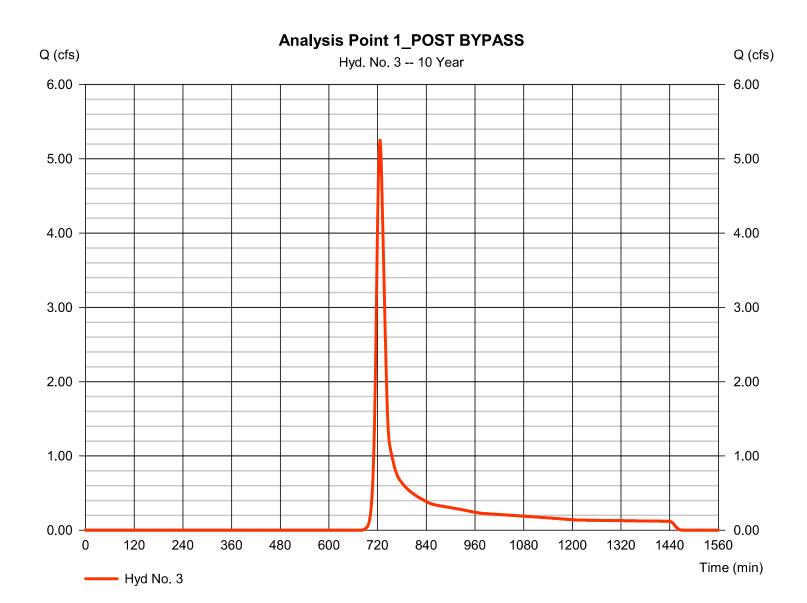
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

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#### Hyd. No. 3

Analysis Point 1\_POST BYPASS

= SCS Runoff Peak discharge = 5.251 cfsHydrograph type Storm frequency Time to peak = 726 min = 10 yrsTime interval = 2 min Hyd. volume = 17,876 cuftCurve number Drainage area = 3.550 ac= 61 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc)  $= 20.00 \, \text{min}$ Tc method = User Total precip. = 5.03 inDistribution = Type II Storm duration = 24 hrs = 484 Shape factor



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

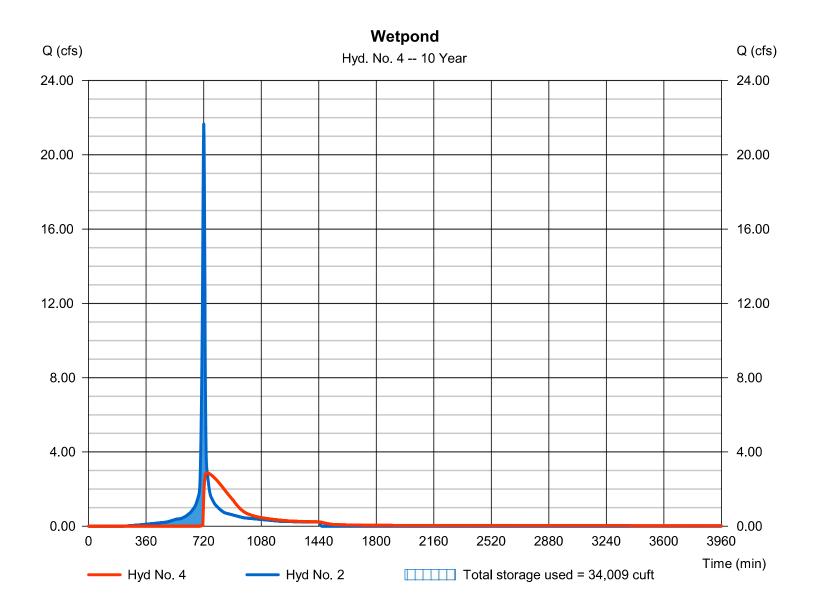
Monday, 01 / 29 / 2024

#### Hyd. No. 4

Wetpond

Hydrograph type = Reservoir Peak discharge = 2.863 cfsStorm frequency = 10 yrsTime to peak = 744 min Hyd. volume Time interval = 2 min = 48,663 cuft= 2 - Analysis Point 1\_POST SCMax. Elevation Inflow hyd. No. = 378.33 ft= Wet Pond Reservoir name Max. Storage = 34,009 cuft

Storage Indication method used.



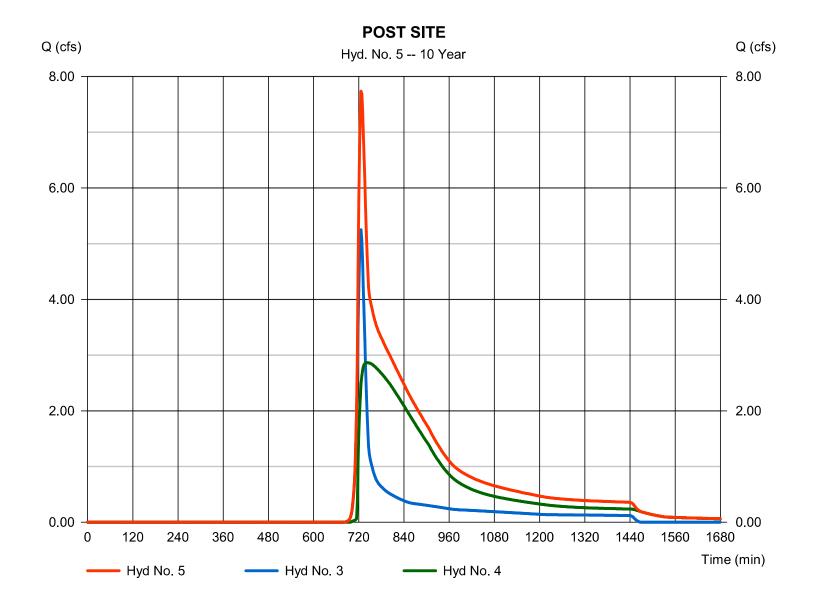
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

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#### Hyd. No. 5

**POST SITE** 

Hydrograph type = Combine Storm frequency = 10 yrs Time interval = 2 min Inflow hyds. = 3, 4 Peak discharge = 7.737 cfs
Time to peak = 726 min
Hyd. volume = 66,539 cuft
Contrib. drain. area = 3.550 ac



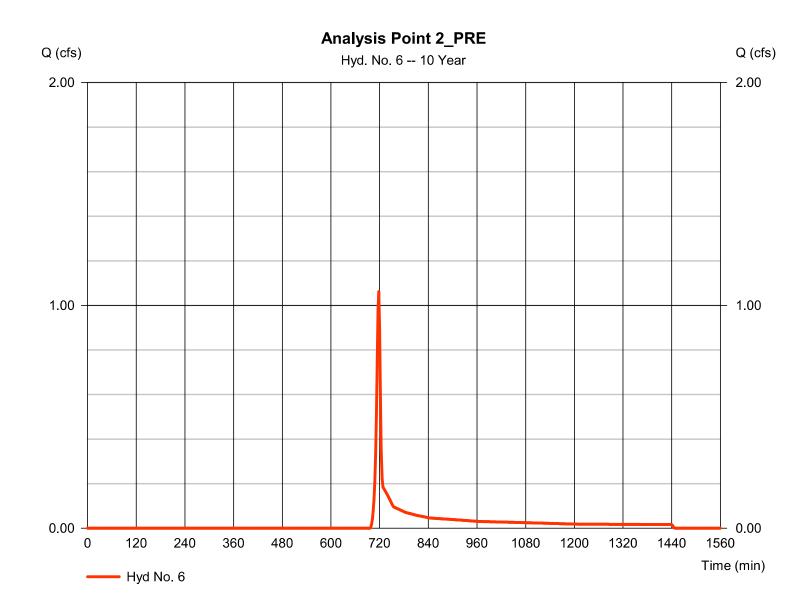
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#### Hyd. No. 6

Analysis Point 2\_PRE

Hydrograph type = SCS Runoff Peak discharge = 1.062 cfsStorm frequency = 10 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 2,212 cuft Drainage area Curve number = 0.580 ac= 57 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc)  $= 5.00 \, \text{min}$ Tc method = User Total precip. = 5.03 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



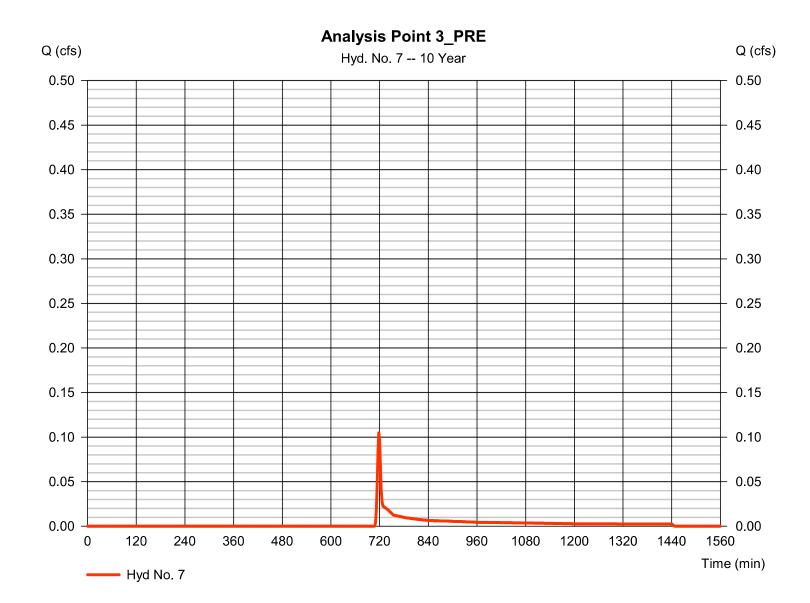
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Monday, 01 / 29 / 2024

#### Hyd. No. 7

Analysis Point 3\_PRE

Hydrograph type = SCS Runoff Peak discharge = 0.105 cfsStorm frequency = 10 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 266 cuft Drainage area Curve number = 0.120 ac= 49 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc)  $= 5.00 \, \text{min}$ Tc method = User Total precip. = 5.03 inDistribution = Type II Storm duration = 24 hrs = 484 Shape factor



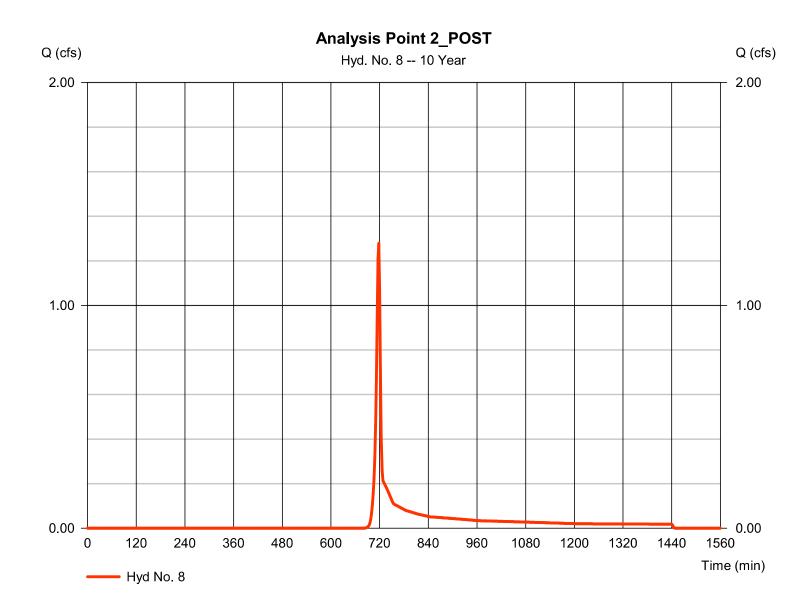
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Monday, 01 / 29 / 2024

#### Hyd. No. 8

Analysis Point 2\_POST

Hydrograph type = SCS Runoff Peak discharge = 1.279 cfsStorm frequency = 10 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 2,603 cuftDrainage area Curve number = 0.580 ac= 60 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc)  $= 5.00 \, \text{min}$ Tc method = User Total precip. = 5.03 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



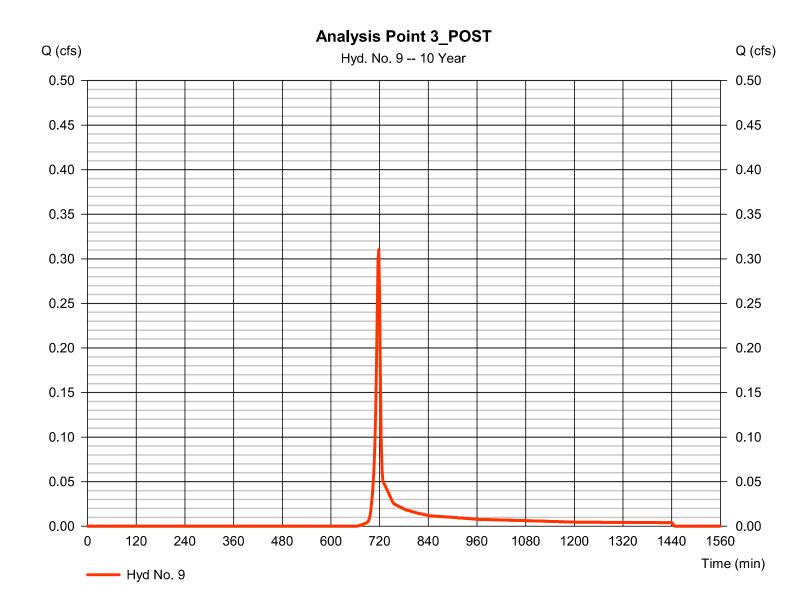
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Monday, 01 / 29 / 2024

#### Hyd. No. 9

Analysis Point 3\_POST

Hydrograph type Peak discharge = SCS Runoff = 0.310 cfsStorm frequency = 10 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 624 cuft Curve number Drainage area = 0.120 ac= 63 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc)  $= 5.00 \, \text{min}$ Tc method = User Total precip. = 5.03 inDistribution = Type II Storm duration = 24 hrs = 484 Shape factor



## **Hydraflow Rainfall Report**

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Monday, 01 / 29 / 2024

Return Period	Intensity-Duration-Frequency Equation Coefficients (FHA)									
(Yrs)	В	D	E	(N/A)						
1	67.6239	13.2000	0.9048							
2	0.0000	0.0000	0.0000							
3	0.0000	0.0000	0.0000							
5	0.0000	0.0000	0.0000							
10	68.3177	11.9000	0.7902							
25	62.3903	11.0000	0.7384							
50	0.0000	0.0000	0.0000							
100	0.0000	0.0000	0.0000							

File name: Town of Rolesville.IDF

#### Intensity = B / (Tc + D)^E

Return	Intensity Values (in/hr)													
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60		
1	4.90	3.93	3.30	2.84	2.50	2.24	2.03	1.86	1.71	1.59	1.48	1.39		
2	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
5	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
10	7.32	5.96	5.07	4.43	3.95	3.57	3.27	3.01	2.80	2.62	2.47	2.33		
25	8.05	6.59	5.63	4.94	4.43	4.02	3.69	3.42	3.19	3.00	2.83	2.68		
50	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
100	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		

Tc = time in minutes. Values may exceed 60.

Precip. file name: S:\333\54832-Rolesville\_Self\_Stor\Calc\Stm\Town of Rolesville.pcp

		Rainfall Precipitation Table (in)											
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr					
SCS 24-hour	2.86	3.46	0.00	4.40	5.03	5.85	6.56	7.50					
SCS 6-Hr	2.05	2.44	0.00	0.00	3.54	4.10	4.55	5.44					
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00					
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00					
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00					
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00					
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00					
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00					